

## **Appendix C – Hydraulic Calculations**

- Normal Depth Calculations – Onsite Streets
- Drop Inlet Calculations
- WSPG Model

## Worksheet for ON3S

Project Description	
Friction Method	Manning Formula
Solve For	Normal Depth

---

Input Data	
Channel Slope	0.840 %
Discharge	8.00 cfs

### Section Definitions

	Station (ft)	Elevation (ft)	
	0+00.00		0.48
	0+05.00		0.38
	0+06.50		0.00
	0+07.50		0.08
	0+07.50		0.13
	0+42.50		0.83
	0+42.50		0.78
	0+43.50		0.70
	0+45.00		1.08

### Roughness Segment Definitions

Start Station	Ending Station	Roughness Coefficient
(0+00.00, 0.48)	(0+45.00, 1.08)	0.016

### Options

Current Roughness Weighted Method	Pavlovskii's Method
Open Channel Weighting Method	Pavlovskii's Method
Closed Channel Weighting Method	Pavlovskii's Method

### Results

Normal Depth	0.44 ft
Roughness Coefficient	0.016
Elevation	0.44 ft
Elevation Range	0.00 to 1.08 ft
Flow Area	3.3 ft <sup>2</sup>
Wetted Perimeter	21.07 ft
Hydraulic Radius	0.15 ft
Top Width	20.97 ft
Normal Depth	0.44 ft
Critical Depth	0.45 ft
Critical Slope	0.693 %
Velocity	2.45 ft/s

## Worksheet for ON3S

---

### Results

---

Velocity Head	0.09 ft
Specific Energy	0.53 ft
Froude Number	1.097
Flow Type	Supercritical

---

### GVF Input Data

---

Downstream Depth	0.00 ft
Length	0.00 ft
Number Of Steps	0

---

### GVF Output Data

---

Upstream Depth	0.00 ft
Profile Description	N/A
Profile Headloss	0.00 ft
Downstream Velocity	Infinity ft/s
Upstream Velocity	Infinity ft/s
Normal Depth	0.44 ft
Critical Depth	0.45 ft
Channel Slope	0.840 %
Critical Slope	0.693 %

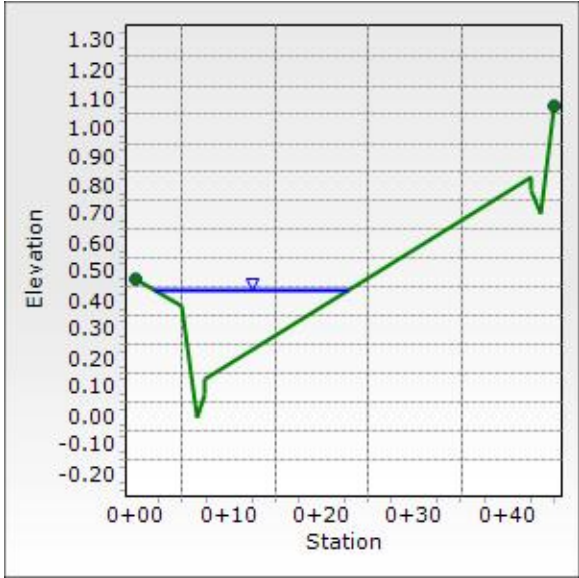
---

# Cross Section for ON3S

Project Description	
Friction Method	Manning Formula
Solve For	Normal Depth

Input Data	
Channel Slope	0.840 %
Normal Depth	0.44 ft
Discharge	8.00 cfs



## Worksheet for ON8

---

### Project Description

---

Friction Method	Manning Formula
Solve For	Normal Depth

---

### Input Data

---

Channel Slope	1.890 %
Discharge	4.00 cfs

---

### Section Definitions

	Station (ft)	Elevation (ft)
	0+00.00	0.38
	0+01.50	0.00
	0+02.50	0.08
	0+02.50	0.13
	0+20.00	0.48
	0+37.50	0.13
	0+37.50	0.08
	0+38.50	0.00
	0+40.00	0.38

### Roughness Segment Definitions

Start Station	Ending Station	Roughness Coefficient
(0+00.00, 0.38)	(0+40.00, 0.38)	0.016

---

### Options

---

Current Roughness Weighted Method	Pavlovskii's Method
Open Channel Weighting Method	Pavlovskii's Method
Closed Channel Weighting Method	Pavlovskii's Method

---



---

### Results

---

Normal Depth	0.26 ft
Roughness Coefficient	0.016
Elevation	0.26 ft
Elevation Range	0.00 to 0.48 ft
Flow Area	1.6 ft <sup>2</sup>
Wetted Perimeter	17.25 ft
Hydraulic Radius	0.09 ft
Top Width	17.08 ft
Normal Depth	0.26 ft
Critical Depth	0.29 ft
Critical Slope	0.799 %
Velocity	2.57 ft/s

## Worksheet for ON8

---

### Results

---

Velocity Head	0.10 ft
Specific Energy	0.36 ft
Froude Number	1.502
Flow Type	Supercritical

---

### GVF Input Data

---

Downstream Depth	0.00 ft
Length	0.00 ft
Number Of Steps	0

---

### GVF Output Data

---

Upstream Depth	0.00 ft
Profile Description	N/A
Profile Headloss	0.00 ft
Downstream Velocity	Infinity ft/s
Upstream Velocity	Infinity ft/s
Normal Depth	0.26 ft
Critical Depth	0.29 ft
Channel Slope	1.890 %
Critical Slope	0.799 %

