

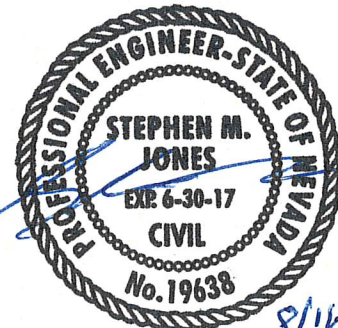
TECHNICAL DRAINAGE STUDY
UPDATE #1
(Sheep Mountain Parkway Design Considerations)

Skye Canyon
Detention Basin 1
City of Las Vegas, Nevada

Job No.: OLY1401.004
Date: August 2016

Prepared By:
Slater Hanifan Group, Inc.
5740 S. Arville Street, Suite 216
Las Vegas, Nevada 89118
Phone: 702-284-5300 • Fax: 702-284-5399

Prepared For:
Ninety-Five Management, LLC
11411 Southern Highlands Parkway, Suite 300
Las Vegas, NV 89141
Phone: 702-220-6565 • Fax: 702-220-6566



Steve Jones, P.E.
NV Professional Engineer No. 19638

HYDROLOGIC CRITERIA AND DRAINAGE DESIGN MANUAL
DRAINAGE STUDY INFORMATION FORM

Name of Development: Skye Canyon Detention Basin 1 (Update #1) Date: August 2016
Location of Development: a) Descriptive (Cross Streets): North/South: Puli Road
East/West: Grand Teton Drive
b) Section: 12 Township: 19 South Range: 59 East
7 19 South 60 East
c) APN: 126-12-000-001, 126-11-000-001, 125-07-602-005, 125-07-602-001

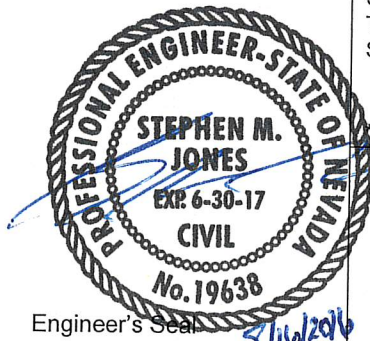
Name of Owner/Developer: Section 12, LLC & East of 95, LLC / Ninety-Five Management, LLC
Telephone No.: 702-220-6565 Fax No.: 702-220-6566 E-mail Address: mbolduc@olympiacompanies.com
Address: 11411 Southern Highlands Parkway, Suite 300; Las Vegas, NV 89141

Contact Person – Name: Steve Jones, P.E. Telephone No.: 702-284-5300
*E-mail Address: sjones@shg-inc.com Fax No.: 702-284-5399
Firm: Slater Hanifan Group
Address: 5740 S. Arville Street, Suite 216; Las Vegas, NV 89118

Type of Land Development/Land Disturbance Process:

☐ Rezoning	☐ Subdivision Map	☐ Clearing and grading Only
☐ Parcel Map	☐ Planned Unit Development	☒ Other (Please specify below)
☐ Large Parcel Map	☐ Building Permit	☐ Roadway & Storm Drain Infrastructure

1. Total Owned Land Area: At Site: 668.9 Acres Being Developed/Disturbed: 35 Acres +/-
2. Is a portion or all of the subject property located in a FEMA Flood Hazard Area? ☐ Yes** ☒ No
3. Is the property bordered or crossed by an existing or proposed Clark County Regional Flood Control District Master Planned Facility? ☒ Yes** ☐ No
4. Proposed type of development (Residential, Commercial, Etc.): Detention Basin
5. Approximate upstream land area which drains to the subject site: 0.9 sq. miles
6. Has the site drainage been evaluated in the past? ☒ Yes** ☐ No If yes, please identify documentation: Master Drainage Study Updates #1, 2 and 3 for Skye Canyon (SHG, DS4420)
Technical Drainage Study for Skye Canyon Detention Basin 1 (SHG, DS4698)
7. If known, please briefly identify the proposed discharge point(s) of runoff from the site: Existing Arroyo
8. Briefly describe your proposed schedule for the subject project: 1st of 2 planned detention basins at west boundary, per DS4420



Engineer's Seal

Submit this form as part of the required drainage study to the local entity which has jurisdiction over The subject property. This form may provide sufficient information to serve as the Conceptual Drainage Study.

*Newly Required Field

**Review and concurrence of the Clark County Regional Flood Control District is required

Local Entity File No.

REFERENCE: SHG Job No. OLY1401.004

STANDARD FORM 1

Updated 05/01/2008

OLY1401.004

August 16, 2016

Peter Jackson, CFM – Senior Engineering Associate
City of Las Vegas – Department of Public Works
Flood Control Section
333 N. Rancho Drive – 7th Floor
Las Vegas, Nevada 89106

**Subject: Update #1 to Technical Drainage Study for
Skye Canyon Detention Basin 1 – DS #4698
(Sheep Mountain Parkway Design Considerations)**

Dear Mr. Jackson:

This letter is submitted to serve as an update to the Technical Drainage Study for *Skye Canyon Detention Basin 1* to address the modified and additional grading in the future Sheep Mountain Parkway alignment that traverses in close to proximity to the previously approved design. In an effort to construct the ultimate condition embankment, emergency spillway and outlet pipe, the detention basin's design has been adjusted to include the fill portions of Sheep Mountain Parkway in the grading activity that will not change in the future. Additionally, the southern half of the collector channel is also being re-aligned to better plan for the future Sheep Mountain Parkway improvements (by others) near Grand Teton Drive and the developer's obligation to the City to construct a temporary roadway in the parkway alignment. Meeting notes detailing the concept of the updated design discussed between SHG, developer and City of Las Vegas (CLV) Public Works has been included in Appendix B.

Note that a grading permit has been obtained by the developer under the context of the previously approved design to begin excavation operations for the detention basin. Excavation is being performed to the toe of slope at the bottom of the basin that does not change between what had been previously approved and this updated design. However, because it is an early grading operation under the context that it is required for the completion of the overall detention basin, this project still shows that the entirety of the earthwork is being performed under the overall project's improvement plans. Meeting notes detailing the concept of the current grading permit discussed between SHG, developer and CLV Building Department has been included in Appendix B.

Summary of Sheep Mountain Parkway Grading:

Generally between the detention basin's emergency PMF spillway and the bounds of the southern embankment utilized for impoundment, the Sheep Mountain grading has been proposed in its ultimate manner following the *Proof of Concept Plans for Sheep Mountain Parkway* by Horrocks Engineers included in Appendix I. This ultimate condition grading includes the full width future roadway, trail, roadside swales and any other appurtenance grading within the right-of-way. South of the point where the on-ramp touches the future parkway, the grading within the Sheep Mountain reservation area slopes and connects into the grading required for the detention basin's collector channel to function. Future design of the

5740 S. Arville Street #216, Las Vegas, NV 89118 ph (702) 284-5300 fax (702) 284-5399

Grand Teton interchange by others will be required to consider the functionality of the collector channel, as discussed in the meeting with CLV Public Works.

In the alignment of the on-ramp, this project proposes grading for a future 32-ft roadway that is considered the developer's obligation for temporary traffic access through the Sheep Mountain reservation area. The ultimate elevations for the on-ramp has been considered for this area with the required grading for the roadway to reach existing ground at Grand Teton Drive, because it will be constructed prior to the interchange planned at Grand Teton. As a result of considering the grading for the future roadway, the alignment of the collector channel has been shifted further west within the existing Sheep Mountain BLM right-of-way grant as it approaches Grand Teton. Upon realignment of the collector channel, revised grading of the NV Energy access road has been performed west of the basin from Grand Teton.

The Sheep Mountain Parkway trail meanders and merges with the proposed collector channel's top bank access road in order to connect into Grand Teton to the south. The proposed trail at the top bank of the collector channel and its extension in its ultimate alignment within Sheep Mountain Parkway is intended to function for maintenance access for this project.

The grading proposed within Sheep Mountain Parkway by this project terminates just north of the emergency spillway and outlet pipe. The elevations of the parkway are similar to existing ground; therefore, no alterations occur to existing condition drainage patterns downstream.

Summary of Detention Basin Footprint Changes:

The toe of slope at the bottom of the detention basin remains the same as previously approved, along with the alignment and design of the collector channel confluence into the basin. The top of the basin's eastern slope from the PMF spillway south to the collector channel has been modified.

The fill required for Sheep Mountain allows for a reasonable elevation difference and moderate slope from the emergency spillway crest to the parkway grading. As a result, the emergency spillway no longer requires a roller compacted concrete (RCC) stepped structure to drop the PMF flow into the existing deep natural wash in this location. Further detail regarding the hydraulic criteria of the modified emergency spillway will follow in a later section of this report.

The proposed Sheep Mountain Parkway grading reduces the future developable space between the basin and the highway; therefore, the dam embankment south of the spillway has been adjusted from a soil cement lined 3:1 (H:V) slope to an earthen 4:1 (H:V) slope that meets City's minimum earthen slope standards. As a result, the volume of the detention basin changes and a new stage-storage relationship has been obtained that modifies the 100-year water surface elevation in the basin. Further detail regarding the modified stage-storage performance parameter for the basin will follow in a later section of this report.

Summary of Updated Hydrologic Models:

As detailed above, the volume has slightly changed in the detention basin due to the new 4:1 slope on the impounding embankment. The stage-storage output table from AutoCad Civil-3D has been included in Appendix C, and the HEC-1 Modified Puls routing elevation-area relationship has been updated accordingly in the proposed, ultimate and PMF condition HEC-1 models.

Additionally, after the previous approval, new soils data published by the NRCS became available. Therefore, the updated HEC-1 models utilize curve numbers that correspond to the new soils data. A custom soils report for this project's contributing watershed, published by the NRCS soils web-service, has been included in Appendix C.

As defined in the previous approved design, the ultimate condition watershed dictates the performance parameters for the detention basin. The following table summarizes the previous versus current parameters for the basin.

Summary of Detention Basin Performance Parameters			
Previous Approved Design		Current Updated Design	
Parameter	Value	Parameter	Value
100-Yr Inflow	794 cfs	100-Yr Inflow	818 cfs
100-Yr Outflow	71 cfs	100-Yr Outflow	71 cfs
Outlet Orifice	24-inch	Outlet Orifice	24-inch
Total Storage Volume	62.9 ac-ft	Total Storage Volume	65.9 ac-ft
100-Yr Volume	57 ac-ft	100-Yr Volume	60 ac-ft
Debris Volume	5.9 ac-ft	Debris Volume	5.9 ac-ft
100-Yr Water Surface	3034.9-ft	100-Yr Water Surface	3035.14-ft
PMF Inflow	6,744 cfs	PMF Inflow	6,786 cfs
PMF Outflow	5,989 cfs	PMF Outflow	6,025 cfs
PMF Spillway Crest Elevation	3037.0-ft	PMF Spillway Crest Elevation	3037.0-ft
PMF Spillway Length	87-ft	PMF Spillway Length	87-ft
PMF Water Surface Elevation	3044.93-ft	PMF Water Surface Elevation	3044.96-ft
Top of Detention Basin Elevation	3046.0-ft	Top of Detention Basin Elevation	3046.0-ft

The updated performance parameters are summarized and graphed on Sheet C-3 of the improvement plans, showing all minimum CCRFCD criteria are satisfied for regional detention basins.

Summary of Collector Channel Hydraulics:

As a result of the revised alignment, the WSPG hydraulic model for the collector channel has been updated accordingly. Hydraulic freeboard, per CCRFCD Manual criteria, and sedimentation freeboard remain provided in the channel with maintenance access provided at the flowline of the channel at the top of eastern bank of the channel. The ultimate condition drainage map, Exhibit F, has been updated with a new collector channel hydraulic summary to show all CCRFCD criteria are satisfied.

Note that the updated hydraulic model shows that the flow characteristics entering the stilling basin at the bottom of the detention basin remain the same as previously approved; therefore, the previous design of the stilling basin remains applicable.

Summary of Outlet Pipe Hydraulics:

Although the footprint at the top of the basin has changed, as described in this letter, the shape and functionality of the bottom of the detention basin remains as previously approved. Hence, the outlet structure and how the basin drains into the outlet pipe has not changed, the outlet

pipe remains a 60-inch RCP storm drain with a 24-inch diameter orifice plate necessary to control the outflow.

The scope of this project now performs grading in the natural wash that the previous design had as the outfall location of the basin's outlet pipe. The outlet pipe has been extended across the fill and now daylight into the same natural wash downstream of the fill activity for Sheep Mountain Parkway. A WSPG hydraulic model has been prepared from outlet of basin to the the outfall into the natural wash. A normal depth Flowmaster cross section in the existing wash downstream of the outfall has been prepared for the starting water surface elevation in the WSPG model. The water surface elevation at the system headworks shows the previous Flowmaster orifice calculation for the outlet pipe included in Appendix C that establishes the stage-discharge relationship in the HEC-1 model remains valid for this update.

The proposed trail for Sheep Mountain Parkway has been extended as Type II maintenance access to the pipe outfall with a turnaround at the headwall.

Additionally, the outlet pipe also provides stubs to be utilized for drainage within the Sheep Mountain Parkway graded area. A separate hydrologic and hydraulic analysis of the flows in this area has been included in Appendix G. A total of three (3) storm drain laterals have been provided in the following manner:

- A 36-inch RCP lateral at SDMH #3 allows for collection of the entirety of flows generated by Subbasins 'SM1' and 'SM2' by NDOT Type 2B drop inlets. Note that the ditches conveying the flow to the inlets dedicated to each Subbasin have capacity to contain the flow. The approach velocity in the SM1 ditch approaches 5 ft/s; however, the SM1 ditch is concrete lined as it merges with the PMF spillway to protect against erosion. The approach velocity in the SM2 ditch approaches 5 ft/s; however, it is considered temporary and only slightly above the erosion threshold until the 30-inch RCP stub provided in the parkway is utilized as described, or the ditch is landscaped typical of a parkway is utilized as described below.
- A 30-inch RCP lateral at SDMH #4 has been provided for future median drainage, as shown in the Sheep Mountain Parkway concept plans. It is proposed as capped until such a time that the Parkway roadway is constructed; therefore, reducing the amount of flow to the SM2 ditch.
- A 24-inch RCP lateral at SDMH #5 allows for collection of the entirety of flow generated by Subbasin 'SM3' by an NDOT Type 2B drop inlet. A 1-ft berm is provided to contain the flow from perpetuating north and allow for construction of an access manhole to the trunk 60-inch RCP outlet pipe at the approximate elevation of the Parkway without being in the flowline. The ditch conveys flow velocities less than 5 ft/s; therefore, erosion is not considered a concern.

Summary of Emergency Spillway Hydraulics:

The crest elevation for the emergency spillway remains 87-ft long at an elevation of 3037.0-ft for use by the PMF event, the same parameters as previously approved. Instead of an RCC stepped structure containing the PMF flow, a concrete lined slope at 3:1 (H:V) is proposed to contain the PMF flow as it exits the basin. The conveyance area for the PMF remains a consistent 87-ft in bottom width with concrete lining up to the depth of the flow. Downstream of the PMF crest, the concrete ends with a cut-off wall to protect the integrity of the embankment utilized for impoundment, and as the PMF overflow path approaches Sheep Mountain Parkway, the overflow begins to be directed north through the daylight grading for the parkway.

Inspection of the energy gradient of the incoming flow shows it is contained by the Parkway to turn the PMF north, with no adverse tailwater conditions occurring to the basin when the spillway is in use.

It is anticipated in the future upon completion of the parkway, the PMF will spill into the roadway itself. Future design considerations may be required to minimize adverse impacts to traffic, property and roadway improvements.

Drainage Easement Adjustments:

The eastern boundary of the public drainage easement for the detention basin has been modified in this update to encompass the following:

- Emergency PMF spillway from the detention basin crest to the location where proposed grading meets existing ground.
- The outlet pipe and its laterals in its entirety from detention basin to downstream outfall into the natural wash east of Sheep Mountain Parkway. The bounds of the proposed drainage easement meet minimum criteria for maintenance access of an underground facility.
- Similar to the previous easement, the boundary of the proposed drainage easement meets the existing BLM right-of-way grant N-7772. No easement is to be dedicated within the area reserved by the existing BLM right-of-way.
- The area for the maintenance staging of the detention basin has been re-located onto an appropriate area considering the new grading and is within the boundary of the new proposed drainage easement. The size remains approximately 1 acre in size which was the previous approved area size.

Conclusions:

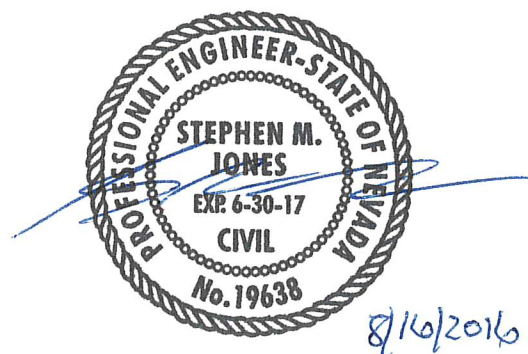
The detention basin design has been revised for inclusion of grading for the Sheep Mountain Parkway area to satisfy master developer obligations for the Skye Canyon community. The updated design allows for an ultimate condition impoundment, emergency spillway configuration and outlet pipe improvements in the vicinity of Sheep Mountain. After coordination with CLV Public Works, the updated design is considered acceptable. It is acknowledged that new approvals from CLV, CCRFCD and Nevada Dam Safety are required to construct the improvements presented in this update and on improvement plans included. If you have any questions or require additional information, please do not hesitate to contact us at 284-5300.

Respectfully Submitted,

SLATER HANIFAN GROUP

Steve Jones, P.E.
Project Manager - Flood Control Division

pc: Mark Failla, P.E., CFM – SHG
Jerry Slater, P.E. – SHG



APPENDIX

Appendix A – Previous Approval Letters

Appendix B – City of Las Vegas Correspondence Summary Notes

Appendix C – Hydrologic Data and Models

- New Soils Report
- Curve Numbers
- Standard Form 4 Lag Time Computations
- Stage-Storage Table from Civil-3D
- Outlet Pipe & Spillway Stage-Discharge Tables
- Proposed Condition HEC-1 Output
- Ultimate Condition HEC-1 Output
- PMF Condition HEC-1 Output

Appendix D – Collector Channel WSPG Hydraulic Analysis

Appendix E – Outlet Pipe WSPG Hydraulic Analysis

Appendix F – Emergency Spillway Normal Depth Computations

Appendix G – Sheep Mountain H&H Calculations

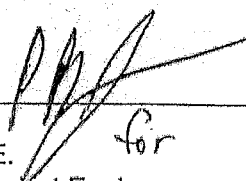
- Hydrology Data and HEC-1 Output
- Ditch Hydraulic Calculations
- Drop Inlet Calculations
- Storm Drain WSPG Hydraulic Analyses (Laterals to Trunk Outlet Pipe)

Appendix H – Updated Drainage Exhibits

Appendix I – Sheep Mountain Parkway Plans, by Horrocks Engineers

APPENDIX A

Previous Approval Letters

CITY OF LAS VEGAS			DATE:
INTER-OFFICE MEMORANDUM			June 1, 2016 
TO: Land Development Services Department of Building & Safety			FROM: Albert Sung, P.E. Flood Control Project Engineer Department of Public Works
SUBJECT: Drainage Study for: Skye Canyon – Detention Basin 1			COPIES TO: Slater Hanifan Group
Cross Streets:	NEC of Grand Teton Boulevard & Puli Road		Section 12 LLC/Ninety-Five Manag.
File Number:	F:\Depot\DSMemos\DS4698F.doc		Bart Anderson, P.E., DevCo
Parcel Number:	126-12-000-001		CCRFCFCD
Zoning Action:	Development Agreement		CCPW
FEMA Flood Zone	YES	NO X	NV Energy
Proposed Storm Drain	YES X	NO	Eddy Quaglieri, P.E., State of Nevada, Division of Water Resources - Dam Safety

HISTORY	DATE RECEIVED	DATE REVIEWED	COMMENTS	REVIEW FEES	FEES PAID Payment Trn #
1 st Submittal	9/16/2014	10/23/2014	Not Approved	\$400.00	370355: \$400
2 nd Submittal	12/24/2014	2/12/2015	Not Approved	\$400.00	380976: \$400
3 rd Submittal	5/7/2015 & 6/01/2015	6/24/2015	Not Approved	\$400.00	396675: \$400
4 th Submittal	10/28/201, 12/17/2015 & 1/11/2016	1/13/2016	Conditional Approval	\$400.00	420475: \$400
CCPW & CCRFCFCD	2/17/2016 & 3/1/2016	3/15/2016	Concurrence Recv'd	N/C	N/C
State of Nevada – DWR and NV Energy	5/16/2016 & 6/1/2016	6/1/2016	Approval	N/C	N/C
TOTAL FEES (LDDRS):				\$1,600.00	*****

REMARKS: Division of Water Resources and NV Energy approval has been provided.

REVISED - 4th Submittal is to address CCRFCFCD Comments, the added Brent Lane storm drain and updated information from State Dam Safety. Includes CCRFCFCD conditions of approval on Master Study – Update #2 dated January 6, 2016.

A Flood Control Master Plan Amendment (MPA) has been approved that covers the Skye Canyon Development. Flood Control understands that the Skye Canyon Master Plan is undergoing significant changes that will impact the adopted Master Plan Amendment. Any further changes to the identified Master Plan facilities shall require further review and the submittal of a new MPA for approval by the City of Las Vegas prior to submitting development plans outside of the shadow of protection from Detention Basin #1 and outside of the limits of the Phase 2 pods.

The Drainage Study for the subject project has been reviewed and:

X	is approved subject to conformance to all City standards and the following conditions:
	must be resubmitted or supplemented including the following:
	is conditionally approved subject to Clark County Regional Flood Control District concurrence.
	is conditionally approved subject to Clark County Public Works Department concurrence. Provide updated concurrence.
	is conditionally approved subject to State of Nevada, Division of Water Resources, Dam Safety concurrence.
	is conditionally approved subject to US Army Corps of Engineers, 404 Permit concurrence.