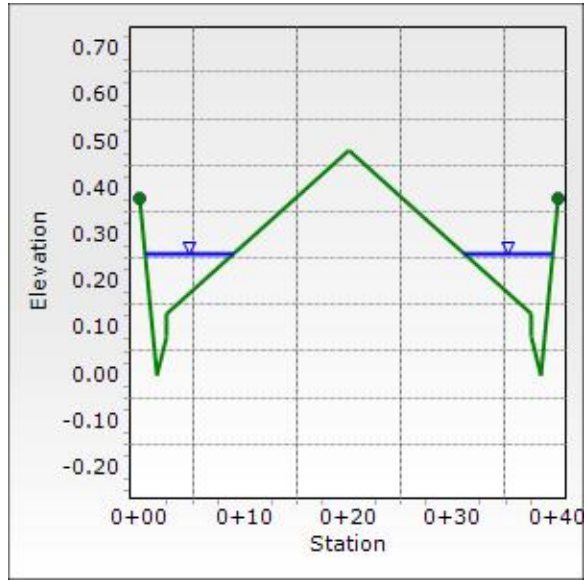
---

# XS for ON8

Project Description	
Friction Method	Manning Formula
Solve For	Normal Depth

Input Data	
Channel Slope	1.890 %
Normal Depth	0.26 ft
Discharge	4.00 cfs



## Worksheet for ON13

---

### Project Description

---

Friction Method	Manning Formula
Solve For	Normal Depth

---

### Input Data

---

Channel Slope	1.890 %
Discharge	3.00 cfs

---

### Section Definitions

	Station (ft)	Elevation (ft)	
	0+00.00		0.38
	0+01.50		0.00
	0+02.50		0.08
	0+02.50		0.13
	0+15.00		0.38
	0+27.50		0.13
	0+27.50		0.08
	0+28.50		0.00
	0+30.00		0.38

### Roughness Segment Definitions

Start Station	Ending Station	Roughness Coefficient
(0+00.00, 0.38)	(0+30.00, 0.38)	0.016

---

### Options

---

Current Roughness Weighted Method	Pavlovskii's Method
Open Channel Weighting Method	Pavlovskii's Method
Closed Channel Weighting Method	Pavlovskii's Method

---



---

### Results

---

Normal Depth	0.24 ft
Roughness Coefficient	0.016
Elevation	0.24 ft
Elevation Range	0.00 to 0.38 ft
Flow Area	1.2 ft <sup>2</sup>
Wetted Perimeter	15.17 ft
Hydraulic Radius	0.08 ft
Top Width	15.00 ft
Normal Depth	0.24 ft
Critical Depth	0.27 ft
Critical Slope	0.828 %
Velocity	2.41 ft/s

## Worksheet for ON13

---

### Results

---

Velocity Head	0.09 ft
Specific Energy	0.33 ft
Froude Number	1.471
Flow Type	Supercritical

---

### GVF Input Data

---

Downstream Depth	0.00 ft
Length	0.00 ft
Number Of Steps	0

---

### GVF Output Data

---

Upstream Depth	0.00 ft
Profile Description	N/A
Profile Headloss	0.00 ft
Downstream Velocity	Infinity ft/s
Upstream Velocity	Infinity ft/s
Normal Depth	0.24 ft
Critical Depth	0.27 ft
Channel Slope	1.890 %
Critical Slope	0.828 %

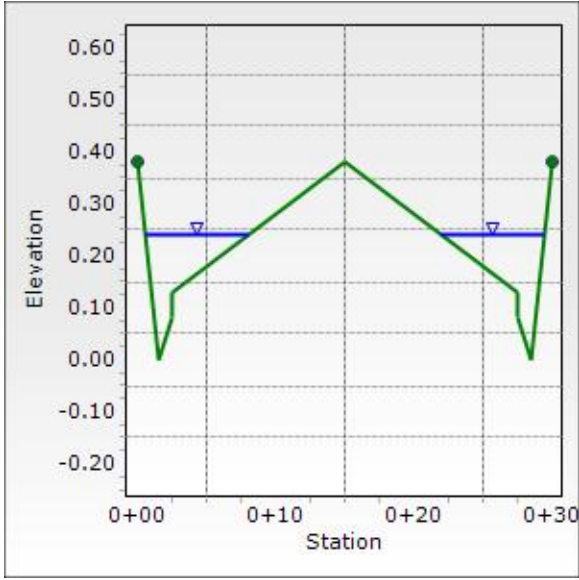
---

# XS for ON13

Project Description	
Friction Method	Manning Formula
Solve For	Normal Depth

Input Data	
Channel Slope	1.890 %
Normal Depth	0.24 ft
Discharge	3.00 cfs



## Worksheet for ON14

Project Description	
Friction Method	Manning Formula
Solve For	Normal Depth

---

Input Data	
Channel Slope	0.500 %
Discharge	4.90 cfs

### Section Definitions

	Station (ft)	Elevation (ft)	
	0+00.00		0.48
	0+05.00		0.38
	0+06.50		0.00
	0+07.50		0.08
	0+07.50		0.13
	0+42.50		0.83
	0+42.50		0.78
	0+43.50		0.70
	0+45.00		1.08

### Roughness Segment Definitions

Start Station	Ending Station	Roughness Coefficient
(0+00.00, 0.48)	(0+45.00, 1.08)	0.016

### Options

Current Roughness Weighted Method	Pavlovskii's Method
Open Channel Weighting Method	Pavlovskii's Method
Closed Channel Weighting Method	Pavlovskii's Method

### Results

Normal Depth	0.41 ft
Roughness Coefficient	0.016
Elevation	0.41 ft
Elevation Range	0.00 to 1.08 ft
Flow Area	2.7 ft <sup>2</sup>
Wetted Perimeter	18.03 ft
Hydraulic Radius	0.15 ft
Top Width	17.92 ft
Normal Depth	0.41 ft
Critical Depth	0.38 ft
Critical Slope	0.715 %
Velocity	1.84 ft/s

## Worksheet for ON14

---

### Results

---

Velocity Head	0.05 ft
Specific Energy	0.46 ft
Froude Number	0.839
Flow Type	Subcritical

---

### GVF Input Data

---

Downstream Depth	0.00 ft
Length	0.00 ft
Number Of Steps	0

---

### GVF Output Data

---

Upstream Depth	0.00 ft
Profile Description	N/A
Profile Headloss	0.00 ft
Downstream Velocity	0.00 ft/s
Upstream Velocity	0.00 ft/s
Normal Depth	0.41 ft
Critical Depth	0.38 ft
Channel Slope	0.500 %
Critical Slope	0.715 %

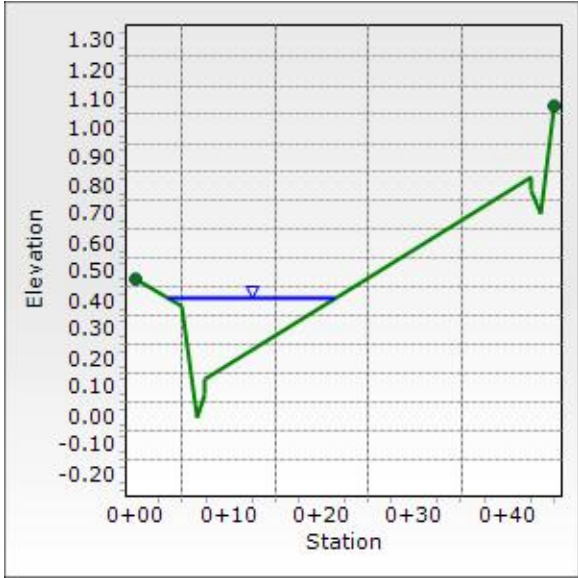
---

# XS for ON14

Project Description	
Friction Method	Manning Formula
Solve For	Normal Depth

Input Data	
Channel Slope	0.500 %
Normal Depth	0.41 ft
Discharge	4.90 cfs



## Type CM Drop Inlet Sizing in Sump

DI#3

### Known:

$D_p$ = Ponding Depth at Inlet	0.33	ft
$W$ = Gutter Width	1.5	ft
$L_g$ = Length of Grate	12.5	ft
$L_c$ = Length of Curb Opening	14.5	ft
$H_c$ = Curb Opening Height	0.5	ft
$C_{wc}$ = Weir coefficient for curb opening	2.3	
$C_{wg}$ = Weir coefficient for grate	3.0	
$C_o$ = Orifice coefficient	0.67	
$R_o$ = Grate opening factor	0.60	
$C_{fg}$ = Clogging Factor for Grate	50	%
$C_{fc}$ = Clogging Factor for Curb Opening	50	%

### Grate Capacity Calculations:

$P_g$ = Active grate weir length (agwl) = $2*W+L_g$	15.50	ft
$P_{agl}$ Adjusted agwl = $2*(1-C_{fg}/100)*W+L_g$	14.00	ft
$A_g$ = Grate area = $W*L_g$	18.75	ft <sup>2</sup>
$A_{aga}$ = Adjusted open grate area = $A_g*R_o*(1-C_{fg}/100)$	5.63	ft <sup>2</sup>
check - If $D_p < 1.792*A_{aga}/P_{agl}$ ; weir; else orifice	weir	
$Q_{ig}$ = Flow intercepted by grate If "weir" controls $Q_{ig}=C_{wg}*P_{agl}*D_p^{1.5}$ If "orifice" controls $Q_{ig}=C_o*A_{aga}*(64.4*D_p)^{0.5}$	8.0	cfs

### Curb Opening Capacity Calculations

$P_c$ = Active curb opening weir length (acowl) = $L_c+1.8*W$	17.20	ft
$P_{acl}$ Adjusted acowl = $L_c+1.8*(1-C_{fc}/100)*W$	15.85	ft
$A_c$ = curb opening area (coa) = $L_c*H_c$	7.25	ft <sup>2</sup>
$A_{aca}$ = Adjusted coa = $A_c*(1-C_{fc}/100)$	3.63	ft <sup>2</sup>
check - If $D_p < H_c$ ; weir; else orifice	weir	
$Q_{ic}$ = Flow intercepted by curb opening If "weir" controls $Q_{ic}=C_{wc}*P_{acl}*D_p^{1.5}$ If "orifice" controls $Q_{ic}=C_o*A_{aca}*(64.4*D_p-H_c/2)^{0.5}$	7.0	cfs

### Total Inlet Calculations

$Q_{ti}$ = Total flow intercepted = $Q_{ig}+Q_{ic}$	15.0	cfs
$q$ = Interception per unit ratio = $Q_{ti}/L_g$	1.03	cfs/ft