1. The **Skye Canyon Master Drainage Study** must be fully approved and shall include the location of the Ultimate Facility conveyance to the downstream MPU facilities prior to the substantial completion of the *Skye Canyon Detention Basin 1*.
2. No further Building Permits will be issued for the downstream residential developments within **Skye Canyon** beyond **Phase 1, Parcels 1.1 through 1.4**. The Building Permits will be released upon substantial completion of the *Skye Canyon Detention Basin 1* drainage facility improvements.
3. Flood Control has received the *State of Nevada, Division of Water Resources Dam Safety (DWR)* concurrence. The plans are in approvable form for permitting based upon the DWR approval. Project shall comply with all DWR requirements.
4. The connectivity of the outfall from Skye Canyon DB1 to a downstream Regional facility shall be addressed for the Ultimate Condition based upon the Overall Skye Canyon Master Plan. The City understands the concept of the proposed interim solution with the included storm drain plans for Brent Lane that collect and route the long term discharge from DB1 into the existing storm drain at Brent & Skye Canyon; however the overall Master Plan facility connectivity needs to be finalized with the overall **Skye Canyon Master Drainage Study** and approved by the City of Las Vegas and CCRFCD prior to the substantial completion of the DB1 improvements.
5. The Clark County Regional Flood Control District's concurrence of the Update #2 for Skye Canyon Master Drainage Study included conditions that shall require modifications to the interim outfall facility. The facility is required to be extended west to US 95 and incorporate a sediment basin. The proposed storm drain extension at Brent Lane west to US 95 to convey the 70 cfs discharge from DB1 is an acceptable concept. The Brent Lane storm drain extension has been included as a part of the Skye Canyon Detention Basin #1 Improvement Plan package and an updated estimate has been provided to include this storm drain and sediment basin as a part of the improvement bond.
6. The included Dam Failure Hazard Evaluation shall be expanded as a part of the required Emergency Action Plan (EAP) submittal. The flows used in Section 3 were shown as 2,989 versus the flow rate of 5,989. Review and revise accordingly. Flood Control recommends a review meeting to go over the EAP requirements and format prior to moving forward with the EAP process.
7. The subject **Skye Canyon Detention Basin #1** is a facility proposed in the approved MPA that covers the Skye Canyon Development. Future changes to the identified Master Plan facilities shall require further review and/or the submittal of an MPA based upon CCRFCD requirements.
8. *NV Energy's* concurrence and approval of the proposed drainage facilities for *Detention Basin 1* within their existing 48-foot easements has been provided.
9. A copy of the approved 404 Permit from the Army Corps of Engineers has been provided.
10. This proposed Detention Basin identifies a 10-foot buffer adjacent to the Red Rock Canyon National Conservation Area (RRCA), a critical area of no disturbance. No grading impacts are allowed to the RRCA and an appropriate buffer with block and cable fencing has been provided.
11. The current design has identified the limits of the Proposed Drainage Easement and Future Detention Basin Dedication as Flood Control Right-of-Way. A copy of the Recorded Drainage Easement necessary for the Detention Basin has been provided. The Detention Basin shall be dedicated to the City of Las Vegas as Drainage Right-of-Way prior to acceptance of the facility for City Maintenance.
12. The intent of the Master Developer is to construct the Detention Basins with associated collection facilities as CCRFCD Master Plan facilities and to then dedicate them to the City of Las Vegas. The Detention Basins shall be dedicated to the City as *Flood Control Right-of-Way*. The proposed dedication should exclude the City's existing BLM Grant N-77772 area. The total area of the

dedication shall be of sufficient size to meet the public operational needs and address expansion of the facility. The dedication needs to include the entire impact area of the dam construction as well as incorporate access and maintenance activities. The maintenance activities include staging for equipment and a temporary storage area for sediment removal.

13. The City understands that the limits of the Detention Basin Easement/Dedication may be reduced by a future Vacation request based upon future development grading impacts. The actual limits of any future vacation request shall be set based upon the City's approval of a Technical Drainage Study that addresses the impacts to the Detention Basin.
14. The westerly 76-feet of the site is encumbered with existing Nevada Power Grants and fiber optic Grants (N-57071, N-78122, and N-75351). This project proposes a 10-foot buffer from the Red Rock Canyon National Conservation Area boundary and provides for a new access road within the existing Power and fiber optic Grants.
15. The westerly access is being provided to service the existing Grants and is not the Flood Control access road. This area is however proposed to be included within the future dedication of the regional basin and therefore the responsibility for this maintenance by *NV Energy* and the owner of the fiber optic shall be provided.
16. The Detention Basin design includes the soil cement lining of the proposed 3:1 slopes in lieu of the City requested 4:1 slopes.
17. This project is considered as a *Capital Improvement Project (CIP)* with developer funding. Structural plans for the proposed storm drain improvements and pertinent flood control facilities have been submitted for Building Department review. Flood Control has received Structural review and approval from the Building Department.
18. All proposed improvements associated with *Detention Basin 1* shall be bonded and inspected. This project shall require Special Inspection. Coordinate the requirements of and the Agreements needed for Special Inspection with the Building Department.
19. The project proposes to build a temporary drainage facility. Prior to final plan approval the developer must complete a maintenance and liability agreement for the interim drainage improvements and post a minimum maintenance bond of \$100,000 or 50-percent of the construction cost for the improvements, whichever is greater. The engineer must submit an estimate of the quantities for constructing the facility and an 8.5-inch by 11-inch exhibit that adequately shows the location and limit of the drainage facility to Flood Control for approval. Once the drainage study is conditionally approved, the engineer should contact the *City's Land Development Section (229-6371)* to begin the agreement process.
20. Prior to the adoption and acceptance of the detention basin as a regional facility by the *City of Las Vegas, Department of Public Works*, the proposed *Detention Basin 1* shall remain a private facility owned and maintained by the master developer.
21. Provide the legal description with an exhibit of the proposed detention basin to *City Right-of-Way Section* for the dedication as Flood Control Right-of-Way to the *City of Las Vegas* after final acceptance of the basin by the *City of Las Vegas, Department of Public Works*.
22. *Streets & Sanitation Department's* approval of the DB1 improvements has been provided.
23. The detention basin will require the submittal and approval of an *Emergency Action Plan (EAP)* for this regional facility. The EAP format shall meet the City of Las Vegas submittal requirements for inclusion with our overall EAP. Contact Flood Control for sample format requirements. The EAP must be approved by the *City of Las Vegas* and then submitted and approved by the *State of Nevada, Division of Water Resources - Dam Safety* prior to the release of any improvement bonds for the

related detention basin improvements. The bonded improvements shall include a line item for the EAP in the amount of \$50,000.

24. The approved plans shall include the complete Specifications approved by DWR and the Building Department.

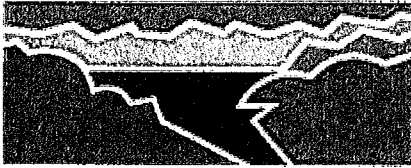
NOTE: Any future changes to the proposed design (or design assumptions) as outlined in the approved drainage study and attached preliminary grading plan which affect drainage must be addressed in a Drainage Study Update and accepted by the *City of Las Vegas Flood Control Section*. Additionally, conditional acceptance of a drainage study is valid for a period of one (1) year. If the proposed construction has not been completed in that time period, the *City of Las Vegas* reserves the right to require additional conditions and/or submission and acceptance of a complete drainage study update prior to further construction of a project.

NOTE: Please be advised that all land surface area disturbances over 1 acre or any area adjacent to a water way must submit to the *Nevada Division of Environmental Protection* a "Notice of Intent" to discharge that certifies a stormwater pollution prevention plan has been developed and is maintained on site; for inclusion in the Stormwater General Permit No. NVR100000. A phased construction unit in a contiguous subdivision is considered under construction until all stripped or disturbed surface areas have been covered by paving, building construction or planting. For more information, including forms and applications see <http://ndep.nv.gov/bwpc/storm01.htm> or call (775) 687-9429.

END OF REMARKS
AYS/PBJ

T/R/S: T19S/R59E/12
AREA F-12

REGIONAL FLOOD CONTROL DISTRICT



February 26, 2016

Steven C. Parrish, P.E.
General Manager/Chief
Engineer

BOARD OF DIRECTORS

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Mr. Oh Sang Kwon, P.E.
City of Las Vegas
333 North Rancho Drive
Las Vegas, NV 89106

**DISTRICT CONCURRENCE: SKYE CANYON - DETENTION BASIN 1
(DS4698D) (RFCD No. 15-9550)**

Dear Mr. Kwon:

Clark County Regional Flood Control District (District) reviewed Technical Drainage Study dated September 10, 2014, Addendum No. 1 dated December 24, 2014, Addendum No. 2 dated May 7, 2015, Addendum No. 3 dated October 28, 2015, Supplement No. 1 dated August 3, 2015, Supplement No. 2 dated December 17, 2015 and Supplement No. 3 dated February 4, 2016 for above-mentioned project as submitted by SLATER HANIFAN GROUP. In addition, District is in receipt of the Conditional Letter of Acceptance from City of Las Vegas dated February 8, 2016.

District concurs with acceptance of this Technical Drainage Study by City of Las Vegas with the following condition(s):

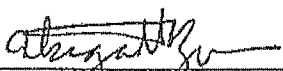
Per District's Uniform Regulations for the Control of Drainage, Section VIII.D.13, the Lead Entity shall provide As-Built plans or record drawings to the District after completion and final inspection of such privately installed flood control facilities that have regional flood control significance.

District's review of this project was limited to issues of Regional Flood Control Significance as defined in *Uniform Regulations for the Control of Drainage*.

Please be aware that as additional information becomes available and/or restudies of Flood Insurance Studies are performed, information submitted by SLATER HANIFAN GROUP may be superseded. Compliance with regulatory elements and design standards specified in *Uniform Regulations for the Control of Drainage* does not imply a guarantee that properties will be free from flooding or flood damage. The District, its officials, or employees assume no liability for information, data, or conclusions presented by consulting engineers. We, therefore, make no warranties, either expressed or implied, in conducting this review.

STEVEN C. PARRISH, P.E.
General Manager/Chief Engineer

BY:


Abigail Mayrena, P.E., CFM
Principal Civil Engineer

AM:fv

c: Steve Jones, P.E. Slater Hanifan Group

P:\Letters and Memos\Local Drainage\Land Development & Drainage\2015\L-15-9550asbuilt.doc

600 S. Grand Central Parkway, Suite 300 • Las Vegas, Nevada 89106-4511
(702) 685-0000 • FAX: (702) 685-0001
Website: <http://www.regionalflood.org>

BRIAN SANDOVAL
Governor

STATE OF NEVADA



LEO DROZDOFF
Director

JASON KING, P.E.
State Engineer

**DEPARTMENT OF CONSERVATION AND NATURAL RESOURCES
DIVISION OF WATER RESOURCES**

901 South Stewart Street, Suite 2002
Carson City, Nevada 89701-5250
(775) 684-2800 • Fax (775) 684-2811
<http://water.nv.gov>

May 9, 2016

Section 12, LLC
c/o Olympia Companies, LLC
11411 Southern Highlands Pkwy., Suite 300
Las Vegas, NV 89141

Re: Skye Canyon Detention Basin #1 Dam, J-716, NV10895

Dear Ladies and Gentlemen:

Please find enclosed approved application J-716 for the construction of Skye Canyon Detention Basin #1 Dam with supporting documents bearing the endorsement of the State Engineer. Please review attachment "A" of the authorization for due dates and "as-built" submittal items with particular attention to items (3) through (8). Please also note noticing requirements for the construction of the outlet and related structures outlined in item (13) as well.

You are responsible for period inspection of the embankment and impoundment during and after construction. Any proposed alterations to the plans and specifications as approved must be submitted to the State Engineer for review prior to initiating said change. Please report any slumps, sloughs, seeps or unusual observations to the State Engineer immediately.

Should you have any questions, please call the undersigned at (775) 684-2844.

Sincerely,

A handwritten signature in black ink, appearing to read "Eddy Quagliari".

Eddy Quagliari, P.E.
Manager II

EQ/ac
Enclosure

Cc: Steve Jones, P.E.
Peter Jackson, P.E., City of Las Vegas
Abigail Mayrena, P.E., Clark Co. Regional Flood Control District

ATTACHMENT "A"**Permit terms for the construction of Skye Canyon Detention Basin #1 Dam.**

- 1) If construction has not commenced within one (1) year of this approval, the permit will become void and a new application must be made prior to initiating construction.
- 2) A Nevada licensed civil engineer shall make periodic inspections during construction to ensure conformance to the approved plans and specifications.
- 3) Results of RCC test section must be conveyed to the State Engineer as soon as practical.
- 4) Should a design change be made, notice must be provided to the State Engineer sufficiently in advance of implementation and with adequate detail to allow review of the change.
- 5) Upon completion of construction but no later than May 9, 2017, the engineer shall submit to the State Engineer a Report of Completion of Work (Report). An Extension of Time is to be filed if Report is not filed by May 9, 2017.
- 6) The Report must include the statutory filing fee, signed verification from the engineer that the structure was completed in substantial conformance to the plans and specifications as approved, and a summary of the results of any gradation, earth compaction, concrete strength or other tests. Actions taken in response to any failed tests shall be described in the Report.
- 7) Report must be accompanied by a full-size "wet stamped" set of record drawings ("as-built"), and/or full sized "wet stamped" electronic as-built drawings, clearly delineating all deviations from the plans as approved.
- 8) An Emergency Action Plan for this dam must accompany the Report.
- 9) Upon satisfactory review of the Report, the State Engineer will issue an Approval to Impound. Water may not be impounded until an approval to impound has been issued.
- 10) Property improvements below or in the vicinity of the structure may alter its hazard classification. The owner is responsible for making any alterations that such a revision may necessitate.
- 11) This office, its officials or employees assume no liability for the information, data or conclusions presented by the consulting engineer. Therefore no warranty, either expressed or implied, has been made in conduct of this review.
- 12) This approval does not waive the necessity of obtaining other required permits from any applicable Federal, State or local agencies.

- 13) Please notify this office two weeks before construction begins on the primary outlet structure and 60" diameter outlet conduit.

STATE OF NEVADA
DEPARTMENT OF CONSERVATION AND NATURAL RESOURCES
DIVISION OF WATER RESOURCES

Filed _____

APPLICANT MUST NOT FILL IN ABOVE BLANKS

J. 716

NV 10895

**APPLICATION FOR APPROVAL OF THE PLANS AND SPECIFICATIONS FOR
THE CONSTRUCTION, RECONSTRUCTION OR ALTERATION OF A DAM**

This Application Involves in No Way the Right to Appropriate Water
To secure the right to appropriate water, application should be made to the State Engineer
on forms which will be furnished upon request.

I, Stephen M. Jones, P.E. of 5740 S. Arville Street, #216
Name of applicant Address
Las Vegas, NV 89118, hereby make application for the approval of

Plans and Specifications for the Construction of Skye Canyon Detention Basin 1 dam.
Construction, reconstruction, alteration Name of dam

The owner of the proposed dam is Section 12, LLC c/o Olympia Companies, LLC
Name of owner
of 11411 Southern Highlands Pkwy, Suite 300; Las Vegas State of Nevada 89141
Address

If the owner is a corporation, give name and address of president and secretary:

Garry V. Goett, Manager; Amanda Graham, Office Manager
11411 Southern Highlands Parkway, Suite 300; Las Vegas, NV 89141

The applicant is acting for the owner in the legal capacity of Agent
Agent, Lessee, Trustee, etc.

Location of Dam

1. The source of water to be stored is N/A which is a tributary of N/A,
and the proposed dam to be located within the SW 1/4, SW 1/4, Sec. 12
T. 19S, R. 59E E., M.D.B.&M. in Clark County, Nevada further described as
being at Latitude 36 deg, 18' N. Longitude 115 deg, 20' W. (at a point pursuant to NAC 535.210(6b).

Description and Dimensions of Dam

(If for an alteration, the data given below is for the altered dam)

2. Type of dam Earth 3. Length of crest 1,400 ft.
Concrete arch or gravity, earth, rockfill, etc.
4. Height stream bed to spillway crest 12 ft. 5. Height foundation to spillway crest 14 ft.
6. Freeboard 11 ft. 7. Thickness at top 16 ft. 8. Thickness at bottom 235 ft.
Spillway crest to top
9. Slope upstream* 4% 10. Slope downstream* 3% 11. Upstream facing Soil cement
*This information to be supplied for earth or rockfill dams. Concrete or rock paving, etc.
12. Amount of material in dam 16,780 cu. yds. 13. Estimated Cost \$5,322,805.00
14. Spillway data RCC Stepped Spillway, Design PMF 5,989 cfs
Type, capacity, etc.
15. Outlet data 60-inch RCP with 24-inch orifice plat, Design 100-yr Flow Rate 71 cfs
Type, capacity, etc.

16. Elevation of crest of dam 3046 above NAVD-88 datum
Approximate elevation to be given if true elevation not available
17. Area of reservoir at spillway level 7.46 acres. 18. Capacity of reservoir 72.5 ac. ft.

General Information

19. State the **purpose** of the dam 100-year flood storage, debris storage, flood control
Diversion only; storage only; storage and diversion; debris storage, flood control, etc.
20. State the **use** that is to be made of water none
municipal, domestic, irrigation, power, mining and milling, recreation, stockwatering or none

21. Engineers

Slater Hanifan Group
5740 S. Arville Street, #216
Las Vegas, NV 89118

Name and address of Engineers preparing plans

22. If the proposed dam is to be built under Federal supervision, state what department has jurisdiction.
N/A

23. The maps, plans and specifications accompanying this application are a part thereof.

[Signed]

this 6 day of August, 20 15

APPROVAL OF APPLICATION NO. _____, INCLUDING
PLANS AND SPECIFICATIONS

This Is to Certify That Application No. _____, including plans and specifications for the
_____ dam has

been examined and the same is hereby approved, subject to the following conditions:

RECEIVED

AUG 18 2015

STATE ENGINEER'S OFFICE

Revised 07/13 - dam_plans

FILE IN TRIPLICATE

Witness my hand and seal this _____ day

of _____, 20 _____

P.R.
State Engineer

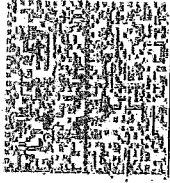
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STATE OF NEVADA
DIVISION OF WATER RESOURCES
901 S. Stewart Street, Suite 2002
Carson City, Nevada 89701



Return Service Requested

FIRST CLASS AUTO



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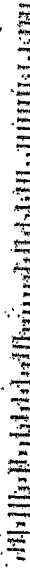
Mailed From 89701

US POSTAGE

MAY 12 2016

Steve Jones, P.E.
Slater Hanifan Group
5740 S. Arville Street #216
Las Vegas, NV 89118

PADJESB 89118



BRIAN SANDOVAL
Governor

STATE OF NEVADA



LEO DROZDOFF
Director

JASON KING, P.E.
State Engineer

DEPARTMENT OF CONSERVATION AND NATURAL RESOURCES
DIVISION OF WATER RESOURCES

901 South Stewart Street, Suite 2002

Carson City, Nevada 89701-5250

(775) 684-2800 • Fax (775) 684-2811

<http://water.nv.gov>

July 20, 2016

Peter Jackson, CFM
City of Las Vegas | Dept. of Public Works
City Engineering Division, Flood Control Section
Development Service Center (DSC) 7th Floor
333 North Rancho Drive | Las Vegas, Nevada 89106

Steve Jones, P.E.
Slater Hanifan Group
5740 S. Arville Street, #216
Las Vegas, NV 89118

Re: Skye Canyon Detention Basin #1 Dam, J-716, NV10895

To whom it may concern:

On July 6th, 2016, this office spoke with the City of Las Vegas concerning the Sky Canyon Detention Basin #1 (dam) and the developer's recent request to be granted a rough grading permit from the City of Las Vegas. A month ago, this office was contacted by Steve Jones P.E., the design engineer with Slater Hanifan Group, concerning a new roadway to be located on the down-stream slope of the embankment. Mr. Jones and I spoke about this major design change that was not a part of the original State Engineer approved plans and specifications under J-716. Therefore, there is still a need to submit new plans and specifications and obtain a new dam permit from this office due to the roadway placement.

This office is in agreement that a rough grading/excavating permit at the site is reasonable; however, the construction of any part of the embankment or any of the facilities in and around the embankment is not allowed due to the upcoming re-submittal of the design plans and specifications.

Please submit a monthly progress report, including inspection information and photos, to this office once grading has begun. Please also submit advance notification before construction begins on any and all parts of the embankment, spillway and water conveyance structures so that appropriate dam safety construction inspections can occur. Should you have any questions, please call the undersigned at (775) 684-2844.

Sincerely,

A handwritten signature in black ink, appearing to read "Eddy Quaglieri".

Eddy Quaglieri, P.E.
Manager II

EQ/PLO/ac

Cc: Steve Jones, P.E.
Peter Jackson, P.E., City of Las Vegas
Jason King, P.E. (email)
Kelvin Hickenbottom (email)
Luke Opperman, P.E. (email)
Abigail Mayrena, P.E., Clark Co. Regional Flood Control District



Department of Public Works

500 S. Grand Central Pky. • Box 554000 • Las Vegas NV 89155-4000
(702) 455-6000 • Fax (702) 455-6040

Denis Cederburg, P.E., Director • E-Mail: dlc@ClarkCountyNV.gov

Drainage Study Concurrence Letter

Date: February 17, 2016
Firm: SHG
Engineer: Steve Jones, P.E.
Address: 5740 S. Ariville Street, Suite 216, Las Vegas, NV 89118
Phone: (702) 284-5300
Fax: (702) 284-5399
E-mail: sjones@shg-inc.com

Subject: Technical Drainage Study Update for Skye Canyon - Detention Basin 1
Location: NEC of Grand Teton Drive and Puli Road
APN: 126-12-000-001
Sec, Township, Range: S12, T19S, R59E
Flood Zone & Panel: Zone X, 1750F, November 16, 2011
Application Tracking #: 16-6205UP

Submittals	Date Received	Date Returned	Reviewer	Miscellaneous
Concurrence	02/10/2016	02/17/2016	WG	original application 15-32969

The above-referenced project is located within the **City of Las Vegas** adjacent to Clark County unincorporated areas. The Drainage Study Update was submitted to the County for concurrence on February 10, 2016. In addition, the County has received the City of Las Vegas acceptance letter dated January 14, 2016 for the Technical Drainage Study (DS4698D).

The County concurs with the acceptance of this Drainage Study by the City of Las Vegas. Please note that the County's review of this development is limited to drainage issues significant to Clark County unincorporated areas.

BOARD OF COUNTY COMMISSIONERS
STEVE SISOLAK, Chairman • LARRY BROWN, Vice Chairman
SUSAN BRAGER • CHRIS GIUNCHIGLIANI • MARILYN KIRKPATRICK • MARY BETH SCOW • LAWRENCE WEEKLY
DONALD G. BURNETTE, County Manager

The County's review is solely based on the information submitted by the Engineer of Record. The County's review is strictly limited to compliance with minimum County Codes and Standards. The Engineer of Record is responsible for researching and addressing situations that may require designs above and beyond the minimum codes for issues of public safety and impacts to upstream, downstream and adjacent properties. The County assumes no liability for information data, designs or conclusions of the Engineer of Record.



Wade Gerstenkorn
Associate
Development Review
wade@ClarkCountyNV.gov



W. Layne Weber, P.E., CFM
Principal Engineer
Development Review
wlweber@ClarkCountyNV.gov

cc: Albert Sung, P.E., City of Las Vegas

APPENDIX B

City of Las Vegas Correspondence Summary Notes

MEETING NOTES

Project Number: OLY1401.004

Project Name: Skye Canyon Detention Basin 1 (DB1)

Meeting Date: 6/23/2016

Meeting Location: CLV Public Works 9th Floor – Gravel Pit Room

Meeting Topic: Sheep Mountain Parkway Grading Coordination with Approved DB1 Design

Attendees:	Company Name:
Oh Sang Kwon	CLV
Peter Jackson	CLV
David Bowers	CLV
Jeremy Leavitt	CLV
Mike Janssen	CLV
Bart Anderson	CLV
Greg McDermott	CLV
Jerry Slater	SHG
Steve Jones	SHG
Marc Bolduc	Olympia
Bobby Williamson	Olympia

Meeting Notes:

Item:	Discussion:
1	Overview – SHG and Olympia discussed that the improvement plans for Detention Basin 1 (DB1) were approved and only needed Olympia to post the bond to gain the permit and begin construction on the project. It was discussed that the current approved design tied into existing ground along all edges of grading, because the project started 2+ years ago before any agreements with regard to Sheep Mountain Parkway were made in the Development Agreement. Considering the obligations of the developer with Sheep Mountain Parkway, SHG and Olympia discussed that the best design moving forward would be to consider Sheep Mountain grading impacts to DB1, and revise the DB1 plans accordingly.
2	Engineering Impacts – SHG demonstrated that it would propose the Sheep Mountain grading and trail (per Horrocks grading design) in proximity of the emergency spillway and the southern embankment, resulting in a gradual fill of approx. 10-ft of elevation difference between top elevation of DB1 and the Parkway, rather than having an approx. 30-ft tall embankment to the natural washes like the current DB1 design shows, and a separate 20-ft to 30-ft fill for the Parkway in short proximity to the east. SHG will also adjust the affected embankment slope to an earthen 4:1 (H:V) design. South of the southern embankment, approaching the Grand Teton interchange area, SHG will grade the future access road/ramp from Grand Teton and meander the trail and DB1 collector channel to avoid any conflicts with NV Energy. CLV wants to make sure the trail upstream of the access road is adequate to convey the runoff from these newly filled areas. It was noted that the trail and collector channel crossing the Parkway right-of-way as it meanders south toward Grand Teton are temporary (similar to the current DB1 design) and a revised design may have to be considered when design begins on the Grand Teton interchange. All parties in attendance agreed that the concept of the revised design moving forward was more desire-able for functionality, dam risk, and maintenance. Exhibits from meeting to support the engineering discussion above have been emailed to all attendees listed above.
3	Processing Design Change – Olympia discussed that since CLV has conditioned that DB1 is required for the developers to receive permits to build homes in Phase 2 that DB1 construction needs to be started in fall to be able to have it completed within his current schedules for Phase 2. Olympia voiced concern regarding the timing of receiving approval on the updated design for DB1 fitting into his schedule. SHG noted that it spoke to Dam Safety and it is understood that a new Dam Safety permit would be required, but also noted that Dam Safety spoke to that excavation would be acceptable based on the current obtained Dam Safety Permit. However, in order to complete the required inspections, testing and proper certifications that placement of fill and construction of the embankment changing due to the updated design needs to be held up until the new approvals are obtained and CLV concurred

Prepared By: Steve Jones, SHG

Please notify the preparer if you have any questions, concerns or additions to these minutes within 2 days. After 2 days the content of this document will be considered accurate and accepted as the final recordation.

Item:	Discussion:
	upon Dam Safety opinion. It was discussed that an early grading permit, allowed for in the development agreement, based on the current approved design could help expedite the schedule while the changes to the design discussed above were under review. CLV agreed that Public Works would support the developer request to obtain an early grading permit since the shape of the interior basin remained the same and allow for the duration of excavation required in the basin and items expected to remain unchanged with the updated design; however, Olympia is required to consult with Chris Knight and Land Development staff to procedurally ensure it has all items required to obtain the early grading permit.

Action Items:

Item:	Discussion:
	SHG will provide an exhibit or plans upon request for an early grading permit that demonstrate areas that remain the same as the current approved design to be completed with the early grading permit and areas subject to change based on updated design that will be withheld until review of the new design is complete.
	SHG will setup meeting with Land Development to discuss the scope of the early grading permit. It was suggested that a representative from Public Works (likely Bart Anderson for his familiarity with the Development Agreement) be also invited to the meeting as support for SHG and the developer's request.

Prepared By: Steve Jones, SHG

Please notify the preparer if you have any questions, concerns or additions to these minutes within 2 days. After 2 days the content of this document will be considered accurate and accepted as the final recordation.

Item:	Discussion:
	and all parties agreed.

Action Items:

Item:	Discussion:
	Early grading through a stockpile permit will be obtained prior to submittal of the TDS Update for the formal design change affected by Sheep Mountain Parkway.
	SHG will prepare and submit a TDS Update for the improvement plan changes affected by Sheep Mountain Parkway.

Prepared By: Steve Jones, SHG

Please notify the preparer if you have any questions, concerns or additions to these minutes within 2 days. After 2 days the content of this document will be considered accurate and accepted as the final recordation.

APPENDIX C

Hydrologic Data & Models

New Soils
Report



United States
Department of
Agriculture

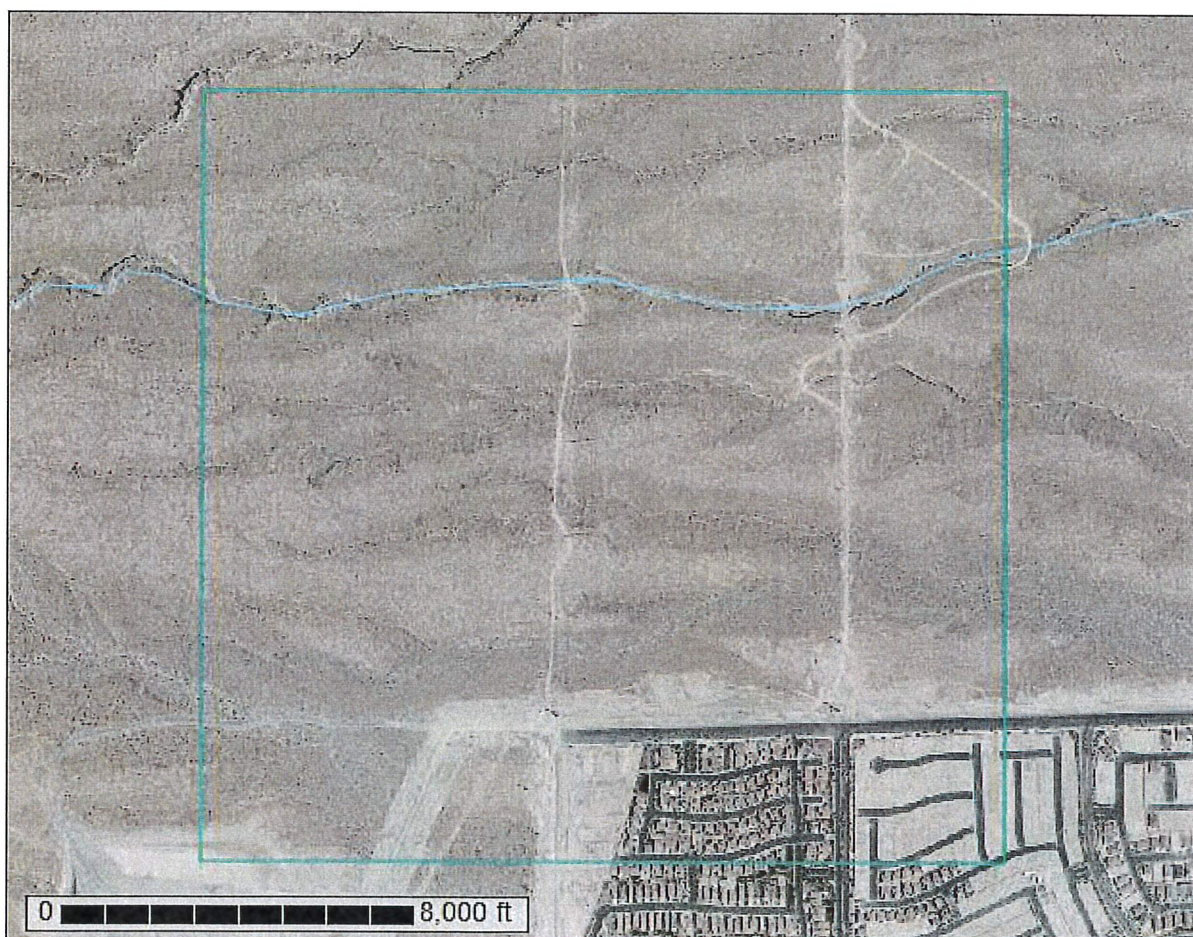
NRCS

Natural
Resources
Conservation
Service

A product of the National
Cooperative Soil Survey,
a joint effort of the United
States Department of
Agriculture and other
Federal agencies, State
agencies including the
Agricultural Experiment
Stations, and local
participants

Custom Soil Resource Report for Las Vegas Valley Area, Nevada, Part of Clark County

Skye Canyon Detention Basin 1



August 9, 2016

Preface

Soil surveys contain information that affects land use planning in survey areas. They highlight soil limitations that affect various land uses and provide information about the properties of the soils in the survey areas. Soil surveys are designed for many different users, including farmers, ranchers, foresters, agronomists, urban planners, community officials, engineers, developers, builders, and home buyers. Also, conservationists, teachers, students, and specialists in recreation, waste disposal, and pollution control can use the surveys to help them understand, protect, or enhance the environment.

Various land use regulations of Federal, State, and local governments may impose special restrictions on land use or land treatment. Soil surveys identify soil properties that are used in making various land use or land treatment decisions. The information is intended to help the land users identify and reduce the effects of soil limitations on various land uses. The landowner or user is responsible for identifying and complying with existing laws and regulations.

Although soil survey information can be used for general farm, local, and wider area planning, onsite investigation is needed to supplement this information in some cases. Examples include soil quality assessments (<http://www.nrcs.usda.gov/wps/portal/nrcs/main/soils/health/>) and certain conservation and engineering applications. For more detailed information, contact your local USDA Service Center (<http://offices.sc.egov.usda.gov/locator/app?agency=nrcs>) or your NRCS State Soil Scientist (http://www.nrcs.usda.gov/wps/portal/nrcs/detail/soils/contactus/?cid=nrcs142p2_053951).

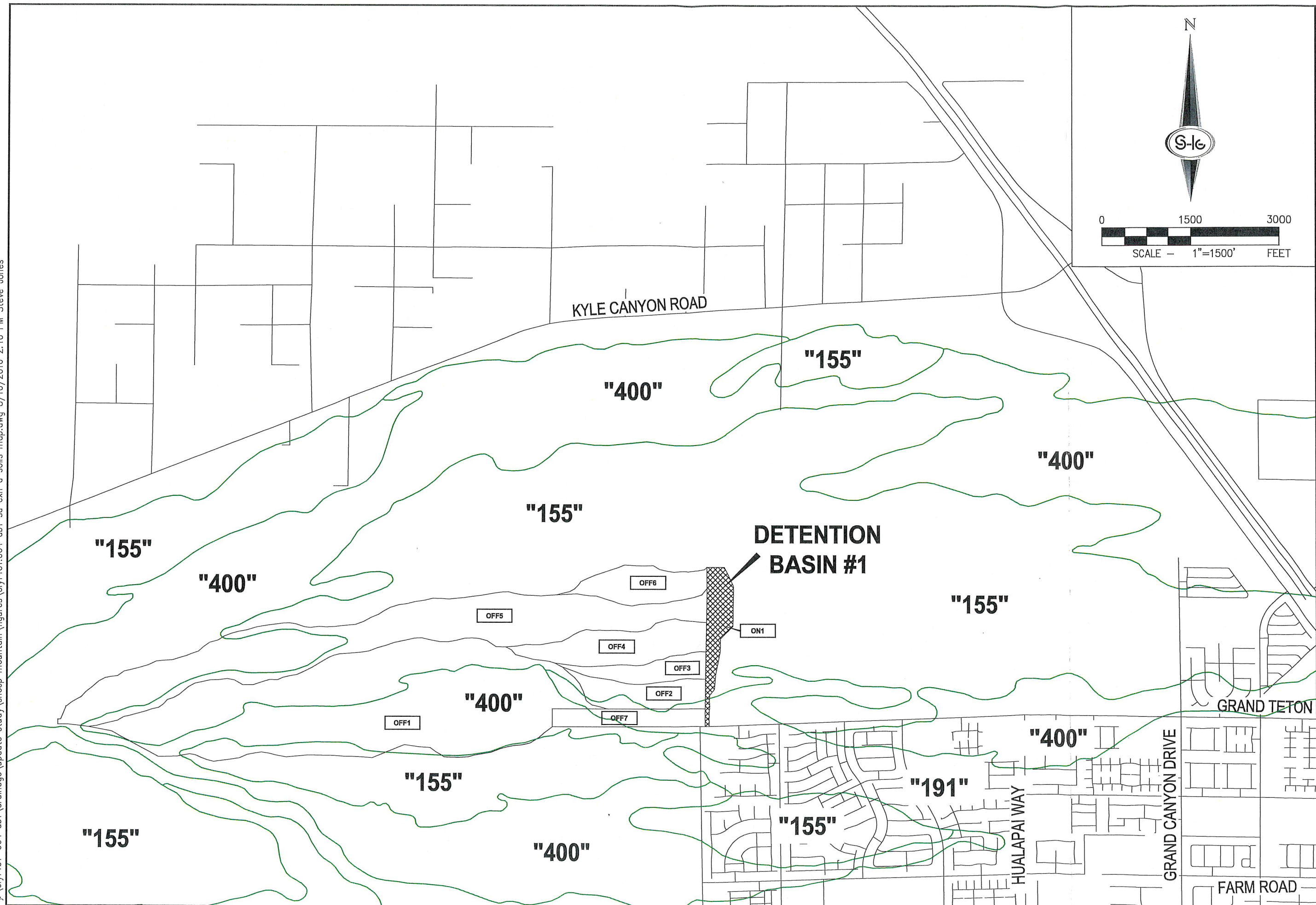
Great differences in soil properties can occur within short distances. Some soils are seasonally wet or subject to flooding. Some are too unstable to be used as a foundation for buildings or roads. Clayey or wet soils are poorly suited to use as septic tank absorption fields. A high water table makes a soil poorly suited to basements or underground installations.

The National Cooperative Soil Survey is a joint effort of the United States Department of Agriculture and other Federal agencies, State agencies including the Agricultural Experiment Stations, and local agencies. The Natural Resources Conservation Service (NRCS) has leadership for the Federal part of the National Cooperative Soil Survey.

Information about soils is updated periodically. Updated information is available through the NRCS Web Soil Survey, the site for official soil survey information.

The U.S. Department of Agriculture (USDA) prohibits discrimination in all its programs and activities on the basis of race, color, national origin, age, disability, and where applicable, sex, marital status, familial status, parental status, religion, sexual orientation, genetic information, political beliefs, reprisal, or because all or a part of an individual's income is derived from any public assistance program. (Not all prohibited bases apply to all programs.) Persons with disabilities who require alternative means

j:\poly\401-004 db1\drainage\update study\sheep mountain\figures\oly\401.004 db1 sa exh d soils map.dwg 8/10/2016 2:10 PM Steve Jones



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How Soil Surveys Are Made

Soil surveys are made to provide information about the soils and miscellaneous areas in a specific area. They include a description of the soils and miscellaneous areas and their location on the landscape and tables that show soil properties and limitations affecting various uses. Soil scientists observed the steepness, length, and shape of the slopes; the general pattern of drainage; the kinds of crops and native plants; and the kinds of bedrock. They observed and described many soil profiles. A soil profile is the sequence of natural layers, or horizons, in a soil. The profile extends from the surface down into the unconsolidated material in which the soil formed or from the surface down to bedrock. The unconsolidated material is devoid of roots and other living organisms and has not been changed by other biological activity.

Currently, soils are mapped according to the boundaries of major land resource areas (MLRAs). MLRAs are geographically associated land resource units that share common characteristics related to physiography, geology, climate, water resources, soils, biological resources, and land uses (USDA, 2006). Soil survey areas typically consist of parts of one or more MLRA.

The soils and miscellaneous areas in a survey area occur in an orderly pattern that is related to the geology, landforms, relief, climate, and natural vegetation of the area. Each kind of soil and miscellaneous area is associated with a particular kind of landform or with a segment of the landform. By observing the soils and miscellaneous areas in the survey area and relating their position to specific segments of the landform, a soil scientist develops a concept, or model, of how they were formed. Thus, during mapping, this model enables the soil scientist to predict with a considerable degree of accuracy the kind of soil or miscellaneous area at a specific location on the landscape.

Commonly, individual soils on the landscape merge into one another as their characteristics gradually change. To construct an accurate soil map, however, soil scientists must determine the boundaries between the soils. They can observe only a limited number of soil profiles. Nevertheless, these observations, supplemented by an understanding of the soil-vegetation-landscape relationship, are sufficient to verify predictions of the kinds of soil in an area and to determine the boundaries.

Soil scientists recorded the characteristics of the soil profiles that they studied. They noted soil color, texture, size and shape of soil aggregates, kind and amount of rock fragments, distribution of plant roots, reaction, and other features that enable them to identify soils. After describing the soils in the survey area and determining their properties, the soil scientists assigned the soils to taxonomic classes (units). Taxonomic classes are concepts. Each taxonomic class has a set of soil characteristics with precisely defined limits. The classes are used as a basis for comparison to classify soils systematically. Soil taxonomy, the system of taxonomic classification used in the United States, is based mainly on the kind and character of soil properties and the arrangement of horizons within the profile. After the soil scientists classified and named the soils in the survey area, they compared the

Custom Soil Resource Report

individual soils with similar soils in the same taxonomic class in other areas so that they could confirm data and assemble additional data based on experience and research.

The objective of soil mapping is not to delineate pure map unit components; the objective is to separate the landscape into landforms or landform segments that have similar use and management requirements. Each map unit is defined by a unique combination of soil components and/or miscellaneous areas in predictable proportions. Some components may be highly contrasting to the other components of the map unit. The presence of minor components in a map unit in no way diminishes the usefulness or accuracy of the data. The delineation of such landforms and landform segments on the map provides sufficient information for the development of resource plans. If intensive use of small areas is planned, onsite investigation is needed to define and locate the soils and miscellaneous areas.

Soil scientists make many field observations in the process of producing a soil map. The frequency of observation is dependent upon several factors, including scale of mapping, intensity of mapping, design of map units, complexity of the landscape, and experience of the soil scientist. Observations are made to test and refine the soil-landscape model and predictions and to verify the classification of the soils at specific locations. Once the soil-landscape model is refined, a significantly smaller number of measurements of individual soil properties are made and recorded. These measurements may include field measurements, such as those for color, depth to bedrock, and texture, and laboratory measurements, such as those for content of sand, silt, clay, salt, and other components. Properties of each soil typically vary from one point to another across the landscape.

Observations for map unit components are aggregated to develop ranges of characteristics for the components. The aggregated values are presented. Direct measurements do not exist for every property presented for every map unit component. Values for some properties are estimated from combinations of other properties.

While a soil survey is in progress, samples of some of the soils in the area generally are collected for laboratory analyses and for engineering tests. Soil scientists interpret the data from these analyses and tests as well as the field-observed characteristics and the soil properties to determine the expected behavior of the soils under different uses. Interpretations for all of the soils are field tested through observation of the soils in different uses and under different levels of management. Some interpretations are modified to fit local conditions, and some new interpretations are developed to meet local needs. Data are assembled from other sources, such as research information, production records, and field experience of specialists. For example, data on crop yields under defined levels of management are assembled from farm records and from field or plot experiments on the same kinds of soil.

Predictions about soil behavior are based not only on soil properties but also on such variables as climate and biological activity. Soil conditions are predictable over long periods of time, but they are not predictable from year to year. For example, soil scientists can predict with a fairly high degree of accuracy that a given soil will have a high water table within certain depths in most years, but they cannot predict that a high water table will always be at a specific level in the soil on a specific date.

After soil scientists located and identified the significant natural bodies of soil in the survey area, they drew the boundaries of these bodies on aerial photographs and identified each as a specific map unit. Aerial photographs show trees, buildings, fields, roads, and rivers, all of which help in locating boundaries accurately.

Soil Map

The soil map section includes the soil map for the defined area of interest, a list of soil map units on the map and extent of each map unit, and cartographic symbols displayed on the map. Also presented are various metadata about data used to produce the map, and a description of each soil map unit.

Custom Soil Resource Report Soil Map



MAP LEGEND

 Area of Interest (AOI)	 Area of Interest (AOI)	 Spoil Area
 Soils	 Soil Map Unit Polygons	 Stony Spot
 Soil Map Unit Lines	 Soil Map Unit Lines	 Very Stony Spot
 Soil Map Unit Points	 Soil Map Unit Points	 Wet Spot
 Special Point Features	 Special Line Features	 Other
 Blowout	 Streams and Canals	 Special Line Features
 Borrow Pit	 Transportation	 RAILS
 Clay Spot	 Interstate Highways	 US Routes
 Closed Depression	 Major Roads	 Local Roads
 Gravel Pit	 Background	 Aerial Photography
 Gravelly Spot		
 Landfill		
 Lava Flow		
 Marsh or swamp		
 Mine or Quarry		
 Miscellaneous Water		
 Perennial Water		
 Rock Outcrop		
 Saline Spot		
 Sandy Spot		
Severely Eroded Spot		
Sinkhole		
Slide or Slip		
Sodic Spot		

MAP INFORMATION

The soil surveys that comprise your AOI were mapped at 1:24,000.

Warning: Soil Map may not be valid at this scale.

Enlargement of maps beyond the scale of mapping can cause misunderstanding of the detail of mapping and accuracy of soil line placement. The maps do not show the small areas of contrasting soils that could have been shown at a more detailed scale.

Please rely on the bar scale on each map sheet for map measurements.

Source of Map: Natural Resources Conservation Service
Web Soil Survey URL: <http://websoilsurvey.nrcs.usda.gov>
Coordinate System: Web Mercator (EPSG:3857)

Maps from the Web Soil Survey are based on the Web Mercator projection, which preserves direction and shape but distorts distance and area. A projection that preserves area, such as the Albers equal-area conic projection, should be used if more accurate calculations of distance or area are required.

This product is generated from the USDA-NRCS certified data as of the version date(s) listed below.

Soil Survey Area: Las Vegas Valley Area, Nevada, Part of Clark County
Survey Area Data: Version 10, Aug 22, 2014

Soil map units are labeled (as space allows) for map scales 1:50,000 or larger.

Date(s) aerial images were photographed: May 7, 2011—Jun 2, 2011

The orthophoto or other base map on which the soil lines were compiled and digitized probably differs from the background imagery displayed on these maps. As a result, some minor shifting of map unit boundaries may be evident.

Map Unit Legend

Las Vegas Valley Area, Nevada, Part of Clark County (NV788)			
Map Unit Symbol	Map Unit Name	Acres in AOI	Percent of AOI
155	Cave gravelly fine sandy loam, 4 to 15 percent slopes	219.3	76.7%
191	Dalian very cobbly fine sandy loam, 2 to 8 percent slopes	60.5	21.2%
400	Tencee very gravelly fine sandy loam, 2 to 8 percent slopes	6.2	2.2%
Totals for Area of Interest		286.0	100.0%

Map Unit Descriptions

The map units delineated on the detailed soil maps in a soil survey represent the soils or miscellaneous areas in the survey area. The map unit descriptions, along with the maps, can be used to determine the composition and properties of a unit.

A map unit delineation on a soil map represents an area dominated by one or more major kinds of soil or miscellaneous areas. A map unit is identified and named according to the taxonomic classification of the dominant soils. Within a taxonomic class there are precisely defined limits for the properties of the soils. On the landscape, however, the soils are natural phenomena, and they have the characteristic variability of all natural phenomena. Thus, the range of some observed properties may extend beyond the limits defined for a taxonomic class. Areas of soils of a single taxonomic class rarely, if ever, can be mapped without including areas of other taxonomic classes. Consequently, every map unit is made up of the soils or miscellaneous areas for which it is named and some minor components that belong to taxonomic classes other than those of the major soils.