## Type CM Drop Inlet Sizing on a C. G.

DI#4

### Known:

Q <sub>s</sub> = Half Street Flow	4.9	cfs
S <sub>o</sub> = Longitudinal Slope	0.005	ft/ft
n= Manning's Roughness Coefficient	0.016	
V <sub>s</sub> = Flow Velocity	1.84	fps
D= Flow Depth	0.41	ft
S <sub>x</sub> = Street Transverse Slope	0.02	ft/ft
W= Gutter Width	2	ft
a= Gutter Depression	5.0	in
L <sub>g</sub> = Length of Grate	7.5	ft
L <sub>c</sub> = Length of Curb Opening	9.5	ft
C <sub>fg</sub> = Clogging Factor for Grate	50	%
C <sub>fc</sub> = Clogging Factor for Curb Opening	50	%

### Grate Capacity Calculations:

R <sub>f</sub> = Frontal flow factor = $1-0.09*(V_s-V_o)$ if $V_s>V_o$ ; else $R_f=1$	1.00
V <sub>o</sub> = Splash over velocity= $p+q*(L_g*(1-(C_{fg}/100)))-r*(L_g*(1-(C_{fg}/100)))^2+s*(L_g*(1-(C_{fg}/100)))^3$	8.71 fps
E <sub>o</sub> = Grate flow ratio = $Q_w/Q_s$	0.28
Q <sub>w</sub> = Frontal flow= $A_w*V_s$	1.36 cfs
Q <sub>x</sub> = Side flow = $Q_s-Q_w$	3.54 cfs
R <sub>s</sub> = Side flow factor = $1/(1+(0.15*V_s^{1.8})/(S_x*(L_g*(1-C_{fg}/100))^{2.3}))$	0.48
Q <sub>ig</sub> = Flow intercepted = $[R_f*E_o+R_s*(1-E_o)]*Q_s$	3.1 cfs
†p=1.76;q=3.12;r=0.45;s=0.03 (based on Bar P 1-1/8 grate)	

### Curb Opening Capacity Calculations

S <sub>e</sub> = Equivalent cross slope = $S_x+S_w*E_o$	0.079 ft/ft
S <sub>w</sub> = Gutter cross slope = $(0.137+afeet)/W$	0.214 ft/ft
L <sub>t</sub> = Total interception L = $0.60*Q_s^{0.42}*S_o^{0.30}*(1/n*S_e)^{0.6}$	13.06 ft
Q <sub>ic</sub> = Flow intercepted = $(1-(1-(L_c*(1-C_{fc}/100))/L_t)^{1.8})*Q_s$	2.7 cfs

### Total Inlet Calculations

Q <sub>ti</sub> = Total flow intercepted = $Q_{ig}+Q_{ic}$	4.9 cfs
Q <sub>tq</sub> = Flow bypass = $Q_s-Q_{ti}$	0.0 cfs
q= Interception per unit ratio = $Q_{ti}/L_g$	0.52 cfs/ft
E <sub>q</sub> = Efficiency = $(Q_{ti}/Q_s)*100$	100 %

## Type CM Drop Inlet Sizing on a C. G.

DI#5

### Known:

Q <sub>s</sub> = Half Street Flow	5.5	cfs
S <sub>o</sub> = Longitudinal Slope	0.0064	ft/ft
n= Manning's Roughness Coefficient	0.016	
V <sub>s</sub> = Flow Velocity	2.07	fps
D= Flow Depth	0.41	ft
S <sub>x</sub> = Street Transverse Slope	0.02	ft/ft
W= Gutter Width	2	ft
a= Gutter Depression	5.0	in
L <sub>g</sub> = Length of Grate	5.0	ft
L <sub>c</sub> = Length of Curb Opening	7.0	ft
C <sub>fg</sub> = Clogging Factor for Grate	50	%
C <sub>fc</sub> = Clogging Factor for Curb Opening	50	%

### Grate Capacity Calculations:

R <sub>f</sub> = Frontal flow factor = $1-0.09*(V_s-V_o)$ if $V_s>V_o$ ; else $R_f=1$	1.00
V <sub>o</sub> = Splash over velocity= $p+q*(L_g*(1-(C_{fg}/100)))-r*(L_g*(1-(C_{fg}/100)))^2+s*(L_g*(1-(C_{fg}/100)))^3$	7.22 fps
E <sub>o</sub> = Grate flow ratio = $Q_w/Q_s$	0.28
Q <sub>w</sub> = Frontal flow= $A_w*V_s$	1.52 cfs
Q <sub>x</sub> = Side flow = $Q_s-Q_w$	3.98 cfs
R <sub>s</sub> = Side flow factor = $1/(1+(0.15*V_s^{1.8})/(S_x*(L_g*(1-C_{fg}/100))^{2.3}))$	0.23
Q <sub>ig</sub> = Flow intercepted = $[R_f*E_o+R_s*(1-E_o)]*Q_s$	2.4 cfs
†p=1.76;q=3.12;r=0.45;s=0.03 (based on Bar P 1-1/8 grate)	

### Curb Opening Capacity Calculations

S <sub>e</sub> = Equivalent cross slope = $S_x+S_w*E_o$	0.079 ft/ft
S <sub>w</sub> = Gutter cross slope = $(0.137+afeet)/W$	0.214 ft/ft
L <sub>t</sub> = Total interception L = $0.60*Q_s^{0.42}*S_o^{0.30}*(1/n*S_e)^{0.6}$	14.75 ft
Q <sub>ic</sub> = Flow intercepted = $(1-(1-(L_c*(1-C_{fc}/100))/L_t)^{1.8})*Q_s$	2.1 cfs