Most minor soils have properties similar to those of the dominant soil or soils in the map unit, and thus they do not affect use and management. These are called noncontrasting, or similar, components. They may or may not be mentioned in a particular map unit description. Other minor components, however, have properties and behavioral characteristics divergent enough to affect use or to require different management. These are called contrasting, or dissimilar, components. They generally are in small areas and could not be mapped separately because of the scale used. Some small areas of strongly contrasting soils or miscellaneous areas are identified by a special symbol on the maps. If included in the database for a given area, the contrasting minor components are identified in the map unit descriptions along with some characteristics of each. A few areas of minor components may not have been observed, and consequently they are not mentioned in the descriptions, especially where the pattern was so complex that it was impractical to make enough observations to identify all the soils and miscellaneous areas on the landscape.

The presence of minor components in a map unit in no way diminishes the usefulness or accuracy of the data. The objective of mapping is not to delineate pure taxonomic classes but rather to separate the landscape into landforms or landform segments that have similar use and management requirements. The delineation of such segments

Custom Soil Resource Report

on the map provides sufficient information for the development of resource plans. If intensive use of small areas is planned, however, onsite investigation is needed to define and locate the soils and miscellaneous areas.

An identifying symbol precedes the map unit name in the map unit descriptions. Each description includes general facts about the unit and gives important soil properties and qualities.

Soils that have profiles that are almost alike make up a *soil series*. Except for differences in texture of the surface layer, all the soils of a series have major horizons that are similar in composition, thickness, and arrangement.

Soils of one series can differ in texture of the surface layer, slope, stoniness, salinity, degree of erosion, and other characteristics that affect their use. On the basis of such differences, a soil series is divided into *soil phases*. Most of the areas shown on the detailed soil maps are phases of soil series. The name of a soil phase commonly indicates a feature that affects use or management. For example, Alpha silt loam, 0 to 2 percent slopes, is a phase of the Alpha series.

Some map units are made up of two or more major soils or miscellaneous areas. These map units are complexes, associations, or undifferentiated groups.

A *complex* consists of two or more soils or miscellaneous areas in such an intricate pattern or in such small areas that they cannot be shown separately on the maps. The pattern and proportion of the soils or miscellaneous areas are somewhat similar in all areas. Alpha-Beta complex, 0 to 6 percent slopes, is an example.

An *association* is made up of two or more geographically associated soils or miscellaneous areas that are shown as one unit on the maps. Because of present or anticipated uses of the map units in the survey area, it was not considered practical or necessary to map the soils or miscellaneous areas separately. The pattern and relative proportion of the soils or miscellaneous areas are somewhat similar. Alpha-Beta association, 0 to 2 percent slopes, is an example.

An *undifferentiated group* is made up of two or more soils or miscellaneous areas that could be mapped individually but are mapped as one unit because similar interpretations can be made for use and management. The pattern and proportion of the soils or miscellaneous areas in a mapped area are not uniform. An area can be made up of only one of the major soils or miscellaneous areas, or it can be made up of all of them. Alpha and Beta soils, 0 to 2 percent slopes, is an example.

Some surveys include *miscellaneous areas*. Such areas have little or no soil material and support little or no vegetation. Rock outcrop is an example.

Las Vegas Valley Area, Nevada, Part of Clark County

155—Cave gravelly fine sandy loam, 4 to 15 percent slopes

Map Unit Setting

National map unit symbol: hr9w
Elevation: 2,000 to 4,800 feet
Mean annual precipitation: 4 to 12 inches
Mean annual air temperature: 57 to 70 degrees F
Frost-free period: 180 to 280 days
Farmland classification: Not prime farmland

Map Unit Composition

Cave and similar soils: 100 percent
Estimates are based on observations, descriptions, and transects of the mapunit.

Description of Cave

Setting

Landform: Fan remnants
Down-slope shape: Linear
Across-slope shape: Convex
Parent material: Mixed alluvium

Typical profile

H1 - 0 to 15 inches: gravelly fine sandy loam
H2 - 15 to 60 inches: indurated

Properties and qualities

Slope: 4 to 15 percent
Depth to restrictive feature: 4 to 20 inches to petrocalcic
Natural drainage class: Well drained
Runoff class: Very high
Capacity of the most limiting layer to transmit water (Ksat): Very low (0.00 to 0.00 in/hr)
Depth to water table: More than 80 inches
Frequency of flooding: None
Frequency of ponding: None
Calcium carbonate, maximum in profile: 40 percent
Salinity, maximum in profile: Nonsaline to slightly saline (0.0 to 4.0 mmhos/cm)
Available water storage in profile: Very low (about 1.5 inches)

Interpretive groups

Land capability classification (irrigated): None specified
Land capability classification (nonirrigated): 7s
Hydrologic Soil Group: D
Other vegetative classification: LIMY 3-5" P.Z. (030XB019NV_3)

191—Dalian very cobbly fine sandy loam, 2 to 8 percent slopes

Map Unit Setting

National map unit symbol: hrb4
Elevation: 2,200 to 3,400 feet
Mean annual precipitation: 4 to 6 inches
Mean annual air temperature: 61 to 66 degrees F
Frost-free period: 210 to 250 days
Farmland classification: Not prime farmland

Map Unit Composition

Dalian and similar soils: 100 percent
Estimates are based on observations, descriptions, and transects of the mapunit.

Description of Dalian

Setting

Landform: Fan skirts
Down-slope shape: Linear
Across-slope shape: Convex
Parent material: Alluvium derived from limestone and dolostone

Typical profile

H1 - 0 to 5 inches: very cobbly fine sandy loam
H2 - 5 to 60 inches: very gravelly fine sandy loam

Properties and qualities

Slope: 2 to 8 percent
Depth to restrictive feature: More than 80 inches
Natural drainage class: Well drained
Runoff class: Low
Capacity of the most limiting layer to transmit water (Ksat): High (1.98 to 5.95 in/hr)
Depth to water table: More than 80 inches
Frequency of flooding: None
Frequency of ponding: None
Calcium carbonate, maximum in profile: 60 percent
Salinity, maximum in profile: Nonsaline to very slightly saline (0.0 to 2.0 mmhos/cm)
Available water storage in profile: Low (about 4.2 inches)

Interpretive groups

Land capability classification (irrigated): None specified
Land capability classification (nonirrigated): 7s
Hydrologic Soil Group: A

400—Tencee very gravelly fine sandy loam, 2 to 8 percent slopes

Map Unit Setting

National map unit symbol: hrc1
Elevation: 2,400 to 5,700 feet
Mean annual precipitation: 4 to 10 inches
Mean annual air temperature: 57 to 64 degrees F
Frost-free period: 180 to 250 days
Farmland classification: Not prime farmland

Map Unit Composition

Tencee and similar soils: 100 percent
Estimates are based on observations, descriptions, and transects of the mapunit.

Description of Tencee

Setting

Landform: Fan remnants
Down-slope shape: Linear
Across-slope shape: Convex
Parent material: Alluvium derived from limestone and dolomite

Typical profile

H1 - 0 to 9 inches: very gravelly fine sandy loam
H2 - 9 to 15 inches: very gravelly sandy loam
H3 - 15 to 19 inches: indurated

Properties and qualities

Slope: 2 to 8 percent
Depth to restrictive feature: 7 to 20 inches to petrocalcic
Natural drainage class: Well drained
Runoff class: Very high
Capacity of the most limiting layer to transmit water (Ksat): Very low (0.00 to 0.00 in/hr)
Depth to water table: More than 80 inches
Frequency of flooding: None
Frequency of ponding: None
Calcium carbonate, maximum in profile: 40 percent
Salinity, maximum in profile: Nonsaline to very slightly saline (0.0 to 2.0 mmhos/cm)
Sodium adsorption ratio, maximum in profile: 2.0
Available water storage in profile: Very low (about 1.0 inches)

Interpretive groups

Land capability classification (irrigated): None specified
Land capability classification (nonirrigated): 7s
Hydrologic Soil Group: D

Soil Information for All Uses

Soil Reports

The Soil Reports section includes various formatted tabular and narrative reports (tables) containing data for each selected soil map unit and each component of each unit. No aggregation of data has occurred as is done in reports in the Soil Properties and Qualities and Suitabilities and Limitations sections.

The reports contain soil interpretive information as well as basic soil properties and qualities. A description of each report (table) is included.

Water Features

This folder contains tabular reports that present soil hydrology information. The reports (tables) include all selected map units and components for each map unit. Water Features include ponding frequency, flooding frequency, and depth to water table.

Hydrologic Soil Group and Surface Runoff

This table gives estimates of various soil water features. The estimates are used in land use planning that involves engineering considerations.

Hydrologic soil groups are based on estimates of runoff potential. Soils are assigned to one of four groups according to the rate of water infiltration when the soils are not protected by vegetation, are thoroughly wet, and receive precipitation from long-duration storms.

The four hydrologic soil groups are:

Group A. Soils having a high infiltration rate (low runoff potential) when thoroughly wet. These consist mainly of deep, well drained to excessively drained sands or gravelly sands. These soils have a high rate of water transmission.

Group B. Soils having a moderate infiltration rate when thoroughly wet. These consist chiefly of moderately deep or deep, moderately well drained or well drained soils that have moderately fine texture to moderately coarse texture. These soils have a moderate rate of water transmission.

Group C. Soils having a slow infiltration rate when thoroughly wet. These consist chiefly of soils having a layer that impedes the downward movement of water or soils of moderately fine texture or fine texture. These soils have a slow rate of water transmission.

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Group D. Soils having a very slow infiltration rate (high runoff potential) when thoroughly wet. These consist chiefly of clays that have a high shrink-swell potential, soils that have a high water table, soils that have a claypan or clay layer at or near the surface, and soils that are shallow over nearly impervious material. These soils have a very slow rate of water transmission.

If a soil is assigned to a dual hydrologic group (A/D, B/D, or C/D), the first letter is for drained areas and the second is for undrained areas.

Surface runoff refers to the loss of water from an area by flow over the land surface. Surface runoff classes are based on slope, climate, and vegetative cover. The concept indicates relative runoff for very specific conditions. It is assumed that the surface of the soil is bare and that the retention of surface water resulting from irregularities in the ground surface is minimal. The classes are negligible, very low, low, medium, high, and very high.

Report—Hydrologic Soil Group and Surface Runoff

Absence of an entry indicates that the data were not estimated. The dash indicates no documented presence.

Hydrologic Soil Group and Surface Runoff—Las Vegas Valley Area, Nevada, Part of Clark County			
Map symbol and soil name	Pct. of map unit	Surface Runoff	Hydrologic Soil Group
155—Cave gravelly fine sandy loam, 4 to 15 percent slopes			
Cave	100	Very high	D
191—Dalian very cobbly fine sandy loam, 2 to 8 percent slopes			
Dalian	100	Low	A
400—Tencee very gravelly fine sandy loam, 2 to 8 percent slopes			
Tencee	100	Very high	D

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Curve Numbers Calculations

Soil Data & Curve Number Computation

Project Name: Skye Canyon - Detention Basin 1

Date: 8/9/2016

File Name: OLY1401-004 UP1 DB1 CNs.xlsx

By: JSR

Data:

Reference:

Exhibit D
SCS Soil Data

Soil Type - Soil Survey Area & Version, Soil Description			A	B	C	D
155	Las Vegas Valley 2014	Cave gravelly fine sandy loam, 4 to 15 percent slopes	0%	0%	0%	100%
191	Las Vegas Valley 2014	Dallan very cobbly fine sandy loam, 2 to 8 percent slopes	100%	0%	0%	0%
400	Las Vegas Valley 2014	Tencee very gravelly fine sandy loam, 2 to 8 percent slopes	0%	0%	0%	100%

Land Use		A	B	C	D
Desert Shrub (poor)		63	77	85	88
Newly Graded		77	86	91	94

PROPOSED CONDITION

BASIN ID	SOIL TYPE	SUBAREA PERCENT	LAND USE	HYDROLOGIC SOIL GROUP				LAND USE CN				SUBAREA ACRES	CURVE NUMBER
				A	B	C	D	A	B	C	D		
OFF1	400	51.0%	Desert Shrub (poor)	0%	0%	0%	100%	63	77	85	88	140.07	44.89
	155	44.2%	Desert Shrub (poor)	0%	0%	0%	100%	63	77	85	88	121.46	38.92
	191	4.8%	Desert Shrub (poor)	100%	0%	0%	0%	63	77	85	88	13.07	3.00
TOTAL PERCENT		100.0%	TOTAL AREA & COMPOSITE CURVE NUMBER:										274.60 86.81

BASIN ID	SOIL TYPE	SUBAREA PERCENT	LAND USE	HYDROLOGIC SOIL GROUP				LAND USE CN				SUBAREA ACRES	CURVE NUMBER
				A	B	C	D	A	B	C	D		
OFF2	155	75.1%	Desert Shrub (poor)	0%	0%	0%	100%	63	77	85	88	20.12	66.09
	191	24.9%	Desert Shrub (poor)	100%	0%	0%	0%	63	77	85	88	6.67	15.69
TOTAL PERCENT		100.0%	TOTAL AREA & COMPOSITE CURVE NUMBER:										26.79 81.78

BASIN ID	SOIL TYPE	SUBAREA PERCENT	LAND USE	HYDROLOGIC SOIL GROUP				LAND USE CN				SUBAREA ACRES	CURVE NUMBER
				A	B	C	D	A	B	C	D		
OFF3	155	100.0%	Desert Shrub (poor)	0%	0%	0%	100%	63	77	85	88	16.58	88.00
TOTAL PERCENT		100.0%	TOTAL AREA & COMPOSITE CURVE NUMBER:										16.58 88.00

BASIN ID	SOIL TYPE	SUBAREA PERCENT	LAND USE	HYDROLOGIC SOIL GROUP				LAND USE CN				SUBAREA ACRES	CURVE NUMBER
				A	B	C	D	A	B	C	D		
OFF4	155	100.0%	Desert Shrub (poor)	0%	0%	0%	100%	63	77	85	88	37.19	88.00
TOTAL PERCENT		100.0%	TOTAL AREA & COMPOSITE CURVE NUMBER:										37.19 88.00

BASIN ID	SOIL TYPE	SUBAREA PERCENT	LAND USE	HYDROLOGIC SOIL GROUP				LAND USE CN				SUBAREA ACRES	CURVE NUMBER
				A	B	C	D	A	B	C	D		
OFF5	155	63.9%	Desert Shrub (poor)	0%	0%	0%	100%	63	77	85	88	105.09	56.25
	400	36.1%	Desert Shrub (poor)	0%	0%	0%	100%	63	77	85	88	59.32	31.75
TOTAL PERCENT		100.0%	TOTAL AREA & COMPOSITE CURVE NUMBER:										164.41 88.00

BASIN ID	SOIL TYPE	SUBAREA PERCENT	LAND USE	HYDROLOGIC SOIL GROUP				LAND USE CN				SUBAREA ACRES	CURVE NUMBER
				A	B	C	D	A	B	C	D		
OFF6	155	100.0%	Desert Shrub (poor)	0%	0%	0%	100%	63	77	85	88	25.54	88.00
TOTAL PERCENT		100.0%	TOTAL AREA & COMPOSITE CURVE NUMBER:										25.54 88.00

BASIN ID	SOIL TYPE	SUBAREA PERCENT	LAND USE	HYDROLOGIC SOIL GROUP				LAND USE CN				SUBAREA ACRES	CURVE NUMBER
				A	B	C	D	A	B	C	D		
ON1	155	100.0%	Newly Graded	0%	0%	0%	100%	77	86	91	94	16.72	94.00
TOTAL PERCENT		100.0%	TOTAL AREA & COMPOSITE CURVE NUMBER:										16.72 94.00

ULTIMATE CONDITION

BASIN ID	SOIL TYPE	SUBAREA PERCENT	LAND USE	HYDROLOGIC SOIL GROUP				LAND USE CN				SUBAREA ACRES	CURVE NUMBER
				A	B	C	D	A	B	C	D		
OFF1	400	51.0%	Desert Shrub (poor)	0%	0%	0%	100%	63	77	85	88	129.73	44.85
	155	47.4%	Desert Shrub (poor)	0%	0%	0%	100%	63	77	85	88	120.71	41.73
	191	1.6%	Desert Shrub (poor)	100%	0%	0%	0%	63	77	85	88	4.11	1.02
TOTAL PERCENT		100.0%	TOTAL AREA & COMPOSITE CURVE NUMBER:										254.55 87.60

BASIN ID	SOIL TYPE	SUBAREA PERCENT	LAND USE	HYDROLOGIC SOIL GROUP				LAND USE CN				SUBAREA ACRES	CURVE NUMBER
				A	B	C	D	A	B	C	D		
OFF2	155	86.2%	Desert Shrub (poor)	0%	0%	0%	100%	63	77	85	88	19.60	75.85
	191	13.8%	Desert Shrub (poor)	100%	0%	0%	0%	63	77	85	88	3.14	8.70
TOTAL PERCENT		100.0%	TOTAL AREA & COMPOSITE CURVE NUMBER:										22.74 84.55

OFF7 Curve Number referenced from CCRFCD MPU.

HYDROLOGIC CRITERIA AND DRAINAGE DESIGN MANUAL

RUNOFF CURVE NUMBERS (URBAN AREAS¹)

Cover description		Curve numbers for hydrologic soil group—			
Cover type and hydrologic condition	Average percent impervious area ²	A	B	C	D
<i>Fully developed urban areas (vegetation established)</i>					
Open space (lawns, parks, golf courses, cemeteries, etc.): ³					
Poor condition (grass cover < 50%)		68	79	86	89
Fair condition (grass cover 50% to 75%).....		49	69	79	84
Good condition (grass cover > 75%)		39	61	74	80
Impervious areas:					
Paved parking lots, roofs, driveways, etc. (excluding right-of-way)		98	98	98	98
Streets and roads:					
Paved; curbs and storm sewers (excluding right-of-way).....		98	98	98	98
Paved; open ditches (including right-of-way)		83	89	92	93
Gravel (including right-of-way)		76	85	89	91
Dirt (including right-of-way)		72	82	87	89
Western desert urban areas:					
Natural desert landscaping (pervious areas only) ⁴ ...		63	77	85	88
Artificial desert landscaping (impervious weed barrier, desert shrub with 1- to 2-inch sand or gravel mulch and basin borders)		96	96	96	96
Urban districts:					
Commercial and business.....	85	89	92	94	95
Industrial.....	72	81	88	91	93
Residential districts by average lot size:					

See Table 602A

Developing urban areas

Newly graded areas (pervious areas only, no vegetation) ⁵	77	86	91	94
--	----	----	----	----

1. Average runoff condition, and $I_p = 0.2S$.

2. The average percent impervious area shown was used to develop the composite CN's. Other assumptions are as follows: impervious areas are directly connected to the drainage system. Impervious areas have a CN of 98, and pervious areas are considered equivalent to open space in good hydrologic condition. CN's for other combinations of conditions may be computed using Figure 603.

3. CN's shown are equivalent to those of pasture. Composite CN's may be computed for other combinations of open space cover type.

4. Composite CN's for natural desert landscaping should be computed using Figure 603 based on the impervious area percentage (CN #98) and the pervious area CN. The pervious area CN's are assumed equivalent to desert shrub in poor hydrologic condition.

5. Composite CN's to use for the design of temporary measures during grading and construction should be computed using Figure 603 based on the degree of development impervious area percentage) and the CN's for the newly graded pervious areas.

Revision	Date

**WRC
ENGINEERING**

REFERENCE:
SCS TR-55, USDA, June 1986.

**TABLE 602
1 of 4**

HYDROLOGIC CRITERIA AND DRAINAGE DESIGN MANUAL

RUNOFF CURVE NUMBERS (SEMIARID RANGELANDS¹)

Cover description		Curve numbers for hydrologic soil group—			
Cover type	Hydrologic condition ²	A ³	B	C	D
Herbaceous—mixture of grass, weeds, and low-growing brush, with brush the minor element.	Poor		80	87	93
	Fair		71	81	89
	Good		62	74	85
Oak-aspen—mountain brush mixture of oak brush, aspen, mountain mahogany, bitter brush, maple, and other brush.	Poor		66	74	79
	Fair		48	57	63
	Good		30	41	48
Pinyon-juniper—pinyon, juniper, or both; grass understory.	Poor		75	85	89
	Fair		58	73	80
	Good		41	61	71
Sagebrush with grass understory.	Poor		67	80	85
	Fair		51	63	70
	Good		35	47	55
<u>Desert shrub</u> —major plants include saltbush, greasewood, creosotebush, blackbrush, bursage, palo verde, mesquite, and cactus.	Poor	63	77	85	88
	Fair	55	72	81	86
	Good	49	68	79	84

¹Average runoff condition, and $I_a = 0.2S$.

²Poor: <30% ground cover (litter, grass, and brush overstory).

Fair: 30 to 70% ground cover.

Good: >70% ground cover.

³Curve numbers for group A have been developed only for desert shrub.

Revision	Date

**WRC
ENGINEERING**

REFERENCE:

SCS TR-55, USDA, June 1986.

**TABLE 602
4 of 4**

Standard Form 4 Lag Time Computations



STANDARD FORM 4

Stage-Storage Table from Civil-3D

STAGE STORAGE TABLE

ELEV	AREA (sq. ft.)	DEPTH (ft)	AVG END INC. VOL. (cu. ft.)	AVG END TOTAL VOL. (cu. ft.)	CONIC INC. VOL. (cu. ft.)	CONIC TOTAL VOL. (cu. ft.)
3,024.00	74,982.51	N/A	N/A	0.00	N/A	0.00
3,025.00	170,154.76	1.00	122568.64	122568.64	119363.79	119363.79
3,026.00	228,998.19	1.00	199576.48	322145.11	198849.61	318213.40
3,027.00	245,049.48	1.00	237023.83	559168.94	236978.53	555191.93
3,028.00	253,122.58	1.00	249086.03	808254.97	249075.12	804267.06
3,029.00	261,333.21	1.00	257227.90	1065482.87	257216.98	1061484.03
3,030.00	269,696.08	1.00	255514.64	1330997.51	265503.67	1326987.70
3,031.00	278,138.02	1.00	273917.05	1604914.56	273906.21	1600893.91
3,032.00	286,530.56	1.00	282334.29	1887248.85	282323.89	1883217.80
3,033.00	295,026.99	1.00	290778.78	2178027.62	290768.43	2173986.23
3,034.00	303,609.93	1.00	299318.46	2477346.09	299308.21	2473294.44
3,035.00	312,269.76	1.00	307939.85	2785285.93	307929.70	2781224.14
3,036.00	320,992.02	1.00	316630.89	3101916.82	316620.88	3097845.01
3,037.00	331,479.47	1.00	326235.74	3428152.57	326221.70	3424066.71
3,038.00	342,512.74	1.00	336996.11	3765148.67	336981.05	3761047.76
3,039.00	352,259.74	1.00	347386.24	4112534.91	347374.85	4108422.61
3,040.00	362,058.02	1.00	357158.88	4469693.80	357147.68	4465570.29
3,041.00	371,872.50	1.00	366965.26	4836659.06	366954.32	4832524.61
3,042.00	381,857.32	1.00	376864.91	5213523.96	376853.88	5209378.50
3,043.00	391,872.49	1.00	386864.90	5600388.87	386854.10	5596232.60
3,044.00	402,046.69	1.00	396959.59	5997348.45	396948.72	5993181.32
3,045.00	412,166.77	1.00	407106.73	6404455.18	407096.25	6400277.56
3,046.00	422,288.06	1.00	417227.41	6821682.60	417217.18	6817494.75

Outlet Pipe & Emergency Spillway Stage-Discharge Tables
(Unchanged from Previous Approved Design)

DB1 - Stage-Discharge Summary

Stage (elevation)	24" Orifice Outlet Pipe Flow (cfs)	Spillway Flow (cfs)	Total Flow (cfs)
3013.8	0.00	0.00	0
3014	0.00	0.00	0
3015	0.00	0.00	0
3016	0.00	0.00	0
3017	13.71	0.00	13.7
3018	21.36	0.00	21.4
3019	26.92	0.00	26.9
3020	31.51	0.00	31.5
3021	35.51	0.00	35.5
3022	39.11	0.00	39.1
3023	42.40	0.00	42.4
3024	45.45	0.00	45.5
3025	48.32	0.00	48.3
3026	51.02	0.00	51.0
3027	53.58	0.00	53.6
3028	56.03	0.00	56.0
3029	58.38	0.00	58.4
3030	60.63	0.00	60.6
3031	62.80	0.00	62.8
3032	64.91	0.00	64.9
3033	66.94	0.00	66.9
3034	68.92	0.00	68.9
3035	70.84	0.00	70.8
3036	72.70	0.00	72.7
3037	74.53	0.00	74.5
3038	76.31	261.00	337.3
3039	78.04	738.22	816.3
3040	79.75	1356.20	1436.0
3041	81.41	2088.00	2169.4
3042	83.04	2918.07	3001.1
3043	84.64	3835.90	3920.5
3044	86.21	4833.79	4920.0
3045	87.75	5905.76	5993.5
3046	89.27	7047.00	7136.3

24-inch Orifice Discharge Curve Rating Table for Circular Orifice

Project Description	
Worksheet	Outfall
Type	Circular Orifice
Solve For	Discharge

Input Data	
Centroid Elevation	3,014.80 ft
Tailwater Elevation	3,016.30 ft
Discharge Coefficient	0.65
Diameter	24.0 in

→ CONSIDERS 2.5' DEPTH IN 60" RCP OUTLET
WSPG MODEL FOR OUTLET PIPE CONFIRMS
TAILWATER IN 60" RCP

Attribute	Minimum	Maximum	Increment
Headwater Elevation (ft)	3,014.00	3,046.00	1.00

Headwater Elevation (ft)	Discharge (cfs)	Velocity (ft/s)
3,014.00	N/A	N/A
3,015.00	N/A	N/A
3,016.00	N/A	N/A
3,017.00	13.71	4.36
3,018.00	21.36	6.80
3,019.00	26.92	8.57
3,020.00	31.51	10.03
3,021.00	35.51	11.30
3,022.00	39.11	12.45
3,023.00	42.40	13.50
3,024.00	45.45	14.47
3,025.00	48.32	15.38
3,026.00	51.02	16.24
3,027.00	53.58	17.06
3,028.00	56.03	17.84
3,029.00	58.38	18.58
3,030.00	60.63	19.30
3,031.00	62.80	19.99
3,032.00	64.91	20.66
3,033.00	66.94	21.31
3,034.00	68.92	21.94
3,035.00	70.84	22.55
3,036.00	72.70	23.14
3,037.00	74.53	23.72
3,038.00	76.31	24.29
3,039.00	78.04	24.84
3,040.00	79.75	25.38
3,041.00	81.41	25.91
3,042.00	83.04	26.43
3,043.00	84.64	26.94
3,044.00	86.21	27.44
3,045.00	87.75	27.93
3,046.00	89.27	28.42

PMF Overflow Spillway Discharge Curve **Rating Table for Generic Weir**

Project Description	
Worksheet	PMF Spillway - Weir
Type	Generic Weir
Solve For	Discharge

Input Data	
Crest Elevation	3,037.00 ft
Discharge Coefficient	3.00 US
Crest Length	87.00 ft

Attribute	Minimum	Maximum	Increment
Headwater Elevation (ft)	3,037.00	3,046.00	1.00

Headwater Elevation (ft)	Discharge (cfs)	Velocity (ft/s)
3,037.00	N/A	N/A
3,038.00	261.00	3.00
3,039.00	738.22	4.24
3,040.00	1,356.20	5.20
3,041.00	2,088.00	6.00
3,042.00	2,918.07	6.71
3,043.00	3,835.90	7.35
3,044.00	4,833.79	7.94
3,045.00	5,905.76	8.49
3,046.00	7,047.00	9.00

Proposed Condition HEC-1 Output

 * FLOOD HYDROGRAPH PACKAGE (HEC-1) *
 * JUN 1998 *
 * VERSION 4.1 *
 * RUN DATE 09AUG16 TIME 15:57:58 *

PRO1.OUT

 * U.S. ARMY CORPS OF ENGINEERS *
 * HYDROLOGIC ENGINEERING CENTER *
 * 609 SECOND STREET *
 * DAVIS, CALIFORNIA 95616 *
 * (916) 756-1104 *

```

      X  X  XXXXXX  XXXXX  X
      X  X  X      X      XX
      X  X  X      X      X
      XXXXXX  XXXX  X      X
      X  X  X      X      X
      X  X  X      X      X
      X  X  XXXXXX  XXXXX  XXX
  
```

THIS PROGRAM REPLACES ALL PREVIOUS VERSIONS OF HEC-1 KNOWN AS HEC1 (JAN 73), HEC1GS, HEC1DB, AND HEC1KW.

THE DEFINITIONS OF VARIABLES -RTIMP- AND -RTIOR- HAVE CHANGED FROM THOSE USED WITH THE 1973-STYLE INPUT STRUCTURE.
 THE DEFINITION OF -AMSKK- ON RM-CARD WAS CHANGED WITH REVISIONS DATED 28 SEP 81. THIS IS THE FORTRAN77 VERSION
 NEW OPTIONS: DAMBREAK OUTFLOW SUBMERGENCE, SINGLE EVENT DAMAGE CALCULATION, DSS:WRITE STAGE FREQUENCY,
 DSS:READ TIME SERIES AT DESIRED CALCULATION INTERVAL LOSS RATE:GREEN AND AMPT INFILTRATION
 KINEMATIC WAVE: NEW FINITE DIFFERENCE ALGORITHM

1 HEC-1 INPUT PAGE 1

```

LINE      ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10
1          *DIAGRAM
2          ID      SLATER HANIFAN GROUP
3          ID      NINETY-FIVE MANAGEMENT, LLC.
4          ID      SKYE CANYON - DETENTION BASIN #1
5          ID      UPDATE (SHEEP MOUNTAIN)
6          ID      PROPOSED DRAINAGE CONDITION
7          ID      RETURN PERIOD ----- 100-YEAR
8          ID      DISTRIBUTION ----- 6-HOUR SDN3
9          ID      PROJECT NO. ----- 0LY1401.004
10         ID      FILENAME ----- PRO1.H1
11         ID      DATE ----- 7/25/2016
12         ID      MODELED BY ----- SMJ
13         ID
14         ID      SIX HOUR DEPTH-AREA REDUCTION FACTORS
15         ID      DRAINAGE AREA (MIA2)      RATIO
16         ID      0.0-0.49      1.00
17         ID      0.5-0.99      0.98
18         ID
19         IT      5      0      0      300
20         IO      5      0      0
21         IN      5      0      0
22         JR      PREC 1.00 0.98
23         *
24         KK      OFF1
25         KM      EXISTING OFFSITE DRAINAGE BASIN
26         KO      DISCHARGES INTO DETENTION BASIN "DB1"
27         PC      3.00
28         PC      0      0.020 0.057 0.070 0.087 0.108 0.124 0.130 0.130 0.130
29         PC      0.130 0.130 0.130 0.133 0.140 0.142 0.148 0.158 0.172 0.181
30         PC      0.190 0.197 0.199 0.200 0.201 0.204 0.214 0.229 0.241 0.249
31         PC      0.251 0.256 0.270 0.278 0.281 0.283 0.295 0.322 0.352 0.409
32         PC      0.499 0.590 0.710 0.744 0.781 0.812 0.819 0.835 0.851 0.856
33         PC      0.860 0.868 0.876 0.888 0.910 0.926 0.937 0.950 0.970 0.976
34         PC      0.982 0.985 0.987 0.989 0.990 0.993 0.993 0.994 0.995 0.998
35         PC      0.998 0.999 1
36         BA      .4291
37         LS      0      86.8
38         UD      0.420
39         *
40         KK      OFF2
41         KM      EXISTING OFFSITE DRAINAGE BASIN
42         KO      DISCHARGES INTO DETENTION BASIN "DB1"
43         BA      .0419
44         LS      0      81.8
45         UD      0.150
  
```

1 HEC-1 INPUT PAGE 2

```

LINE      ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10
46         KK      OFF3
47         KM      EXISTING OFFSITE DRAINAGE BASIN
48         KO      DISCHARGES INTO DETENTION BASIN "DB1"
49         PC      3
50         BA      .0259
51         LS      0      88.0
52         UD      0.112
53         *
54         KK      CP1
55         KM      COMBINE OFF1, OFF2, AND OFF3
56         HC      3
57         *
58         KK      OFF4
59         KM      EXISTING OFFSITE DRAINAGE BASIN
60         KO      DISCHARGES INTO DETENTION BASIN "DB1"
61         PC      3
62         BA      .0581
63         LS      0      88.0
64         UD      0.136
65         *
66         KK      OFF5
67         KM      EXISTING OFFSITE DRAINAGE BASIN
68         KO      DISCHARGES INTO DETENTION BASIN "DB1"
69         PC      3
70         BA      .2569
71         LS      0      88.0
72         UD      0.419
73         *
74         KK      OFF6
75         KM      EXISTING OFFSITE DRAINAGE BASIN
76         KO      DISCHARGES INTO DETENTION BASIN "DB1"
  
```

PRO1.OUT

73 KO 3
74 BA .0399
75 LS 0 88.0
76 UD 0.121
*

77 KK ON1
78 KM PROPOSED ONSITE DRAINAGE BASIN
79 BA .0261
80 LS 0 94.0
81 UD 0.116
*

82 KK DB1
83 KM COMBINE CP1, OFF4, OFF5, OFF6 AND ON1
84 HC 5
*

1

HEC-1 INPUT

PAGE 3

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

85 KK RDB1
86 KM OUTLET = 24" ORIFICE @ INVERT ELEVATION = 3013.8-FT
87 RS 1 ELEV 3025.5
88 SA 4.58 5.63 5.81 6.00 6.19 7.17 7.61 8.31 9.00 9.69
89 SE 3025.5 3027 3028 3029 3030 3035 3037 3040 3043 3046
90 SQ 49.7 60.6 70.8 74.5 1436.0 3920.5 7136.3
91 SE 3025.5 3030 3035 3037 3040 3043 3046
*

1

SCHEMATIC DIAGRAM OF STREAM NETWORK

INPUT LINE (V) ROUTING (--->) DIVERSION OR PUMP FLOW
NO. (.) CONNECTOR (<---) RETURN OF DIVERTED OR PUMPED FLOW

```

23 OFF1
.
39 OFF2
.
46 OFF3
.
53 CP1.....
.
56 OFF4
.
63 OFF5
.
70 OFF6
.
77 ON1
.
82 DB1.....
V
85 RDB1
V

```

(***) RUNOFF ALSO COMPUTED AT THIS LOCATION

* FLOOD HYDROGRAPH PACKAGE (HEC-1) *
* JUN 1998 *
* VERSION 4.1 *
* RUN DATE 09AUG16 TIME 15:57:58 *

* U.S. ARMY CORPS OF ENGINEERS *
* HYDROLOGIC ENGINEERING CENTER *
* 609 SECOND STREET *
* DAVIS, CALIFORNIA 95616 *
* (916) 756-1104 *

SLATER HANIFAN GROUP
NINETY-FIVE MANAGEMENT, LLC.
SKYE CANYON - DETENTION BASIN #1
UPDATE (SHEEP MOUNTAIN)
PROPOSED DRAINAGE CONDITION

RETURN PERIOD ----- 100-YEAR
DISTRIBUTION ----- 6-HOUR SDN3
PROJECT NO. ----- OLY1401.004
FILENAME ----- PRO1.H1
DATE ----- 7/25/2016
MODELED BY ----- SMJ

SIX HOUR DEPTH-AREA REDUCTION FACTORS
DRAINAGE AREA (MI^2) RATIO
0.0-0.49 1.00
0.5-0.99 0.98

20 IO OUTPUT CONTROL VARIABLES
IPRNT 5 PRINT CONTROL
IPLOT 0 PLOT CONTROL
QSCAL 0. HYDROGRAPH PLOT SCALE

IT HYDROGRAPH TIME DATA
NMIN 5 MINUTES IN COMPUTATION INTERVAL
IDATE 1 0 STARTING DATE
ITIME 0000 STARTING TIME
NQ 300 NUMBER OF HYDROGRAPH ORDINATES
NDATE 2 0 ENDING DATE
NDTIME 0055 ENDING TIME
ICENT 19 CENTURY MARK

COMPUTATION INTERVAL .08 HOURS
TOTAL TIME BASE 24.92 HOURS

ENGLISH UNITS
DRAINAGE AREA SQUARE MILES
PRECIPITATION DEPTH INCHES
LENGTH, ELEVATION FEET
FLOW CUBIC FEET PER SECOND
STORAGE VOLUME ACRE-Feet
SURFACE AREA ACRES
TEMPERATURE DEGREES FAHRENHEIT

JP MULTI-PLAN OPTION
NPLAN 1 NUMBER OF PLANS

JR MULTI-RATIO OPTION
RATIOS OF PRECIPITATION
1.00 .98

*** **

```

*****
*      *
* OFF1 *
*      *
*****

26 KO      OUTPUT CONTROL VARIABLES
            IPRNT      3  PRINT CONTROL
            IPLOT      0  PLOT CONTROL
            QSCAL      0.  HYDROGRAPH PLOT SCALE

21 IN      TIME DATA FOR INPUT TIME SERIES
            JXMIN      5  TIME INTERVAL IN MINUTES
            JXDATE     1  0  STARTING DATE
            JXTIME     0  STARTING TIME

SUBBASIN RUNOFF DATA

36 BA      SUBBASIN CHARACTERISTICS
            TAREA      .43  SUBBASIN AREA

PRECIPITATION DATA

27 PB      STORM      3.00  BASIN TOTAL PRECIPITATION

28 PI      INCREMENTAL PRECIPITATION PATTERN
            .02      .04      .01      .02      .02      .02      .01      .00      .00      .00
            .00      .00      .00      .01      .00      .01      .01      .01      .01      .01
            .01      .00      .00      .00      .00      .00      .02      .02      .01      .00
            .01      .01      .01      .00      .00      .01      .03      .03      .06      .09
            .09      .12      .03      .04      .03      .01      .02      .02      .00      .00
            .01      .01      .01      .02      .02      .01      .01      .02      .01      .01
            .00      .00      .00      .00      .00      .00      .00      .00      .00      .00

37 LS      SCS LOSS RATE
            STRTL      .30  INITIAL ABSTRACTION
            CRVNBK     86.80  CURVE NUMBER
            RTIMP      .00  PERCENT IMPERVIOUS AREA

38 UD      SCS DIMENSIONLESS UNITGRAPH
            TLAG      .42  LAG

```

```

UNIT HYDROGRAPH
27 END-OF-PERIOD ORDINATES
39. 118. 246. 379. 444. 445. 397. 330. 240. 174.
129. 98. 73. 55. 41. 30. 22. 17. 12. 9.
7. 5. 4. 3. 2. 1. 1.

TOTAL RAINFALL = 3.00, TOTAL LOSS = 1.28, TOTAL EXCESS = 1.72

PEAK FLOW TIME 6-HR MAXIMUM AVERAGE FLOW 24.92-HR
+ (CFS) (HR) 24-HR 72-HR
+ 399. 3.83 (CFS) 79. 20. 19. 19.
(INCHES) 1.721 1.724 1.724 1.724
(AC-FT) 39. 39. 39. 39.

CUMULATIVE AREA = .43 SQ MI

```

*** **

```

HYDROGRAPH AT STATION OFF1
FOR PLAN 1, RATIO = 1.00

TOTAL RAINFALL = 3.00, TOTAL LOSS = 1.28, TOTAL EXCESS = 1.72

PEAK FLOW TIME 6-HR MAXIMUM AVERAGE FLOW 24.92-HR
+ (CFS) (HR) 24-HR 72-HR
+ 399. 3.83 (CFS) 79. 20. 19. 19.
(INCHES) 1.721 1.724 1.724 1.724
(AC-FT) 39. 39. 39. 39.

CUMULATIVE AREA = .43 SQ MI

```

*** **

```

HYDROGRAPH AT STATION OFF1
FOR PLAN 1, RATIO = .98

TOTAL RAINFALL = 2.94, TOTAL LOSS = 1.27, TOTAL EXCESS = 1.67

PEAK FLOW TIME 6-HR MAXIMUM AVERAGE FLOW 24.92-HR
+ (CFS) (HR) 24-HR 72-HR
+ 387. 3.83 (CFS) 77. 19. 19. 19.
(INCHES) 1.669 1.671 1.671 1.671
(AC-FT) 38. 38. 38. 38.

CUMULATIVE AREA = .43 SQ MI

```

*** **

```

*****
*      *
* OFF2 *
*      *
*****

42 KO      OUTPUT CONTROL VARIABLES
            IPRNT      3  PRINT CONTROL
            IPLOT      0  PLOT CONTROL
            QSCAL      0.  HYDROGRAPH PLOT SCALE

SUBBASIN RUNOFF DATA

43 BA      SUBBASIN CHARACTERISTICS

```

TAREA .04 SUBBASIN AREA PRO1.OUT

PRECIPITATION DATA

27 PB STORM 3.00 BASIN TOTAL PRECIPITATION

28 PI INCREMENTAL PRECIPITATION PATTERN

.02	.04	.01	.02	.02	.02	.01	.00	.00	.00
.00	.00	.00	.01	.00	.01	.01	.01	.01	.01
.01	.00	.00	.00	.00	.01	.02	.01	.01	.00
.01	.01	.01	.00	.00	.01	.03	.03	.06	.09
.09	.12	.03	.04	.03	.01	.02	.02	.00	.00
.01	.01	.01	.02	.02	.01	.01	.02	.01	.01
.00	.00	.00	.00	.00	.00	.00	.00	.00	.00

44 LS SCS LOSS RATE .44 INITIAL ABSTRACTION

STRTL 81.80 CURVE NUMBER

CRVNBRTIMP .00 PERCENT IMPERVIOUS AREA

45 UD SCS DIMENSIONLESS UNITGRAPH

TLAG .15 LAG

UNIT HYDROGRAPH
11 END-OF-PERIOD ORDINATES

39. 103. 91. 46. 23. 11. 6. 3. 1. 1.

TOTAL RAINFALL = 3.00, TOTAL LOSS = 1.63, TOTAL EXCESS = 1.37

PEAK FLOW TIME 6-HR MAXIMUM AVERAGE FLOW 24-HR 72-HR 24.92-HR

+ (CFS) (HR) (CFS)

+ 49. 3.58 6. 2. 1. 1.

(INCHES) 1.366 1.366 1.366 1.366

(AC-FT) 3. 3. 3. 3.

CUMULATIVE AREA = .04 SQ MI

HYDROGRAPH AT STATION OFF2
FOR PLAN 1, RATIO = 1.00

TOTAL RAINFALL = 3.00, TOTAL LOSS = 1.63, TOTAL EXCESS = 1.37

PEAK FLOW TIME 6-HR MAXIMUM AVERAGE FLOW 24-HR 72-HR 24.92-HR

+ (CFS) (HR) (CFS)

+ 49. 3.58 6. 2. 1. 1.

(INCHES) 1.366 1.366 1.366 1.366

(AC-FT) 3. 3. 3. 3.

CUMULATIVE AREA = .04 SQ MI

HYDROGRAPH AT STATION OFF2
FOR PLAN 1, RATIO = .98

TOTAL RAINFALL = 2.94, TOTAL LOSS = 1.62, TOTAL EXCESS = 1.32

PEAK FLOW TIME 6-HR MAXIMUM AVERAGE FLOW 24-HR 72-HR 24.92-HR

+ (CFS) (HR) (CFS)

+ 48. 3.58 6. 1. 1. 1.

(INCHES) 1.319 1.319 1.319 1.319

(AC-FT) 3. 3. 3. 3.

CUMULATIVE AREA = .04 SQ MI

*** **

46 KK OFF3

49 KO OUTPUT CONTROL VARIABLES

IPRNT 3 PRINT CONTROL

IPLOT 0 PLOT CONTROL

QSCAL 0. HYDROGRAPH PLOT SCALE

SUBBASIN RUNOFF DATA

50 BA SUBBASIN CHARACTERISTICS

TAREA .03 SUBBASIN AREA

PRECIPITATION DATA

27 PB STORM 3.00 BASIN TOTAL PRECIPITATION

28 PI INCREMENTAL PRECIPITATION PATTERN

.02	.04	.01	.02	.02	.02	.01	.00	.00	.00
.00	.00	.00	.01	.00	.01	.01	.01	.01	.01
.01	.00	.00	.00	.00	.01	.02	.01	.01	.00
.01	.01	.01	.00	.00	.01	.03	.03	.06	.09
.09	.12	.03	.04	.03	.01	.02	.02	.00	.00
.01	.01	.01	.02	.02	.01	.01	.02	.01	.01
.00	.00	.00	.00	.00	.00	.00	.00	.00	.00

51 LS SCS LOSS RATE .27 INITIAL ABSTRACTION

STRTL 88.00 CURVE NUMBER

CRVNBRTIMP .00 PERCENT IMPERVIOUS AREA

52 UD SCS DIMENSIONLESS UNITGRAPH

TLAG .11 LAG

UNIT HYDROGRAPH
9 END-OF-PERIOD ORDINATES

45. 81. 44. 18. 7. 3. 1. 1. 0.

PRO1.OUT

TOTAL RAINFALL = 3.00, TOTAL LOSS = 1.18, TOTAL EXCESS = 1.82

PEAK FLOW (CFS)	TIME (HR)	6-HR (CFS)	24-HR MAXIMUM AVERAGE FLOW	72-HR MAXIMUM AVERAGE FLOW	24.92-HR
40.	3.58	5. (INCHES) (AC-FT)	1.818 3.	1.818 3.	1.818 3.

CUMULATIVE AREA = .03 SQ MI

HYDROGRAPH AT STATION OFF3
FOR PLAN 1, RATIO = 1.00

TOTAL RAINFALL = 3.00, TOTAL LOSS = 1.18, TOTAL EXCESS = 1.82

PEAK FLOW (CFS)	TIME (HR)	6-HR (CFS)	24-HR MAXIMUM AVERAGE FLOW	72-HR MAXIMUM AVERAGE FLOW	24.92-HR
40.	3.58	5. (INCHES) (AC-FT)	1.818 3.	1.818 3.	1.818 3.

CUMULATIVE AREA = .03 SQ MI

HYDROGRAPH AT STATION OFF3
FOR PLAN 1, RATIO = .98

TOTAL RAINFALL = 2.94, TOTAL LOSS = 1.18, TOTAL EXCESS = 1.76

PEAK FLOW (CFS)	TIME (HR)	6-HR (CFS)	24-HR MAXIMUM AVERAGE FLOW	72-HR MAXIMUM AVERAGE FLOW	24.92-HR
39.	3.58	5. (INCHES) (AC-FT)	1.765 2.	1.765 2.	1.765 2.