### Total Inlet Calculations

Q <sub>ti</sub> = Total flow intercepted = $Q_{ig}+Q_{ic}$	4.6 cfs
Q <sub>tq</sub> = Flow bypass = $Q_s-Q_{ti}$	0.9 cfs
q= Interception per unit ratio = $Q_{ti}/L_g$	0.65 cfs/ft
E <sub>q</sub> = Efficiency = $(Q_{ti}/Q_s)*100$	83 %

## Type CM Drop Inlet Sizing in Sump

DI#6

### Known:

$D_p$ = Ponding Depth at Inlet	0.42	ft
$W$ = Gutter Width	1.5	ft
$L_g$ = Length of Grate	7.5	ft
$L_c$ = Length of Curb Opening	9.5	ft
$H_c$ = Curb Opening Height	0.5	ft
$C_{wc}$ = Weir coefficient for curb opening	2.3	
$C_{wg}$ = Weir coefficient for grate	3.0	
$C_o$ = Orifice coefficient	0.67	
$R_o$ = Grate opening factor	0.60	
$C_{fg}$ = Clogging Factor for Grate	50	%
$C_{fc}$ = Clogging Factor for Curb Opening	50	%

### Grate Capacity Calculations:

$P_g$ = Active grate weir length (agwl) = $2*W+L_g$	10.50	ft
$P_{agl}$ Adjusted agwl = $2*(1-C_{fg}/100)*W+L_g$	9.00	ft
$A_g$ = Grate area = $W*L_g$	11.25	ft <sup>2</sup>
$A_{aga}$ = Adjusted open grate area = $A_g*R_o*(1-C_{fg}/100)$	3.38	ft <sup>2</sup>
check - If $D_p < 1.792*A_{aga}/P_{agl}$ ; weir; else orifice		weir
$Q_{ig}$ = Flow intercepted by grate If "weir" controls $Q_{ig}=C_{wg}*P_{agl}*D_p^{1.5}$ If "orifice" controls $Q_{ig}=C_o*A_{aga}*(64.4*D_p)^{0.5}$		7.3 cfs

### Curb Opening Capacity Calculations

$P_c$ = Active curb opening weir length (acowl) = $L_c+1.8*W$	12.20	ft
$P_{acl}$ Adjusted acowl = $L_c+1.8*(1-C_{fc}/100)*W$	10.85	ft
$A_c$ = curb opening area (coa) = $L_c*H_c$	4.75	ft <sup>2</sup>
$A_{aca}$ = Adjusted coa = $A_c*(1-C_{fc}/100)$	2.38	ft <sup>2</sup>
check - If $D_p < H_c$ ; weir; else orifice		weir
$Q_{ic}$ = Flow intercepted by curb opening If "weir" controls $Q_{ic}=C_{wc}*P_{acl}*D_p^{1.5}$ If "orifice" controls $Q_{ic}=C_o*A_{aca}*(64.4*D_p-H_c/2)^{0.5}$		6.7 cfs

### Total Inlet Calculations

$Q_{ti}$ = Total flow intercepted = $Q_{ig}+Q_{ic}$	14.0	cfs
$q$ = Interception per unit ratio = $Q_{ti}/L_g$	1.47	cfs/ft



Station	Invert Elev	Depth (FT)	Water Elev	Q (CFS)	Vel (FPS)	Vel Head	Energy Grd. El.	Super Elev	Critical Depth	Flow Top Width	Height/Dia. -FT	Base Wt or I.D.	ZL	No Wth Prs/Pip
L/Elem	Ch Slope					SF Ave	HF	SE Dpth	Froude N	Norm Dp	"N"	X-Fall	ZR	Type Ch
1000.000	2473.000	3.400	2476.400	38.50	7.84	.96	2477.36	.00	2.10	.00	2.500	.000	.00	0 .0
35.720	.0100					.0088	.31	3.40	.00	1.92	.013	.00	.00	PIPE
1035.720	2473.357	3.358	2476.715	38.50	7.84	.96	2477.67	.00	2.10	.00	2.500	.000	.00	0 .0
4.000	.0123					.0088	.04	3.36	.00	1.77	.013	.00	.00	PIPE
1039.720	2473.406	3.675	2477.081	38.50	7.84	.96	2478.04	.00	2.10	.00	2.500	.000	.00	0 .0
100.715	.0143					.0088	.89	3.68	.00	1.67	.013	.00	.00	PIPE
1140.435	2474.846	3.123	2477.969	38.50	7.84	.96	2478.92	.00	2.10	.00	2.500	.000	.00	0 .0
JUNCT STR	.0123					.0069	.03	.00	.00		.013	.00	.00	PIPE
1144.435	2474.895	3.906	2478.801	29.00	5.91	.54	2479.34	.00	1.84	.00	2.500	.000	.00	0 .0
56.404	.0100					.0050	.28	3.91	.00	1.55	.013	.00	.00	PIPE
1200.839	2475.459	3.624	2479.083	29.00	5.91	.54	2479.63	.00	1.84	.00	2.500	.000	.00	0 .0
JUNCT STR	.0128					.0031	.01	.00	.00		.013	.00	.00	PIPE
1204.839	2475.510	4.130	2479.640	14.00	4.46	.31	2479.95	.00	1.35	.00	2.000	.000	.00	0 .0
179.353	.0157					.0038	.68	4.13	.00	.99	.013	.00	.00	PIPE
1384.192	2478.327	2.000	2480.327	14.00	4.46	.31	2480.64	.00	1.35	.00	2.000	.000	.00	0 .0
6.632	.0157					.0035	.02	2.00	.00	.99	.013	.00	.00	PIPE
1390.825	2478.431	1.909	2480.340	14.00	4.53	.32	2480.66	.00	1.35	.83	2.000	.000	.00	0 .0

Station	Invert Elev	Depth (FT)	Water Elev	Q (CFS)	Vel (FPS)	Vel Head	Energy Grd. El.	Super Elev	Critical Depth	Flow Top Width	Height/Dia. -FT	Base Wt or I.D.	ZL	No Wth Prs/Pip
L/Elem	Ch Slope					SF Ave	HF	SE Dpth	Froude N	Norm Dp	"N"	X-Fall	ZR	Type Ch
1390.825	2478.431	.918	2479.349	14.00	9.95	1.54	2480.89	.00	1.35	1.99	2.000	.000	.00	0 .0
7.828	.0157					.0206	.16	.92	2.09	.99	.013	.00	.00	PIPE
1398.652	2478.554	.918	2479.472	14.00	9.95	1.54	2481.01	.00	1.35	1.99	2.000	.000	.00	0 .0
19.252	.0157					.0220	.42	.92	2.09	.99	.013	.00	.00	PIPE
1417.904	2478.856	.885	2479.741	14.00	10.43	1.69	2481.43	.00	1.35	1.99	2.000	.000	.00	0 .0
4.000	.0198					.0236	.09	.89	2.24	.93	.013	.00	.00	PIPE
1421.904	2478.935	.882	2479.817	14.00	10.49	1.71	2481.53	.00	1.35	1.99	2.000	.000	.00	0 .0
85.339	.0237					.0237	2.03	.88	2.26	.88	.013	.00	.00	PIPE
1507.243	2480.962	.882	2481.844	14.00	10.49	1.71	2483.55	.00	1.35	1.99	2.000	.000	.00	0 .0
85.119	.0237					.0246	2.10	.88	2.26	.88	.013	.00	.00	PIPE
1592.362	2482.983	.864	2483.848	14.00	10.77	1.80	2485.65	.00	1.35	1.98	2.000	.000	.00	0 .0
42.642	.0237					.0273	1.16	.86	2.34	.88	.013	.00	.00	PIPE
1635.004	2483.996	.834	2484.830	14.00	11.29	1.98	2486.81	.00	1.35	1.97	2.000	.000	.00	0 .0
5.000	.0298					.0290	.14	.83	2.51	.83	.013	.00	.00	PIPE
1640.004	2484.145	.834	2484.979	14.00	11.28	1.98	2486.95	.00	1.35	1.97	2.000	.000	.00	0 .0

MAIN OUT

9.341	.0360						.0281	.26	.83	2.51	.79	.013	.00	.00	PIPE
1649.345	2484.481	.849	2485.330	14.00	11.02	1.89	2487.22	.00	1.35	1.98	2.000	.000	.00	0	.0
13.383	.0360						.0255	.34	.85	2.42	.79	.013	.00	.00	PIPE

FILE: MAIN.WSW      W S P G W - CIVILDESIGN Version 14.06      PAGE 3  
 Program Package Serial Number: 1911      Date: 6-20-2024 Time: 2: 2:29  
 WATER SURFACE PROFILE LISTING

DURANGO AND GRAND MONTECITO  
 MAIN1  
 KHA JOB# 092935040 BY: SS/MS

Station	Invert Elev	Depth (FT)	Water Elev	Q (CFS)	Vel (FPS)	Vel Head	Energy Grd. El.	Super Elev	Critical Depth	Flow Top Width	Height/Dia. -FT	Base Wt or I.D.	ZL	No Wth Prs/Pip
L/Elem	Ch Slope					SF Ave	HF	SE Dpth	Froude N	Norm Dp	"N"	X-Fall	ZR	Type Ch
1662.728	2484.963	.880	2485.843	14.00	10.51	1.72	2487.56	.00	1.35	1.99	2.000	.000	.00	0 .0
9.080	.0360					.0224	.20	.88	2.26	.79	.013	.00	.00	PIPE
1671.807	2485.290	.913	2486.203	14.00	10.02	1.56	2487.76	.00	1.35	1.99	2.000	.000	.00	0 .0
6.606	.0360					.0197	.13	.91	2.11	.79	.013	.00	.00	PIPE
1678.414	2485.528	.947	2486.475	14.00	9.56	1.42	2487.89	.00	1.35	2.00	2.000	.000	.00	0 .0
4.995	.0360					.0174	.09	.95	1.97	.79	.013	.00	.00	PIPE
1683.409	2485.708	.983	2486.691	14.00	9.11	1.29	2487.98	.00	1.35	2.00	2.000	.000	.00	0 .0
3.843	.0360					.0153	.06	.98	1.83	.79	.013	.00	.00	PIPE
1687.252	2485.846	1.020	2486.866	14.00	8.69	1.17	2488.04	.00	1.35	2.00	2.000	.000	.00	0 .0
2.977	.0360					.0135	.04	1.02	1.71	.79	.013	.00	.00	PIPE
1690.229	2485.953	1.060	2487.013	14.00	8.28	1.07	2488.08	.00	1.35	2.00	2.000	.000	.00	0 .0
2.297	.0360					.0119	.03	1.06	1.59	.79	.013	.00	.00	PIPE
1692.525	2486.036	1.101	2487.137	14.00	7.90	.97	2488.11	.00	1.35	1.99	2.000	.000	.00	0 .0
1.740	.0360					.0105	.02	1.10	1.47	.79	.013	.00	.00	PIPE
1694.265	2486.099	1.145	2487.244	14.00	7.53	.88	2488.12	.00	1.35	1.98	2.000	.000	.00	0 .0
1.272	.0360					.0093	.01	1.14	1.37	.79	.013	.00	.00	PIPE
1695.538	2486.145	1.191	2487.335	14.00	7.18	.80	2488.14	.00	1.35	1.96	2.000	.000	.00	0 .0
.865	.0360					.0082	.01	1.19	1.27	.79	.013	.00	.00	PIPE