CUMULATIVE AREA = .03 SQ MI

*** **

*
* OFF4 *
*

59 KO OUTPUT CONTROL VARIABLES
IPRNT 3 PRINT CONTROL
IPLOT 0 PLOT CONTROL
QSCAL 0. HYDROGRAPH PLOT SCALE

SUBBASIN RUNOFF DATA
60 BA SUBBASIN CHARACTERISTICS
TAREA .06 SUBBASIN AREA

PRECIPITATION DATA
27 PB STORM 3.00 BASIN TOTAL PRECIPITATION

28 PI INCREMENTAL PRECIPITATION PATTERN

.02	.04	.01	.02	.02	.02	.01	.00	.00	.00
.00	.00	.00	.01	.00	.01	.01	.01	.01	.01
.01	.00	.00	.00	.00	.01	.02	.01	.01	.00
.01	.01	.01	.00	.00	.01	.03	.03	.06	.09
.09	.12	.03	.04	.03	.01	.02	.02	.00	.00
.01	.01	.01	.02	.02	.01	.01	.02	.01	.01
.00	.00	.00	.00	.00	.00	.00	.00	.00	.00
.00	.00								

61 LS SCS LOSS RATE
STRTL .27 INITIAL ABSTRACTION
CRVNB 88.00 CURVE NUMBER
RTIMP .00 PERCENT IMPERVIOUS AREA

62 UD SCS DIMENSIONLESS UNITGRAPH
TLAG .14 LAG

UNIT HYDROGRAPH
10 END-OF-PERIOD ORDINATES

67.	158.	123.	55.	26.	12.	6.	3.	1.	0.
TOTAL RAINFALL =	3.00	TOTAL LOSS =	1.18	TOTAL EXCESS =	1.82				
PEAK FLOW (CFS)	TIME (HR)	6-HR (CFS)	24-HR MAXIMUM AVERAGE FLOW	72-HR MAXIMUM AVERAGE FLOW	24.92-HR				
90.	3.58	11. (INCHES) (AC-FT)	1.818 6.	1.818 6.	1.818 6.				

CUMULATIVE AREA = .06 SQ MI

HYDROGRAPH AT STATION OFF4
FOR PLAN 1, RATIO = 1.00

TOTAL RAINFALL = 3.00, TOTAL LOSS = 1.18, TOTAL EXCESS = 1.82

PEAK FLOW (CFS)	TIME (HR)	6-HR (CFS)	24-HR MAXIMUM AVERAGE FLOW	72-HR MAXIMUM AVERAGE FLOW	24.92-HR
90.	3.58	11. (INCHES) (AC-FT)	1.818 6.	1.818 6.	1.818 6.

CUMULATIVE AREA = .06 SQ MI

```

***          ***          ***          ***          ***
HYDROGRAPH AT STATION OFF4
FOR PLAN 1, RATIO = .98
TOTAL RAINFALL = 2.94, TOTAL LOSS = 1.18, TOTAL EXCESS = 1.76
PEAK FLOW      TIME      6-HR      MAXIMUM AVERAGE FLOW
+ (CFS)        (HR)      24-HR      72-HR      24.92-HR
+ 87.          3.58      (CFS)
                        11.          3.          3.          3.
                        (INCHES) 1.765 1.765 1.765 1.765
                        (AC-FT) 5.    5.    5.    5.
CUMULATIVE AREA = .06 SQ MI

```

```

*****
*          *
63 KK      * OFF5 *
*          *
*****

```

```

66 KO      OUTPUT CONTROL VARIABLES
            IPRNT      3 PRINT CONTROL
            IPLOT      0 PLOT CONTROL
            QSCAL      0. HYDROGRAPH PLOT SCALE

```

SUBBASIN RUNOFF DATA

```

67 BA      SUBBASIN CHARACTERISTICS
            TAREA      .26 SUBBASIN AREA

```

PRECIPITATION DATA

```

27 PB      STORM      3.00 BASIN TOTAL PRECIPITATION

```

```

28 PI      INCREMENTAL PRECIPITATION PATTERN
            .02      .04      .01      .02      .02      .02      .01      .00      .00      .00
            .00      .00      .00      .01      .00      .01      .01      .01      .01      .01
            .01      .00      .00      .00      .00      .01      .02      .01      .01      .00
            .01      .01      .01      .00      .00      .01      .03      .03      .06      .09
            .09      .12      .03      .04      .03      .01      .02      .02      .00      .00
            .01      .01      .01      .02      .02      .01      .01      .02      .01      .01
            .00      .00      .00      .00      .00      .00      .00      .00      .00      .00

```

```

68 LS      SCS LOSS RATE
            STRTL      .27 INITIAL ABSTRACTION
            CRVNR      88.00 CURVE NUMBER
            RTIMP      .00 PERCENT IMPERVIOUS AREA

```

```

69 UD      SCS DIMENSIONLESS UNITGRAPH
            TLAG      .42 LAG

```

```

UNIT HYDROGRAPH
27 END-OF-PERIOD ORDINATES
23. 71. 148. 228. 267. 267. 238. 197. 143. 104.
77. 59. 43. 32. 24. 18. 13. 10. 7. 6.
4. 3. 2. 2. 1. 1. 0.

```

```

TOTAL RAINFALL = 3.00, TOTAL LOSS = 1.18, TOTAL EXCESS = 1.82
PEAK FLOW      TIME      6-HR      MAXIMUM AVERAGE FLOW
+ (CFS)        (HR)      24-HR      72-HR      24.92-HR
+ 251.         3.83      (CFS)
                        50.          13.          12.          12.
                        (INCHES) 1.814 1.818 1.818 1.818
                        (AC-FT) 25.    25.    25.    25.
CUMULATIVE AREA = .26 SQ MI

```

*** *** *** *** ***

```

HYDROGRAPH AT STATION OFF5
FOR PLAN 1, RATIO = 1.00

```

```

TOTAL RAINFALL = 3.00, TOTAL LOSS = 1.18, TOTAL EXCESS = 1.82
PEAK FLOW      TIME      6-HR      MAXIMUM AVERAGE FLOW
+ (CFS)        (HR)      24-HR      72-HR      24.92-HR
+ 251.         3.83      (CFS)
                        50.          13.          12.          12.
                        (INCHES) 1.814 1.818 1.818 1.818
                        (AC-FT) 25.    25.    25.    25.
CUMULATIVE AREA = .26 SQ MI

```

*** *** *** *** ***

```

HYDROGRAPH AT STATION OFF5
FOR PLAN 1, RATIO = .98

```

```

TOTAL RAINFALL = 2.94, TOTAL LOSS = 1.18, TOTAL EXCESS = 1.76
PEAK FLOW      TIME      6-HR      MAXIMUM AVERAGE FLOW
+ (CFS)        (HR)      24-HR      72-HR      24.92-HR
+ 244.         3.83      (CFS)
                        49.          12.          12.          12.
                        (INCHES) 1.761 1.765 1.765 1.765
                        (AC-FT) 24.    24.    24.    24.
CUMULATIVE AREA = .26 SQ MI

```

```

*****
*          *
70 KK      * OFF6 *
*          *
*****

```

73 KO OUTPUT CONTROL VARIABLES PRO1.OUT
 IPRNT 3 PRINT CONTROL
 IPLOT 0 PLOT CONTROL
 QSCAL 0. HYDROGRAPH PLOT SCALE

SUBBASIN RUNOFF DATA

74 BA SUBBASIN CHARACTERISTICS
 TAREA .04 SUBBASIN AREA

PRECIPITATION DATA

27 PB STORM 3.00 BASIN TOTAL PRECIPITATION

28 PI INCREMENTAL PRECIPITATION PATTERN
 .02 .04 .01 .02 .02 .01 .00 .00 .00
 .00 .00 .00 .01 .00 .01 .01 .01 .01
 .01 .00 .00 .00 .00 .01 .02 .01 .01
 .01 .01 .01 .00 .00 .01 .03 .03 .06 .09
 .09 .12 .03 .04 .03 .01 .02 .02 .00 .00
 .01 .01 .01 .02 .02 .01 .01 .02 .01 .01
 .00 .00 .00 .00 .00 .00 .00 .00 .01 .01
 .00 .00 .00 .00 .00 .00 .00 .00 .00 .00

75 LS SCS LOSS RATE
 STRTL .27 INITIAL ABSTRACTION
 CRVNR 88.00 CURVE NUMBER
 RTIMP .00 PERCENT IMPERVIOUS AREA

76 UD SCS DIMENSIONLESS UNITGRAPH
 TLAG .12 LAG

UNIT HYDROGRAPH
 9 END-OF--PERIOD ORDINATES
 14. 6. 3. 1. 0.
 59. 119. 76. 31. 14. 6. 3. 1. 0.
 TOTAL RAINFALL = 3.00, TOTAL LOSS = 1.18, TOTAL EXCESS = 1.82
 PEAK FLOW TIME 6-HR MAXIMUM AVERAGE FLOW 24.92-HR
 + (CFS) (HR) (CFS) 24-HR 72-HR
 + 62. 3.58 (CFS) 2. 2. 2.
 (INCHES) 1.818 1.818 1.818 1.818
 (AC-FT) 4. 4. 4. 4.
 CUMULATIVE AREA = .04 SQ MI

*** *** *** *** ***

HYDROGRAPH AT STATION OFF6
 FOR PLAN 1, RATIO = 1.00
 TOTAL RAINFALL = 3.00, TOTAL LOSS = 1.18, TOTAL EXCESS = 1.82
 PEAK FLOW TIME 6-HR MAXIMUM AVERAGE FLOW 24.92-HR
 + (CFS) (HR) (CFS) 24-HR 72-HR
 + 62. 3.58 (CFS) 2. 2. 2.
 (INCHES) 1.818 1.818 1.818 1.818
 (AC-FT) 4. 4. 4. 4.
 CUMULATIVE AREA = .04 SQ MI

*** *** *** *** ***

HYDROGRAPH AT STATION OFF6
 FOR PLAN 1, RATIO = .98
 TOTAL RAINFALL = 2.94, TOTAL LOSS = 1.18, TOTAL EXCESS = 1.76
 PEAK FLOW TIME 6-HR MAXIMUM AVERAGE FLOW 24.92-HR
 + (CFS) (HR) (CFS) 24-HR 72-HR
 + 61. 3.58 (CFS) 2. 2. 2.
 (INCHES) 1.765 1.765 1.765 1.765
 (AC-FT) 4. 4. 4. 4.
 CUMULATIVE AREA = .04 SQ MI

1

PEAK FLOW AND STAGE (END-OF-PERIOD) SUMMARY FOR MULTIPLE PLAN-RATIO ECONOMIC COMPUTATIONS
 FLOWS IN CUBIC FEET PER SECOND, AREA IN SQUARE MILES
 TIME TO PEAK IN HOURS

OPERATION	STATION	AREA	PLAN	RATIOS APPLIED TO PRECIPITATION	
				RATIO 1	RATIO 2
				1.00	.98
HYDROGRAPH AT	OFF1	.43	1	399.	387.
+			FLOW	3.83	3.83
TIME					
HYDROGRAPH AT	OFF2	.04	1	49.	48.
+			FLOW	3.58	3.58
TIME					
HYDROGRAPH AT	OFF3	.03	1	40.	39.
+			FLOW	3.58	3.58
TIME					
3 COMBINED AT	CP1	.50	1	440.	428.
+			FLOW	3.83	3.83
TIME					
HYDROGRAPH AT	OFF4	.06	1	90.	87.
+			FLOW	3.58	3.58
TIME					
HYDROGRAPH AT	OFF5	.26	1	251.	244.
+			FLOW	3.83	3.83
TIME					
HYDROGRAPH AT	OFF6	.04	1	62.	61.
+			FLOW	3.58	3.58
TIME					
HYDROGRAPH AT	ON1	.03	1	49.	48.
+			FLOW	3.50	3.50
TIME					

PRO1.OUT

5 COMBINED AT						
+	DB1	.88	1	FLOW	784.	761.
				TIME	3.75	3.75
ROUTED TO						
+	RDB1	.88	1	FLOW	71.	71.
				TIME	5.67	5.58
				** PEAK STAGES IN FEET **		
			1	STAGE	3035.27	3034.99
				TIME	5.67	5.58

*** NORMAL END OF HEC-1 ***

Ultimate Condition HEC-1 Output

```

*****
* FLOOD HYDROGRAPH PACKAGE (HEC-1)
* JUN 1998
* VERSION 4.1
* RUN DATE 09AUG16 TIME 15:58:04
*****

```

ULT1.OUT

```

*****
* U.S. ARMY CORPS OF ENGINEERS
* HYDROLOGIC ENGINEERING CENTER
* 609 SECOND STREET
* DAVIS, CALIFORNIA 95616
* (916) 756-1104
*****

```

```

      X  X  XXXXXX  XXXXX  X
      X  X  X      X      XX
      X  X  X      X      X
      XXXXXX XXXX  X      XXXX
      X  X  X      X      X
      X  X  X      X      X
      X  X  XXXXXX  XXXXX  XXX

```

THIS PROGRAM REPLACES ALL PREVIOUS VERSIONS OF HEC-1 KNOWN AS HEC1 (JAN 73), HECIGS, HECIDB, AND HEC1KW.
 THE DEFINITIONS OF VARIABLES -RTIMP- AND -RTIOR- HAVE CHANGED FROM THOSE USED WITH THE 1973-STYLE INPUT STRUCTURE.
 THE DEFINITION OF -AMSK- ON RM-CARD WAS CHANGED WITH REVISIONS DATED 28 SEP 81. THIS IS THE FORTRAN77 VERSION
 NEW OPTIONS: DAMBREAK OUTFLOW SUBMERGENCE, SINGLE EVENT DAMAGE CALCULATION, DSS:WRITE STAGE FREQUENCY,
 DSS:READ TIME SERIES AT DESIRED CALCULATION INTERVAL LOSS RATE:GREEN AND AMPT INFILTRATION
 KINEMATIC WAVE: NEW FINITE DIFFERENCE ALGORITHM

1 HEC-1 INPUT PAGE 1

```

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10
*DIAGRAM
ID SLATER HANIFAN GROUP
ID NINETY-FIVE MANAGEMENT, LLC.
ID SKYE CANYON - DETENTION BASIN #1
ID UPDATE (SHEEP MOUNTAIN)
ID ULTIMATE DRAINAGE CONDITION
ID
ID RETURN PERIOD ----- 100-YEAR
ID DISTRIBUTION ----- 6-HOUR SDN3
ID PROJECT NO. ----- OLY1401.004
ID FILENAME ----- ULT1.H1
ID DATE ----- 7/25/2016
ID MODELED BY ----- SMJ
ID
ID SIX HOUR DEPTH-AREA REDUCTION FACTORS
ID DRAINAGE AREA (MI^2) RATIO
ID 0.0-0.49 1.00
ID 0.5-0.99 0.98
ID
ID IT 5 0 0 500
ID IO 5 0 0
ID IN 5 0 0
ID JR PREC 1.00 0.98
*
23 KK OFF1
24 KM EXISTING OFFSITE DRAINAGE BASIN
25 KM DISCHARGES INTO DETENTION BASIN "DB1"
26 PB 3.00
27 PC 0 0.020 0.057 0.070 0.087 0.108 0.124 0.130 0.130 0.130
28 PC 0.130 0.130 0.130 0.133 0.140 0.142 0.148 0.158 0.172 0.181
29 PC 0.190 0.197 0.199 0.200 0.201 0.204 0.214 0.229 0.241 0.249
30 PC 0.251 0.256 0.270 0.278 0.281 0.283 0.295 0.322 0.352 0.409
31 PC 0.499 0.590 0.710 0.744 0.781 0.812 0.819 0.835 0.851 0.856
32 PC 0.860 0.868 0.876 0.888 0.910 0.926 0.937 0.950 0.970 0.976
33 PC 0.982 0.985 0.987 0.989 0.990 0.993 0.993 0.994 0.995 0.998
34 PC 0.998 0.999 1
35 BA .4073
36 LS 0
37 UD 0.356
*
38 KK OFF7
39 KM FUTURE DEVELOPED OFFSITE DRAINAGE BASIN
40 KM DISCHARGES INTO DETENTION BASIN "DB1"
41 BA .0281
42 LS 0 96.0
43 UD 0.118
*

```

1 HEC-1 INPUT PAGE 2

```

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10
44 KK CP2
45 KM COMBINE OFF1, OFF7
46 HC 2
*
47 KK OFF2
48 KM EXISTING OFFSITE DRAINAGE BASIN
49 KM DISCHARGES INTO DETENTION BASIN "DB1"
50 BA .0355
51 LS 0 81.8
52 UD 0.148
*
53 KK OFF3
54 KM EXISTING OFFSITE DRAINAGE BASIN
55 KM DISCHARGES INTO DETENTION BASIN "DB1"
56 BA .0259
57 LS 0 88.0
58 UD 0.112
*
59 KK CP1
60 KM COMBINE CP2, OFF2, AND OFF3
61 HC 3
*
62 KK OFF4
63 KM EXISTING OFFSITE DRAINAGE BASIN
64 KM DISCHARGES INTO DETENTION BASIN "DB1"
65 BA .0581
66 LS 0 88.0
67 UD 0.136
*
68 KK OFF5
69 KM EXISTING OFFSITE DRAINAGE BASIN
70 KM DISCHARGES INTO DETENTION BASIN "DB1"

```

ULT1.OUT

```

71      BA .2569
72      LS 0      88.0
73      UD 0.419
      *
74      KK OFF6
75      KM EXISTING OFFSITE DRAINAGE BASIN
76      KM DISCHARGES INTO DETENTION BASIN "DB1"
77      BA .0399
78      LS 0      88.0
79      UD 0.121
      *

```

1 HEC-1 INPUT PAGE 3

```

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10
80      KK ON1
81      KM PROPOSED ONSITE DRAINAGE BASIN
82      BA .0261
83      LS 0      94.0
84      UD 0.116
      *
85      KK DB1
86      KM COMBINE CP1, OFF4, OFF5, OFF6 AND ON1
87      KO 1
88      HC 5
      *
89      KK RDB1
90      KM OUTLET = 24" ORIFICE @ INVERT ELEVATION = 3013.8-FT
91      KO 1
92      RS 1      ELEV 3025.5
93      SA 4.58 5.63 5.81 6.00 6.19 6.77 7.17 7.37 7.61 8.31
94      SE 3025.5 3027 3028 3029 3030 3033 3035 3036 3037 3040
95      SQ 49.7 60.6 70.8 74.5 1436.0 3920.5 7136.3
96      SE 3025.5 3030 3035 3037 3040 3043 3046
97      ZZ

```

1 SCHEMATIC DIAGRAM OF STREAM NETWORK

```

INPUT LINE (V) ROUTING (--->) DIVERSION OR PUMP FLOW
NO. (.) CONNECTOR (<---) RETURN OF DIVERTED OR PUMPED FLOW

```

```

23      OFF1
      .
38      .      OFF7
      .
44      CP2.....
      .
47      .      OFF2
      .
53      .      OFF3
      .
59      CP1.....
      .
62      .      OFF4
      .
68      .      OFF5
      .
74      .      OFF6
      .
80      .      ON1
      .
85      DB1.....
      V
89      RDB1

```

(***) RUNOFF ALSO COMPUTED AT THIS LOCATION

```

*****
* FLOOD HYDROGRAPH PACKAGE (HEC-1) *
* JUN 1998 *
* VERSION 4.1 *
* RUN DATE 09AUG16 TIME 15:58:04 *
*****

```

```

*****
* U.S. ARMY CORPS OF ENGINEERS *
* HYDROLOGIC ENGINEERING CENTER *
* 609 SECOND STREET *
* DAVIS, CALIFORNIA 95616 *
* (916) 756-1104 *
*****

```

SLATER HANIFAN GROUP
NINETY-FIVE MANAGEMENT, LLC.
SKYE CANYON - DETENTION BASIN #1
UPDATE (SHEEP MOUNTAIN)
ULTIMATE DRAINAGE CONDITION

RETURN PERIOD ----- 100-YEAR
DISTRIBUTION ----- 6-HOUR SDN3
PROJECT NO. ----- OLY1401.004
FILENAME ----- ULT1.H1
DATE ----- 7/25/2016
MODELED BY ----- SMJ

SIX HOUR DEPTH-AREA REDUCTION FACTORS
DRAINAGE AREA (MI^2) RATIO
0.0-0.49 1.00
0.5-0.99 0.98

20 IO OUTPUT CONTROL VARIABLES

```

IPRNT 5 PRINT CONTROL
IPLOT 0 PLOT CONTROL
QSCAL 0. HYDROGRAPH PLOT SCALE

```

IT HYDROGRAPH TIME DATA

```

NMIN 5 MINUTES IN COMPUTATION INTERVAL
IDATE 1 0 STARTING DATE
ITIME 0000 STARTING TIME
NQ 500 NUMBER OF HYDROGRAPH ORDINATES
NBDATE 2 0 ENDING DATE
NDTIME 1735 ENDING TIME
ICENT 19 CENTURY MARK

```

ULT1.OUT

COMPUTATION INTERVAL .08 HOURS
TOTAL TIME BASE 41.58 HOURS

ENGLISH UNITS
DRAINAGE AREA SQUARE MILES
PRECIPITATION DEPTH INCHES
LENGTH, ELEVATION FEET
FLOW CUBIC FEET PER SECOND
STORAGE VOLUME ACRE-Feet
SURFACE AREA ACRES
TEMPERATURE DEGREES FAHRENHEIT