FILE: MAIN.WSW      W S P G W - CIVILDESIGN Version 14.06      PAGE 4  
 Program Package Serial Number: 1911      Date: 6-20-2024 Time: 2: 2:29  
 WATER SURFACE PROFILE LISTING

DURANGO AND GRAND MONTECITO  
 MAIN1  
 KHA JOB# 092935040 BY: SS/MS

Station	Invert Elev	Depth (FT)	Water Elev	Q (CFS)	Vel (FPS)	Vel Head	Energy Grd. El.	Super Elev	Critical Depth	Flow Top Width	Height/Dia. -FT	Base Wt or I.D.	ZL	No Wth Prs/Pip
L/Elem	Ch Slope					SF Ave	HF	SE Dpth	Froude N	Norm Dp	"N"	X-Fall	ZR	Type Ch
1696.402	2486.176	1.239	2487.416	14.00	6.85	.73	2488.14	.00	1.35	1.94	2.000	.000	.00	0 .0
.499	.0360					.0073	.00	1.24	1.18	.79	.013	.00	.00	PIPE
1696.901	2486.194	1.291	2487.485	14.00	6.53	.66	2488.15	.00	1.35	1.91	2.000	.000	.00	0 .0
.163	.0360					.0064	.00	1.29	1.09	.79	.013	.00	.00	PIPE
1697.065	2486.200	1.348	2487.547	14.00	6.22	.60	2488.15	.00	1.35	1.88	2.000	.000	.00	0 .0

MAIN\_LAT1.WSW

T1 DURANGO AND GRAND MONTECITO  
T2 MAIN\_LAT1  
T3 KHA JOB# 092935040 BY: SS/MS

0

SO	1000.0002475.478	30		2479.9					
R	1032.7722475.806	30	.013		.000	.000	.000	.000	0
R	1036.7722475.901	30	.013		.000	-64.300	.000	.000	1
R	1055.3232476.596	30	.013		.000	.000	.000	.000	0
SH	1055.3232476.596	30							
CD	6	4	0	.000	0.500	.000	.000	.000	.00
CD	8	4	0	.000	0.670	.000	.000	.000	.00
CD	12	4	0	.000	1.000	.000	.000	.000	.00
CD	18	4	0	.000	1.500	.000	.000	.000	.00
CD	24	4	0	.000	2.000	.000	.000	.000	.00
CD	30	4	0	.000	2.500	.000	.000	.000	.00
CD	36	4	0	.000	3.000	.000	.000	.000	.00
CD	42	4	0	.000	3.500	.000	.000	.000	.00
CD	48	4	0	.000	4.000	.000	.000	.000	.00
CD	54	4	0	.000	4.500	.000	.000	.000	.00
CD	60	4	0	.000	5.000	.000	.000	.000	.00
CD	66	4	0	.000	5.500	.000	.000	.000	.00
CD	72	4	0	.000	6.000	.000	.000	.000	.00
Q				15.000	.0				

Station	Invert Elev	Depth (FT)	Water Elev	Q (CFS)	Vel (FPS)	Vel Head	Energy Grd. El.	Super Elev	Critical Depth	Flow Top Width	Height/Dia. -FT	Base Wt or I.D.	ZL	No Wth Prs/Pip
L/Elem	Ch Slope					SF Ave	HF	SE Dpth	Froude N	Norm Dp	"N"	X-Fall	ZR	Type Ch
1000.000	2475.478	4.422	2479.900	15.00	3.06	.14	2480.04	.00	1.31	.00	2.500	.000	.00	0 .0
32.772	.0100					.0013	.04	4.42	.00	1.05	.013	.00	.00	PIPE
1032.772	2475.806	4.138	2479.944	15.00	3.06	.14	2480.09	.00	1.31	.00	2.500	.000	.00	0 .0
4.000	.0237					.0013	.01	4.14	.00	.83	.013	.00	.00	PIPE
1036.772	2475.901	4.086	2479.987	15.00	3.06	.14	2480.13	.00	1.31	.00	2.500	.000	.00	0 .0
18.551	.0375					.0013	.02	4.09	.00	.74	.013	.00	.00	PIPE
1055.323	2476.596	3.416	2480.012	15.00	3.06	.14	2480.16	.00	1.31	.00	2.500	.000	.00	0 .0