JP MULTI-PLAN OPTION
NPLAN 1 NUMBER OF PLANS

JR MULTI-RATIO OPTION
RATIOS OF PRECIPITATION
1.00 .98

85 KK DB1

87 KO OUTPUT CONTROL VARIABLES
IPRNT 1 PRINT CONTROL
IPLOT 0 PLOT CONTROL
QSCAL 0. HYDROGRAPH PLOT SCALE

88 HC HYDROGRAPH COMBINATION
ICOMP 5 NUMBER OF HYDROGRAPHS TO COMBINE

HYDROGRAPH AT STATION DB1
SUM OF 5 HYDROGRAPHS
PLAN 1, RATIO = 1.00

DA	MON	HRMN	ORD	FLOW	DA	MON	HRMN	ORD	FLOW	DA	MON	HRMN	ORD	FLOW	DA	MON	HRMN	ORD	FLOW
1	0000	1	0.	1	1025	126	0.	1	2050	251	0.	2	0715	376	0.				
1	0005	2	0.	1	1030	127	0.	1	2055	252	0.	2	0720	377	0.				
1	0010	3	1.	1	1035	128	0.	1	2100	253	0.	2	0725	378	0.				
1	0015	4	2.	1	1040	129	0.	1	2105	254	0.	2	0730	379	0.				
1	0020	5	4.	1	1045	130	0.	1	2110	255	0.	2	0735	380	0.				
1	0025	6	7.	1	1050	131	0.	1	2115	256	0.	2	0740	381	0.				
1	0030	7	11.	1	1055	132	0.	1	2120	257	0.	2	0745	382	0.				
1	0035	8	13.	1	1100	133	0.	1	2125	258	0.	2	0750	383	0.				
1	0040	9	11.	1	1105	134	0.	1	2130	259	0.	2	0755	384	0.				
1	0045	10	8.	1	1110	135	0.	1	2135	260	0.	2	0800	385	0.				
1	0050	11	6.	1	1115	136	0.	1	2140	261	0.	2	0805	386	0.				
1	0055	12	5.	1	1120	137	0.	1	2145	262	0.	2	0810	387	0.				
1	0100	13	4.	1	1125	138	0.	1	2150	263	0.	2	0815	388	0.				
1	0105	14	4.	1	1130	139	0.	1	2155	264	0.	2	0820	389	0.				
1	0110	15	6.	1	1135	140	0.	1	2200	265	0.	2	0825	390	0.				
1	0115	16	8.	1	1140	141	0.	1	2205	266	0.	2	0830	391	0.				
1	0120	17	9.	1	1145	142	0.	1	2210	267	0.	2	0835	392	0.				
1	0125	18	13.	1	1150	143	0.	1	2215	268	0.	2	0840	393	0.				
1	0130	19	21.	1	1155	144	0.	1	2220	269	0.	2	0845	394	0.				
1	0135	20	28.	1	1200	145	0.	1	2225	270	0.	2	0850	395	0.				
1	0140	21	34.	1	1205	146	0.	1	2230	271	0.	2	0855	396	0.				
1	0145	22	38.	1	1210	147	0.	1	2235	272	0.	2	0900	397	0.				
1	0150	23	39.	1	1215	148	0.	1	2240	273	0.	2	0905	398	0.				
1	0155	24	36.	1	1220	149	0.	1	2245	274	0.	2	0910	399	0.				
1	0200	25	32.	1	1225	150	0.	1	2250	275	0.	2	0915	400	0.				
1	0205	26	29.	1	1230	151	0.	1	2255	276	0.	2	0920	401	0.				
1	0210	27	28.	1	1235	152	0.	1	2300	277	0.	2	0925	402	0.				
1	0215	28	38.	1	1240	153	0.	1	2305	278	0.	2	0930	403	0.				
1	0220	29	48.	1	1245	154	0.	1	2310	279	0.	2	0935	404	0.				
1	0225	30	56.	1	1250	155	0.	1	2315	280	0.	2	0940	405	0.				
1	0230	31	57.	1	1255	156	0.	1	2320	281	0.	2	0945	406	0.				
1	0235	32	61.	1	1300	157	0.	1	2325	282	0.	2	0950	407	0.				
1	0240	33	61.	1	1305	158	0.	1	2330	283	0.	2	0955	408	0.				
1	0245	34	69.	1	1310	159	0.	1	2335	284	0.	2	1000	409	0.				
1	0250	35	68.	1	1315	160	0.	1	2340	285	0.	2	1005	410	0.				
1	0255	36	62.	1	1320	161	0.	1	2345	286	0.	2	1010	411	0.				
1	0300	37	62.	1	1325	162	0.	1	2350	287	0.	2	1015	412	0.				
1	0305	38	80.	1	1330	163	0.	1	2355	288	0.	2	1020	413	0.				
1	0310	39	114.	1	1335	164	0.	2	0000	289	0.	2	1025	414	0.				
1	0315	40	173.	1	1340	165	0.	2	0005	290	0.	2	1030	415	0.				
1	0320	41	287.	1	1345	166	0.	2	0010	291	0.	2	1035	416	0.				
1	0325	42	446.	1	1350	167	0.	2	0015	292	0.	2	1040	417	0.				
1	0330	43	640.	1	1355	168	0.	2	0020	293	0.	2	1045	418	0.				
1	0335	44	789.	1	1400	169	0.	2	0025	294	0.	2	1050	419	0.				
1	0340	45	833.	1	1405	170	0.	2	0030	295	0.	2	1055	420	0.				
1	0345	46	842.	1	1410	171	0.	2	0035	296	0.	2	1100	421	0.				
1	0350	47	803.	1	1415	172	0.	2	0040	297	0.	2	1105	422	0.				
1	0355	48	718.	1	1420	173	0.	2	0045	298	0.	2	1110	423	0.				
1	0400	49	630.	1	1425	174	0.	2	0050	299	0.	2	1115	424	0.				
1	0405	50	534.	1	1430	175	0.	2	0055	300	0.	2	1120	425	0.				
1	0410	51	437.	1	1435	176	0.	2	0100	301	0.	2	1125	426	0.				
1	0415	52	358.	1	1440	177	0.	2	0105	302	0.	2	1130	427	0.				
1	0420	53	300.	1	1445	178	0.	2	0110	303	0.	2	1135	428	0.				
1	0425	54	260.	1	1450	179	0.	2	0115	304	0.	2	1140	429	0.				
1	0430	55	245.	1	1455	180	0.	2	0120	305	0.	2	1145	430	0.				
1	0435	56	247.	1	1500	181	0.	2	0125	306	0.	2	1150	431	0.				
1	0440	57	245.	1	1505	182	0.	2	0130	307	0.	2	1155	432	0.				
1	0445	58	242.	1	1510	183	0.	2	0135	308	0.	2	1200	433	0.				
1	0450	59	249.	1	1515	184	0.	2	0140	309	0.	2	1205	434	0.				
1	0455	60	252.	1	1520	185	0.	2	0145	310	0.	2	1210	435	0.				
1	0500	61	236.	1	1525	186	0.	2	0150	311	0.	2	1215	436	0.				
1	0505	62	212.	1	1530	187	0.	2	0155	312	0.	2	1220	437	0.				
1	0510	63	185.	1	1535	188	0.	2	0200	313	0.	2	1225	438	0.				
1	0515	64	157.	1	1540	189	0.	2	0205	314	0.	2	1230	439	0.				
1	0520	65	130.	1	1545	190	0.	2	0210	315	0.	2	1235	440	0.				
1	0525	66	106.	1	1550	191	0.	2	0215	316	0.	2	1240	441	0.				
1	0530	67	86.	1	1555	192	0.	2	0220	317	0.	2	1245	442	0.				
1	0535	68	68.	1	1600	193	0.	2	0225	318	0.	2	1250	443	0.				
1	0540	69	55.	1	1605	194	0.	2	0230	319	0.	2	1255	444	0.				
1	0545	70	47.	1	1610	195	0.	2	0235	320	0.	2	1300	445	0.				
1	0550	71	42.	1	1615	196	0.	2	0240	321	0.	2	1305	446	0.				
1	0555	72	35.	1	1620	197	0.	2	0245	322	0.	2	1310	447	0.				
1	0600	73	31.	1	1625	198	0.	2	0250	323	0.	2	1315	448	0.				
1	0605	74	27.	1	1630	199	0.	2	0255	324	0.	2	1320	449	0.				
1	0610	75	22.	1	1635	200	0.	2	0300	325	0.	2	1325	450	0.				
1	0615	76	17.	1	1640	201	0.	2	0305	326	0.	2	1330	451	0.				
1	0620	77	14.	1	1645	202	0.	2	0310	327	0.	2	1335	452	0.				
1	0625	78	10.	1	1650	203	0.	2	0315	328	0.	2	1340	453	0.				

															ULT1.OUT				
1	0450	59	243.	*	1	1515	184	0.	*	2	0140	309	0.	*	2	1205	434	0.	
1	0455	60	246.	*	1	1520	185	0.	*	2	0145	310	0.	*	2	1210	435	0.	
1	0500	61	230.	*	1	1525	186	0.	*	2	0150	311	0.	*	2	1215	436	0.	
1	0505	62	207.	*	1	1530	187	0.	*	2	0155	312	0.	*	2	1220	437	0.	
1	0510	63	181.	*	1	1535	188	0.	*	2	0200	313	0.	*	2	1225	438	0.	
1	0515	64	153.	*	1	1540	189	0.	*	2	0205	314	0.	*	2	1230	439	0.	
1	0520	65	127.	*	1	1545	190	0.	*	2	0210	315	0.	*	2	1235	440	0.	
1	0525	66	103.	*	1	1550	191	0.	*	2	0215	316	0.	*	2	1240	441	0.	
1	0530	67	84.	*	1	1555	192	0.	*	2	0220	317	0.	*	2	1245	442	0.	
1	0535	68	67.	*	1	1600	193	0.	*	2	0225	318	0.	*	2	1250	443	0.	
1	0540	69	54.	*	1	1605	194	0.	*	2	0230	319	0.	*	2	1255	444	0.	
1	0545	70	46.	*	1	1610	195	0.	*	2	0235	320	0.	*	2	1300	445	0.	
1	0550	71	41.	*	1	1615	196	0.	*	2	0240	321	0.	*	2	1305	446	0.	
1	0555	72	34.	*	1	1620	197	0.	*	2	0245	322	0.	*	2	1310	447	0.	
1	0600	73	30.	*	1	1625	198	0.	*	2	0250	323	0.	*	2	1315	448	0.	
1	0605	74	26.	*	1	1630	199	0.	*	2	0255	324	0.	*	2	1320	449	0.	
1	0610	75	21.	*	1	1635	200	0.	*	2	0300	325	0.	*	2	1325	450	0.	
1	0615	76	17.	*	1	1640	201	0.	*	2	0305	326	0.	*	2	1330	451	0.	
1	0620	77	13.	*	1	1645	202	0.	*	2	0310	327	0.	*	2	1335	452	0.	
1	0625	78	10.	*	1	1650	203	0.	*	2	0315	328	0.	*	2	1340	453	0.	
1	0630	79	7.	*	1	1655	204	0.	*	2	0320	329	0.	*	2	1345	454	0.	
1	0635	80	5.	*	1	1700	205	0.	*	2	0325	330	0.	*	2	1350	455	0.	
1	0640	81	4.	*	1	1705	206	0.	*	2	0330	331	0.	*	2	1355	456	0.	
1	0645	82	3.	*	1	1710	207	0.	*	2	0335	332	0.	*	2	1400	457	0.	
1	0650	83	2.	*	1	1715	208	0.	*	2	0340	333	0.	*	2	1405	458	0.	
1	0655	84	1.	*	1	1720	209	0.	*	2	0345	334	0.	*	2	1410	459	0.	
1	0700	85	1.	*	1	1725	210	0.	*	2	0350	335	0.	*	2	1415	460	0.	
1	0705	86	1.	*	1	1730	211	0.	*	2	0355	336	0.	*	2	1420	461	0.	
1	0710	87	0.	*	1	1735	212	0.	*	2	0400	337	0.	*	2	1425	462	0.	
1	0715	88	0.	*	1	1740	213	0.	*	2	0405	338	0.	*	2	1430	463	0.	
1	0720	89	0.	*	1	1745	214	0.	*	2	0410	339	0.	*	2	1435	464	0.	
1	0725	90	0.	*	1	1750	215	0.	*	2	0415	340	0.	*	2	1440	465	0.	
1	0730	91	0.	*	1	1755	216	0.	*	2	0420	341	0.	*	2	1445	466	0.	
1	0735	92	0.	*	1	1800	217	0.	*	2	0425	342	0.	*	2	1450	467	0.	
1	0740	93	0.	*	1	1805	218	0.	*	2	0430	343	0.	*	2	1455	468	0.	
1	0745	94	0.	*	1	1810	219	0.	*	2	0435	344	0.	*	2	1500	469	0.	
1	0750	95	0.	*	1	1815	220	0.	*	2	0440	345	0.	*	2	1505	470	0.	
1	0755	96	0.	*	1	1820	221	0.	*	2	0445	346	0.	*	2	1510	471	0.	
1	0800	97	0.	*	1	1825	222	0.	*	2	0450	347	0.	*	2	1515	472	0.	
1	0805	98	0.	*	1	1830	223	0.	*	2	0455	348	0.	*	2	1520	473	0.	
1	0810	99	0.	*	1	1835	224	0.	*	2	0500	349	0.	*	2	1525	474	0.	
1	0815	100	0.	*	1	1840	225	0.	*	2	0505	350	0.	*	2	1530	475	0.	
1	0820	101	0.	*	1	1845	226	0.	*	2	0510	351	0.	*	2	1535	476	0.	
1	0825	102	0.	*	1	1850	227	0.	*	2	0515	352	0.	*	2	1540	477	0.	
1	0830	103	0.	*	1	1855	228	0.	*	2	0520	353	0.	*	2	1545	478	0.	
1	0835	104	0.	*	1	1900	229	0.	*	2	0525	354	0.	*	2	1550	479	0.	
1	0840	105	0.	*	1	1905	230	0.	*	2	0530	355	0.	*	2	1555	480	0.	
1	0845	106	0.	*	1	1910	231	0.	*	2	0535	356	0.	*	2	1600	481	0.	
1	0850	107	0.	*	1	1915	232	0.	*	2	0540	357	0.	*	2	1605	482	0.	
1	0855	108	0.	*	1	1920	233	0.	*	2	0545	358	0.	*	2	1610	483	0.	
1	0900	109	0.	*	1	1925	234	0.	*	2	0550	359	0.	*	2	1615	484	0.	
1	0905	110	0.	*	1	1930	235	0.	*	2	0555	360	0.	*	2	1620	485	0.	
1	0910	111	0.	*	1	1935	236	0.	*	2	0600	361	0.	*	2	1625	486	0.	
1	0915	112	0.	*	1	1940	237	0.	*	2	0605	362	0.	*	2	1630	487	0.	
1	0920	113	0.	*	1	1945	238	0.	*	2	0610	363	0.	*	2	1635	488	0.	
1	0925	114	0.	*	1	1950	239	0.	*	2	0615	364	0.	*	2	1640	489	0.	
1	0930	115	0.	*	1	1955	240	0.	*	2	0620	365	0.	*	2	1645	490	0.	
1	0935	116	0.	*	1	2000	241	0.	*	2	0625	366	0.	*	2	1650	491	0.	
1	0940	117	0.	*	1	2005	242	0.	*	2	0630	367	0.	*	2	1655	492	0.	
1	0945	118	0.	*	1	2010	243	0.	*	2	0635	368	0.	*	2	1700	493	0.	
1	0950	119	0.	*	1	2015	244	0.	*	2	0640	369	0.	*	2	1705	494	0.	
1	0955	120	0.	*	1	2020	245	0.	*	2	0645	370	0.	*	2	1710	495	0.	
1	1000	121	0.	*	1	2025	246	0.	*	2	0650	371	0.	*	2	1715	496	0.	
1	1005	122	0.	*	1	2030	247	0.	*	2	0655	372	0.	*	2	1720	497	0.	
1	1010	123	0.	*	1	2035	248	0.	*	2	0700	373	0.	*	2	1725	498	0.	
1	1015	124	0.	*	1	2040	249	0.	*	2	0705	374	0.	*	2	1730	499	0.	
1	1020	125	0.	*	1	2045	250	0.	*	2	0710	375	0.	*	2	1735	500	0.	

PEAK FLOW	TIME	6-HR	MAXIMUM AVERAGE FLOW	41.58-HR
(CFS)	(HR)		24-HR	72-HR
818.	3.75	164.	41.	24.
		1.736	1.743	1.743
		81.	82.	82.
		(INCHES)		
		(AC-FT)		

CUMULATIVE AREA = .88 SQ MI

*** **

89 KK *****
*
* RDB1 *
*

91 KO OUTPUT CONTROL VARIABLES
IPRINT 1 PRINT CONTROL
IPLOT 0 PLOT CONTROL
QSCAL 0 HYDROGRAPH PLOT SCALE

HYDROGRAPH ROUTING DATA
92 RS STORAGE ROUTING
NSTPS 1 NUMBER OF SUBREACHES
ITYP ELEV TYPE OF INITIAL CONDITION
RSVRIC 3025.50 INITIAL CONDITION
X .00 WORKING R AND D COEFFICIENT

	AREA	4.6	5.6	5.8	6.0	6.2	6.8	7.2	7.4	7.6	8.3
94 SE	ELEVATION	3025.50	3027.00	3028.00	3029.00	3030.00	3033.00	3035.00	3036.00	3037.00	3040.00
95 SQ	DISCHARGE	50.	61.	71.	75.	1436.	3921.	7136.			
96 SE	ELEVATION	3025.50	3030.00	3035.00	3037.00	3040.00	3043.00	3046.00			

COMPUTED STORAGE-ELEVATION DATA

STORAGE	.00	7.64	13.36	19.27	25.36	44.80	58.73	66.00	73.49	97.37
ELEVATION	3025.50	3027.00	3028.00	3029.00	3030.00	3033.00	3035.00	3036.00	3037.00	3040.00

COMPUTED STORAGE-OUTFLOW-ELEVATION DATA

STORAGE	.00	7.64	13.36	19.27	25.36	44.80	58.73	66.00	73.49	97.37
OUTFLOW	49.70	53.33	55.76	58.18	60.60	66.72	70.80	72.65	74.50	1436.00
ELEVATION	3025.50	3027.00	3028.00	3029.00	3030.00	3033.00	3035.00	3036.00	3037.00	3040.00

+	71.	5.58	(INCHES) (AC-FT)	68. .723 34.	57. 2.417 113.	54. 3.960 185.	54. 3.960 185.	ULT1.0UT
PEAK STORAGE	TIME			6-HR	MAXIMUM AVERAGE 24-HR	STORAGE 72-HR		
+	(AC-FT) 60.	(HR) 5.58		50.	19.	11.	11.	
PEAK STAGE	TIME			6-HR	MAXIMUM AVERAGE 24-HR	STAGE 72-HR		
+	(FEET) 3035.14	(HR) 5.58		3033.74	3028.70	3027.35	3027.35	
CUMULATIVE AREA =				.88 SQ MI				

1

PEAK FLOW AND STAGE (END-OF-PERIOD) SUMMARY FOR MULTIPLE PLAN-RATIO ECONOMIC COMPUTATIONS
 FLOWS IN CUBIC FEET PER SECOND, AREA IN SQUARE MILES
 TIME TO PEAK IN HOURS

OPERATION	STATION	AREA	PLAN	RATIOS APPLIED TO PRECIPITATION	
				RATIO 1 1.00	RATIO 2 .98
HYDROGRAPH AT					
+	OFF1	.41	1 FLOW TIME	410. 3.75	397. 3.75
HYDROGRAPH AT					
+	OFF7	.03	1 FLOW TIME	55. 3.50	54. 3.50
2 COMBINED AT					
+	CP2	.44	1 FLOW TIME	439. 3.75	426. 3.75
HYDROGRAPH AT					
+	OFF2	.04	1 FLOW TIME	42. 3.58	41. 3.58
HYDROGRAPH AT					
+	OFF3	.03	1 FLOW TIME	40. 3.58	39. 3.58
3 COMBINED AT					
+	CP1	.50	1 FLOW TIME	489. 3.75	475. 3.75
HYDROGRAPH AT					
+	OFF4	.06	1 FLOW TIME	90. 3.58	87. 3.58
HYDROGRAPH AT					
+	OFF5	.26	1 FLOW TIME	251. 3.83	244. 3.83
HYDROGRAPH AT					
+	OFF6	.04	1 FLOW TIME	62. 3.58	61. 3.58
HYDROGRAPH AT					
+	ON1	.03	1 FLOW TIME	49. 3.50	48. 3.50
5 COMBINED AT					
+	DB1	.88	1 FLOW TIME	842. 3.75	818. 3.75
ROUTED TO					
+	RDB1	.88	1 FLOW TIME	72. 5.58	71. 5.58
** PEAK STAGES IN FEET **					
1	STAGE	3035.43	3035.14		
	TIME	5.58	5.58		

*** NORMAL END OF HEC-1 ***

PMF Condition HEC-1 Output

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1*****
*
* FLOOD HYDROGRAPH PACKAGE (HEC-1)
* JUN 1998
* VERSION 4.1
* RUN DATE 09AUG16 TIME 15:58:11
*
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PMF.OUT

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*****
*
* U.S. ARMY CORPS OF ENGINEERS
* HYDROLOGIC ENGINEERING CENTER
* 609 SECOND STREET
* DAVIS, CALIFORNIA 95616
* (916) 756-1104
*
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THIS PROGRAM REPLACES ALL PREVIOUS VERSIONS OF HEC-1 KNOWN AS HEC1 (JAN 73), HEC1GS, HEC1DB, AND HEC1KW.