MAI N2. WSW

T1 DURANGO AND GRAND MONTECITO  
T2 MAI N2  
T3 KHA JOB# 092935040 BY: SS/MS  
SO 1000.0002474.874 24 2479.0  
R 1049.8202476.304 24 .013  
JX 1053.8202476.381 18 18 .013 4.9 2474.874 .000 .000 0  
R 1210.8722477.952 18 .013 .000 .000 0  
R 1214.8722477.994 18 .013 .000 61.400 1  
R 1233.9472478.186 18 .013 .000 .000 0  
SH 1233.9472478.186 18  
CD 6 4 0 .000 0.500 .000 .000 .000 .00  
CD 8 4 0 .000 0.670 .000 .000 .000 .00  
CD 12 4 0 .000 1.000 .000 .000 .000 .00  
CD 18 4 0 .000 1.500 .000 .000 .000 .00  
CD 24 4 0 .000 2.000 .000 .000 .000 .00  
CD 30 4 0 .000 2.500 .000 .000 .000 .00  
CD 36 4 0 .000 3.000 .000 .000 .000 .00  
CD 42 4 0 .000 3.500 .000 .000 .000 .00  
CD 48 4 0 .000 4.000 .000 .000 .000 .00  
CD 54 4 0 .000 4.500 .000 .000 .000 .00  
CD 60 4 0 .000 5.000 .000 .000 .000 .00  
CD 66 4 0 .000 5.500 .000 .000 .000 .00  
CD 72 4 0 .000 6.000 .000 .000 .000 .00  
Q 4.600 .0

DURANGO AND GRAND MONTECITO

MAIN2

KHA JOB# 092935040 BY: SS/MS

Station	Invert Elev	Depth (FT)	Water Elev	Q (CFS)	Vel (FPS)	Vel Head	Energy Grd. El.	Super Elev	Critical Depth	Flow Top Width	Height/Dia. -FT	Base Wt or I.D.	ZL	No Wth Prs/Pip
L/Elem	Ch Slope					SF Ave	HF	SE Dpth	Froude N	Norm Dp	"N"	X-Fall	ZR	Type Ch
1000.000	2474.874	4.126	2479.000	9.50	3.02	.14	2479.14	.00	1.10	.00	2.000	.000	.00	0 .0
49.820	.0287					.0018	.09	4.13	.00	.68	.013	.00	.00	PIPE
1049.820	2476.304	2.784	2479.088	9.50	3.02	.14	2479.23	.00	1.10	.00	2.000	.000	.00	0 .0
JUNCT STR	.0193					.0018	.01	2.78	.00		.013	.00	.00	PIPE
1053.820	2476.381	2.805	2479.186	4.60	2.60	.11	2479.29	.00	.82	.00	1.500	.000	.00	0 .0
157.052	.0100					.0019	.30	2.80	.00	.69	.013	.00	.00	PIPE
1210.872	2477.952	1.535	2479.487	4.60	2.60	.11	2479.59	.00	.82	.00	1.500	.000	.00	0 .0
4.000	.0105					.0019	.01	1.53	.00	.68	.013	.00	.00	PIPE
1214.872	2477.994	1.527	2479.521	4.60	2.60	.11	2479.63	.00	.82	.00	1.500	.000	.00	0 .0
3.332	.0101					.0019	.01	1.53	.00	.69	.013	.00	.00	PIPE
1218.204	2478.027	1.500	2479.527	4.60	2.60	.11	2479.63	.00	.82	.00	1.500	.000	.00	0 .0
15.377	.0101					.0018	.03	1.50	.00	.69	.013	.00	.00	PIPE
1233.581	2478.182	1.361	2479.543	4.60	2.73	.12	2479.66	.00	.82	.87	1.500	.000	.00	0 .0
.366	.0101					.0017	.00	1.36	.35	.69	.013	.00	.00	PIPE
1233.947	2478.186	1.358	2479.543	4.60	2.73	.12	2479.66	.00	.82	.88	1.500	.000	.00	0 .0

MAIN2\_LAT1.WSW

T1 DURANGO AND GRAND MONTECITO  
T2 MAIN2\_LAT1 0  
T3 KHA JOB# 092935040 BY: SS/MS  
S0 1000.0002476.362 18 2479.2  
R 1026.1752477.158 18 .013 .000 .000 0  
SH 1026.1752477.158 18  
CD 6 4 0 .000 0.500 .000 .000 .000 .00  
CD 8 4 0 .000 0.670 .000 .000 .000 .00  
CD 12 4 0 .000 1.000 .000 .000 .000 .00  
CD 18 4 0 .000 1.500 .000 .000 .000 .00  
CD 24 4 0 .000 2.000 .000 .000 .000 .00  
CD 30 4 0 .000 2.500 .000 .000 .000 .00  
CD 36 4 0 .000 3.000 .000 .000 .000 .00  
CD 42 4 0 .000 3.500 .000 .000 .000 .00  
CD 48 4 0 .000 4.000 .000 .000 .000 .00  
CD 54 4 0 .000 4.500 .000 .000 .000 .00  
CD 60 4 0 .000 5.000 .000 .000 .000 .00  
CD 66 4 0 .000 5.500 .000 .000 .000 .00  
CD 72 4 0 .000 6.000 .000 .000 .000 .00  
Q 4.900 .0

DURANGO AND GRAND MONTECITO

0

MAIN2\_LAT1

KHA JOB# 092935040 BY: SS/MS

Station	Invert Elev	Depth (FT)	Water Elev	Q (CFS)	Vel (FPS)	Vel Head	Energy Grd. El.	Super Elev	Critical Depth	Flow Top	Height/Dia. -FT	Base Wt or I.D.	ZL	No Wth Prs/Pip
L/Elem	Ch Slope					SF Ave	HF	SE Dpth	Froude N	Norm Dp	"N"	X-Fall	ZR	Type Ch
1000.000	2476.362	2.838	2479.200	4.90	2.77	.12	2479.32	.00	.85	.00	1.500	.000	.00	0 .0
26.175	.0304					.0022	.06	2.84	.00	.53	.013	.00	.00	PIPE
1026.175	2477.158	2.099	2479.257	4.90	2.77	.12	2479.38	.00	.85	.00	1.500	.000	.00	0 .0