THE DEFINITIONS OF VARIABLES -RTIMP- AND -RYTOR- HAVE CHANGED FROM THOSE USED WITH THE 1973-STYLE INPUT STRUCTURE.
 THE DEFINITION OF -AMSK- ON RW-CARD WAS CHANGED WITH REVISIONS DATED 28 SEP 81. THIS IS THE FORTRAN77 VERSION
 NEW OPTIONS: DAMBREAK OUTFLOW SUBMERGENCE, SINGLE EVENT DAMAGE CALCULATION, DSS:WRITE STAGE FREQUENCY,
 DSS:READ TIME SERIES AT DESIRED CALCULATION INTERVAL LOSS RATE:GREEN AND AMPT INFILTRATION
 KINEMATIC WAVE: NEW FINITE DIFFERENCE ALGORITHM

1 HEC-1 INPUT PAGE 1

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LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10
*DIAGRAM
1 ID SLATER HANIFAN GROUP
2 ID NINETY-FIVE MANAGEMENT, LLC.
3 ID SKYE CANYON - DETENTION BASIN 1
4 ID UPDATE (SHEEP MOUNTAIN)
5 ID PMF DRAINAGE CONDITION
6 ID
7 ID RETURN PERIOD ----- 100-YEAR
8 ID DISTRIBUTION ----- 6-HOUR SDN3
9 ID PROJECT NO. ----- OLY1401.004
10 ID FILENAME ----- PMF.H1
11 ID DATE ----- 7/25/2016
12 ID MODELED BY ----- ETM
13 ID
14 ID SIX HOUR DEPTH-AREA REDUCTION FACTORS
15 ID DRAINAGE AREA (MI^2) RATIO
16 ID 0.0-0.49 1.00
17 ID 0.5-0.99 0.98
18 ID
19 IT 15 0 0 500
20 IO 5 0 0
21 IN 15 0 0
22 JR PREC 1.00
*
23 KK OFF1
24 KM EXISTING OFFSITE DRAINAGE BASIN
25 KM DISCHARGES INTO DETENTION BASIN "DB1"
26 PB 14.1
27 PC 0.000 0.004 0.007 0.011 0.014 0.023 0.032 0.041 0.050 0.078
28 PC 0.106 0.135 0.163 0.681 0.816 0.872 0.915 0.929 0.943 0.957
29 PC 0.972 0.979 0.986 0.993 1.000
30 BA .4073
31 LS 0 86.8
32 UD 0.356
*
33 KK OFF7
34 KM FUTURE DEVELOPED OFFSITE DRAINAGE BASIN
35 KM DISCHARGES INTO DETENTION BASIN "DB1"
36 BA .0281
37 LS 0 96.0
38 UD 0.118
*
39 KK CP2
40 KM COMBINE OFF1, OFF7
41 HC 2
*

```

1 HEC-1 INPUT PAGE 2

```

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10
42 KK OFF2
43 KM EXISTING OFFSITE DRAINAGE BASIN
44 KM DISCHARGES INTO DETENTION BASIN "DB1"
45 BA .0355
46 LS 0 81.8
47 UD 0.148
*
48 KK OFF3
49 KM EXISTING OFFSITE DRAINAGE BASIN
50 KM DISCHARGES INTO DETENTION BASIN "DB1"
51 BA .0259
52 LS 0 88.0
53 UD 0.112
*
54 KK CP1
55 KM COMBINE CP2, OFF2, AND OFF3
56 HC 3
*
57 KK OFF4
58 KM EXISTING OFFSITE DRAINAGE BASIN
59 KM DISCHARGES INTO DETENTION BASIN "DB1"
60 BA .0581
61 LS 0 88.0
62 UD 0.136
*
63 KK OFF5
64 KM EXISTING OFFSITE DRAINAGE BASIN
65 KM DISCHARGES INTO DETENTION BASIN "DB1"
66 BA .2569
67 LS 0 88.0
68 UD 0.419
*

```

```

69      KK  OFF6
70      KM  EXISTING OFFSITE DRAINAGE BASIN
71      KM  DISCHARGES INTO DETENTION BASIN "DB1"
72      BA  .0399
73      LS  0      88.0
74      UD  0.121
      *
75      KK  ON1
76      KM  PROPOSED ONSITE DRAINAGE BASIN
77      BA  .0261
78      LS  0      94.0
79      UD  0.116
      *

```

PMF.OUT

1 HEC-1 INPUT PAGE 3

```

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

80      KK  DB1
81      KM  COMBINE CP1, OFF4, OFF5, OFF6 AND ON1
82      KO  1
83      HC  5
      *
84      KK  RDB1
85      KM  OUTLET = 24" ORIFICE @ INVERT ELEVATION = 3013.8-FT
86      KM  87-FT WIDE SPILLWAY @ CREST ELEVATION = 3037.0-FT
87      KO  1
88      RS  1      ELEV 3025.5
89      SA  4.58  5.63  5.81  6.00  6.19  7.17  7.61  8.31  9.00  9.69
90      SE  3025.5 3027 3028 3029 3030 3035 3037 3040 3043 3046
91      SQ  49.7  60.6  70.8  74.5 1436.0 3920.5 7136.3
92      SE  3025.5 3030 3035 3037 3040 3043 3046
      *
93      ZZ

```

1 SCHEMATIC DIAGRAM OF STREAM NETWORK

```

INPUT LINE NO. (V) ROUTING (--->) DIVERSION OR PUMP FLOW
              (.) CONNECTOR (<---) RETURN OF DIVERTED OR PUMPED FLOW

23  OFF1
   .
   .
33  OFF7
   .
   .
39  CP2.....
   .
   .
42  OFF2
   .
   .
48  OFF3
   .
   .
54  CP1.....
   .
   .
57  OFF4
   .
   .
63  OFF5
   .
   .
69  OFF6
   .
   .
75  ON1
   .
   .
80  DB1.....
   V
84  RDB1

```

(***) RUNOFF ALSO COMPUTED AT THIS LOCATION

```

*****
* FLOOD HYDROGRAPH PACKAGE (HEC-1) *
* JUN 1998 *
* VERSION 4.1 *
* RUN DATE 09AUG16 TIME 15:58:11 *
*****

```

```

*****
* U.S. ARMY CORPS OF ENGINEERS *
* HYDROLOGIC ENGINEERING CENTER *
* 609 SECOND STREET *
* DAVIS, CALIFORNIA 95616 *
* (916) 756-1104 *
*****

```

SLATER HANIFAN GROUP
NINETY-FIVE MANAGEMENT, LLC.
SKYE CANYON - DETENTION BASIN 1
UPDATE (SHEEP MOUNTAIN)
PMF DRAINAGE CONDITION

RETURN PERIOD ----- 100-YEAR
DISTRIBUTION ----- 6-HOUR SDN3
PROJECT NO. ----- OLY1401.004
FILENAME ----- PMF.H1
DATE ----- 7/25/2016
MODELED BY ----- ETM

SIX HOUR DEPTH-AREA REDUCTION FACTORS
DRAINAGE AREA (MI^2) RATIO
0.0-0.49 1.00
0.5-0.99 0.98

20 IO

OUTPUT CONTROL VARIABLES

```

IPRNT 5 PRINT CONTROL
IPLOT 0 PLOT CONTROL
QSCAL 0. HYDROGRAPH PLOT SCALE

```

IT

HYDROGRAPH TIME DATA

```

NMEN 15 MINUTES IN COMPUTATION INTERVAL
IDATE 1 STARTING DATE
ITIME 0000 STARTING TIME
NQ 500 NUMBER OF HYDROGRAPH ORDINATES
NDDATE 6 ENDING DATE
NDTIME 0445 ENDING TIME
ICENT 19 CENTURY MARK

```

COMPUTATION INTERVAL .25 HOURS
TOTAL TIME BASE 124.75 HOURS

ENGLISH UNITS
DRAINAGE AREA
PRECIPITATION DEPTH
LENGTH, ELEVATION
FLOW
STORAGE VOLUME
SURFACE AREA
TEMPERATURE

SQUARE MILES
INCHES
FEET
CUBIC FEET PER SECOND
ACRE-Feet
ACRES
DEGREES FAHRENHEIT

PMF.OUT

JP MULTI-PLAN OPTION
NPLAN 1 NUMBER OF PLANS

JR MULTI-RATIO OPTION
RATIOS OF PRECIPITATION
1.00

*
80 KK DB1
*

82 KO OUTPUT CONTROL VARIABLES
IPRNT 1 PRINT CONTROL
IPLOT 0 PLOT CONTROL
QSCAL 0. HYDROGRAPH PLOT SCALE

83 HC HYDROGRAPH COMBINATION
ICOMP 5 NUMBER OF HYDROGRAPHS TO COMBINE

HYDROGRAPH AT STATION DB1
SUM OF 5 HYDROGRAPHS
PLAN 1, RATIO = 1.00

DA	MON	HRMN	ORD	FLOW	DA	MON	HRMN	ORD	FLOW	DA	MON	HRMN	ORD	FLOW	DA	MON	HRMN	ORD	FLOW
1	0000	1	0.	2	0715	126	0.	3	1430	251	0.	4	2145	376	0.				
1	0015	2	0.	2	0730	127	0.	3	1445	252	0.	4	2200	377	0.				
1	0030	3	0.	2	0745	128	0.	3	1500	253	0.	4	2215	378	0.				
1	0045	4	1.	2	0800	129	0.	3	1515	254	0.	4	2230	379	0.				
1	0100	5	1.	2	0815	130	0.	3	1530	255	0.	4	2245	380	0.				
1	0115	6	6.	2	0830	131	0.	3	1545	256	0.	4	2300	381	0.				
1	0130	7	19.	2	0845	132	0.	3	1600	257	0.	4	2315	382	0.				
1	0145	8	41.	2	0900	133	0.	3	1615	258	0.	4	2330	383	0.				
1	0200	9	68.	2	0915	134	0.	3	1630	259	0.	4	2345	384	0.				
1	0215	10	202.	2	0930	135	0.	3	1645	260	0.	5	0000	385	0.				
1	0230	11	368.	2	0945	136	0.	3	1700	261	0.	5	0015	386	0.				
1	0245	12	517.	2	1000	137	0.	3	1715	262	0.	5	0030	387	0.				
1	0300	13	618.	2	1015	138	0.	3	1730	263	0.	5	0045	388	0.				
1	0315	14	5295.	2	1030	139	0.	3	1745	264	0.	5	0100	389	0.				
1	0330	15	6786.	2	1045	140	0.	3	1800	265	0.	5	0115	390	0.				
1	0345	16	5235.	2	1100	141	0.	3	1815	266	0.	5	0130	391	0.				
1	0400	17	3248.	2	1115	142	0.	3	1830	267	0.	5	0145	392	0.				
1	0415	18	1921.	2	1130	143	0.	3	1845	268	0.	5	0200	393	0.				
1	0430	19	1143.	2	1145	144	0.	3	1900	269	0.	5	0215	394	0.				
1	0445	20	751.	2	1200	145	0.	3	1915	270	0.	5	0230	395	0.				
1	0500	21	593.	2	1215	146	0.	3	1930	271	0.	5	0245	396	0.				
1	0515	22	447.	2	1230	147	0.	3	1945	272	0.	5	0300	397	0.				
1	0530	23	330.	2	1245	148	0.	3	2000	273	0.	5	0315	398	0.				
1	0545	24	265.	2	1300	149	0.	3	2015	274	0.	5	0330	399	0.				
1	0600	25	240.	2	1315	150	0.	3	2030	275	0.	5	0345	400	0.				
1	0615	26	161.	2	1330	151	0.	3	2045	276	0.	5	0400	401	0.				
1	0630	27	82.	2	1345	152	0.	3	2100	277	0.	5	0415	402	0.				
1	0645	28	35.	2	1400	153	0.	3	2115	278	0.	5	0430	403	0.				
1	0700	29	15.	2	1415	154	0.	3	2130	279	0.	5	0445	404	0.				
1	0715	30	7.	2	1430	155	0.	3	2145	280	0.	5	0500	405	0.				
1	0730	31	3.	2	1445	156	0.	3	2200	281	0.	5	0515	406	0.				
1	0745	32	0.	2	1500	157	0.	3	2215	282	0.	5	0530	407	0.				
1	0800	33	0.	2	1515	158	0.	3	2230	283	0.	5	0545	408	0.				
1	0815	34	0.	2	1530	159	0.	3	2245	284	0.	5	0600	409	0.				
1	0830	35	0.	2	1545	160	0.	3	2300	285	0.	5	0615	410	0.				
1	0845	36	0.	2	1600	161	0.	3	2315	286	0.	5	0630	411	0.				
1	0900	37	0.	2	1615	162	0.	3	2330	287	0.	5	0645	412	0.				
1	0915	38	0.	2	1630	163	0.	3	2345	288	0.	5	0700	413	0.				
1	0930	39	0.	2	1645	164	0.	4	0000	289	0.	5	0715	414	0.				
1	0945	40	0.	2	1700	165	0.	4	0015	290	0.	5	0730	415	0.				
1	1000	41	0.	2	1715	166	0.	4	0030	291	0.	5	0745	416	0.				
1	1015	42	0.	2	1730	167	0.	4	0045	292	0.	5	0800	417	0.				
1	1030	43	0.	2	1745	168	0.	4	0100	293	0.	5	0815	418	0.				
1	1045	44	0.	2	1800	169	0.	4	0115	294	0.	5	0830	419	0.				
1	1100	45	0.	2	1815	170	0.	4	0130	295	0.	5	0845	420	0.				
1	1115	46	0.	2	1830	171	0.	4	0145	296	0.	5	0900	421	0.				
1	1130	47	0.	2	1845	172	0.	4	0200	297	0.	5	0915	422	0.				
1	1145	48	0.	2	1900	173	0.	4	0215	298	0.	5	0930	423	0.				
1	1200	49	0.	2	1915	174	0.	4	0230	299	0.	5	0945	424	0.				
1	1215	50	0.	2	1930	175	0.	4	0245	300	0.	5	1000	425	0.				
1	1230	51	0.	2	1945	176	0.	4	0300	301	0.	5	1015	426	0.				
1	1245	52	0.	2	2000	177	0.	4	0315	302	0.	5	1030	427	0.				
1	1300	53	0.	2	2015	178	0.	4	0330	303	0.	5	1045	428	0.				
1	1315	54	0.	2	2030	179	0.	4	0345	304	0.	5	1100	429	0.				
1	1330	55	0.	2	2045	180	0.	4	0400	305	0.	5	1115	430	0.				
1	1345	56	0.	2	2100	181	0.	4	0415	306	0.	5	1130	431	0.				
1	1400	57	0.	2	2115	182	0.	4	0430	307	0.	5	1145	432	0.				
1	1415	58	0.	2	2130	183	0.	4	0445	308	0.	5	1200	433	0.				
1	1430	59	0.	2	2145	184	0.	4	0500	309	0.	5	1215	434	0.				
1	1445	60	0.	2	2200	185	0.	4	0515	310	0.	5	1230	435	0.				
1	1500	61	0.	2	2215	186	0.	4	0530	311	0.	5	1245	436	0.				
1	1515	62	0.	2	2230	187	0.	4	0545	312	0.	5	1300	437	0.				
1	1530	63	0.	2	2245	188	0.	4	0600	313	0.	5	1315	438	0.				
1	1545	64	0.	2	2300	189	0.	4	0615	314	0.	5	1330	439	0.				
1	1600	65	0.	2	2315	190	0.	4	0630	315	0.	5	1345	440	0.				
1	1615	66	0.	2	2330	191	0.	4	0645	316	0.	5	1400	441	0.				
1	1630	67	0.	2	2345	192	0.	4	0700	317	0.	5	1415	442	0.				
1	1645	68	0.	3	0000	193	0.	4	0715	318	0.	5	1430	443	0.				
1	1700	69	0.	3	0015	194	0.	4	0730	319	0.	5	1445	444	0.				
1	1715	70	0.	3	0030	195	0.	4	0745	320	0.	5	1500	445	0.				
1	1730	71	0.	3	0045	196	0.	4	0800	321	0.	5	1515	446	0.				
1	1745	72	0.	3	0100	197	0.	4	0815	322	0.	5	1530	447	0.				
1	1800	73	0.	3	0115	198	0.	4	0830	323	0.	5	1545	448	0.				
1	1815	74	0.	3	0130	199	0.	4	0845	324	0.	5	1600	449	0.				
1	1830	75	0.	3	0145	200	0.	4	0900	325	0.	5	1615	450	0.				
1	1845	76	0.	3	0200	201	0.	4	0915	326	0.	5	1630	451	0.				
1	1900	77	0.	3	0215	202	0.	4	0930	327	0.	5	1645	452	0.				
1	1915	78	0.	3	0230	203	0.	4	0945	328	0.	5	1700	453	0.				
1	1930	79	0.	3	0245	204	0.	4	1000	329	0.	5	1715	454	0.				
1	1945	80	0.	3	0300	205	0.	4	1015	330	0.	5	1730	455	0.				
1	2000	81	0.	3	0315	206	0.	4	1030	331	0.	5	1745	456	0.				
1	2015	82	0.	3	0330	207	0.	4	1045	332	0.	5	1800	457	0.				

										PMF.OUT									
1	2030	83	0.	*	3	0345	208	0.	*	4	1100	333	0.	*	5	1815	458	0.	
1	2045	84	0.	*	3	0400	209	0.	*	4	1115	334	0.	*	5	1830	459	0.	
1	2100	85	0.	*	3	0415	210	0.	*	4	1130	335	0.	*	5	1845	460	0.	
1	2115	86	0.	*	3	0430	211	0.	*	4	1145	336	0.	*	5	1900	461	0.	
1	2130	87	0.	*	3	0445	212	0.	*	4	1200	337	0.	*	5	1915	462	0.	
1	2145	88	0.	*	3	0500	213	0.	*	4	1215	338	0.	*	5	1930	463	0.	
1	2200	89	0.	*	3	0515	214	0.	*	4	1230	339	0.	*	5	1945	464	0.	
1	2215	90	0.	*	3	0530	215	0.	*	4	1245	340	0.	*	5	2000	465	0.	
1	2230	91	0.	*	3	0545	216	0.	*	4	1300	341	0.	*	5	2015	466	0.	
1	2245	92	0.	*	3	0600	217	0.	*	4	1315	342	0.	*	5	2030	467	0.	
1	2300	93	0.	*	3	0615	218	0.	*	4	1330	343	0.	*	5	2045	468	0.	
1	2315	94	0.	*	3	0630	219	0.	*	4	1345	344	0.	*	5	2100	469	0.	
1	2330	95	0.	*	3	0645	220	0.	*	4	1400	345	0.	*	5	2115	470	0.	
1	2345	96	0.	*	3	0700	221	0.	*	4	1415	346	0.	*	5	2130	471	0.	
2	0000	97	0.	*	3	0715	222	0.	*	4	1430	347	0.	*	5	2145	472	0.	
2	0015	98	0.	*	3	0730	223	0.	*	4	1445	348	0.	*	5	2200	473	0.	
2	0030	99	0.	*	3	0745	224	0.	*	4	1500	349	0.	*	5	2215	474	0.	
2	0045	100	0.	*	3	0800	225	0.	*	4	1515	350	0.	*	5	2230	475	0.	
2	0100	101	0.	*	3	0815	226	0.	*	4	1530	351	0.	*	5	2245	476	0.	
2	0115	102	0.	*	3	0830	227	0.	*	4	1545	352	0.	*	5	2300	477	0.	
2	0130	103	0.	*	3	0845	228	0.	*	4	1600	353	0.	*	5	2315	478	0.	
2	0145	104	0.	*	3	0900	229	0.	*	4	1615	354	0.	*	5	2330	479	0.	
2	0200	105	0.	*	3	0915	230	0.	*	4	1630	355	0.	*	5	2345	480	0.	
2	0215	106	0.	*	3	0930	231	0.	*	4	1645	356	0.	*	6	0000	481	0.	
2	0230	107	0.	*	3	0945	232	0.	*	4	1700	357	0.	*	6	0015	482	0.	
2	0245	108	0.	*	3	1000	233	0.	*	4	1715	358	0.	*	6	0030	483	0.	
2	0300	109	0.	*	3	1015	234	0.	*	4	1730	359	0.	*	6	0045	484	0.	
2	0315	110	0.	*	3	1030	235	0.	*	4	1745	360	0.	*	6	0100	485	0.	
2	0330	111	0.	*	3	1045	236	0.	*	4	1800	361	0.	*	6	0115	486	0.	
2	0345	112	0.	*	3	1100	237	0.	*	4	1815	362	0.	*	6	0130	487	0.	
2	0400	113	0.	*	3	1115	238	0.	*	4	1830	363	0.	*	6	0145	488	0.	
2	0415	114	0.	*	3	1130	239	0.	*	4	1845	364	0.	*	6	0200	489	0.	
2	0430	115	0.	*	3	1145	240	0.	*	4	1900	365	0.	*	6	0215	490	0.	
2	0445	116	0.	*	3	1200	241	0.	*	4	1915	366	0.	*	6	0230	491	0.	
2	0500	117	0.	*	3	1215	242	0.	*	4	1930	367	0.	*	6	0245	492	0.	
2	0515	118	0.	*	3	1230	243	0.	*	4	1945	368	0.	*	6	0300	493	0.	
2	0530	119	0.	*	3	1245	244	0.	*	4	2000	369	0.	*	6	0315	494	0.	
2	0545	120	0.	*	3	1300	245	0.	*	4	2015	370	0.	*	6	0330	495	0.	
2	0600	121	0.	*	3	1315	246	0.	*	4	2030	371	0.	*	6	0345	496	0.	
2	0615	122	0.	*	3	1330	247	0.	*	4	2045	372	0.	*	6	0400	497	0.	
2	0630	123	0.	*	3	1345	248	0.	*	4	2100	373	0.	*	6	0415	498	0.	
2	0645	124	0.	*	3	1400	249	0.	*	4	2115	374	0.	*	6	0430	499	0.	
2	0700	125	0.	*	3	1415	250	0.	*	4	2130	375	0.	*	6	0445	500	0.	

PEAK FLOW	TIME	MAXIMUM AVERAGE FLOW			
+	(CFS)	6-HR	24-HR	72-HR	124.75-HR
+	6786.	3.50	1183.	296.	99.
		(INCHES)	12.528	12.534	12.534
		(AC-FT)	587.	587.	587.
CUMULATIVE AREA =		.88 SQ MI			

84 KK *****
* RDB1 *

87 KO OUTPUT CONTROL VARIABLES
IPRNT 1 PRINT CONTROL
IPLPT 0 PLOT CONTROL
QSCAL 0. HYDROGRAPH PLOT SCALE

HYDROGRAPH ROUTING DATA
88 RS STORAGE ROUTING
NSTPS 1 NUMBER OF SUBREACHES
ITYP ELEV TYPE OF INITIAL CONDITION
RSVKIC 3025.50 INITIAL CONDITION
X .00 WORKING R AND D COEFFICIENT

89 SA	AREA	4.6	5.6	5.8	6.0	6.2	7.2	7.6	8.3	9.0	9.7
90 SE	ELEVATION	3025.50	3027.00	3028.00	3029.00	3030.00	3035.00	3037.00	3040.00	3043.00	3046.00
91 SQ	DISCHARGE	50.	61.	71.	75.	1436.	3921.	7136.			
92 SE	ELEVATION	3025.50	3030.00	3035.00	3037.00	3040.00	3043.00	3046.00			

COMPUTED STORAGE-ELEVATION DATA											
STORAGE	.00	7.64	13.36	19.27	25.36	58.73	73.51	97.38	123.34	151.37	
ELEVATION	3025.50	3027.00	3028.00	3029.00	3030.00	3035.00	3037.00	3040.00	3043.00	3046.00	

COMPUTED STORAGE-OUTFLOW-ELEVATION DATA											
STORAGE	.00	7.64	13.36	19.27	25.36	58.73	73.51	97.38	123.34	151.37	
OUTFLOW	49.70	53.33	55.76	58.18	60.60	70.80	74.50	1436.00	3920.50	7136.30	
ELEVATION	3025.50	3027.00	3028.00	3029.00	3030.00	3035.00	3037.00	3040.00	3043.00	3046.00	

*** WARNING *** MODIFIED PULS ROUTING MAY BE NUMERICALLY UNSTABLE FOR OUTFLOWS BETWEEN 3921. TO 7136.
THE ROUTED HYDROGRAPH SHOULD BE EXAMINED FOR OSCILLATIONS OR OUTFLOWS GREATER THAN PEAK INFLOWS.
THIS CAN BE CORRECTED BY DECREASING THE TIME INTERVAL OR INCREASING STORAGE (USE A LONGER REACH.)

HYDROGRAPH AT STATION RDB1
PLAN 1, RATIO = 1.00

DA MON HRMN ORD					OUTFLOW	STORAGE	STAGE	*	DA MON HRMN ORD					OUTFLOW	STORAGE	STAGE	*	DA MON HRMN ORD					OUTFLOW	STORAGE	STAGE			
							*		*								*		*								*	
1	0000	1		50.		-0	3025.5	*	2		1745	168		50.		-5	3025.5	*	4		1130	335		50.		-5	3025.5	*
1	0015	2		50.		-5	3025.5	*	2		1800	169		50.		-5	3025.5	*	4		1145	336		50.		-5	3025.5	*
1	0030	3		50.		-5	3025.5	*	2		1815	170		50.		-5	3025.5	*	4		1200	337		50.		-5	3025.5	*
1	0045	4		50.		-5	3025.5	*	2		1830	171		50.		-5	3025.5	*	4		1215	338		50.		-5	3025.5	*
1	0100	5		50.		-5	3025.5	*	2		1845	172		50.		-5	3025.5	*	4		1230	339		50.		-5	3025.5	*
1	0115	6		50.		-5	3025.5	*	2		1900	173		50.		-5	3025.5	*	4		1245	340		50.		-5	3025.5	*
1	0130	7		50.		-5	3025.5	*	2		1915	174		50.		-5	3025.5	*	4		1300	341		50.		-5	3025.5	*
1	0145	8		50.		-5	3025.5	*	2		1930	175		50.		-5	3025.5	*	4		1315	342		50.		-5	3025.5	*
1	0200	9		50.		-4	3025.5	*	2		1945	176		50.		-5	3025.5	*	4		1330	343		50.		-5	3025.5	*
1	0215	10		50.		1.3	3025.8	*	2		2000	177		50.		-5	3025.5	*	4		1345	344		50.		-5	3025.5	*

1	0230	11	53.	6.2	3026.7	*	2	2015	178	50.	-5	3025.5	**	4	1400	345	50.	-5	3025.5
1	0245	12	56.	14.	3028.1	*	2	2030	179	50.	-5	3025.5	**	4	1415	346	50.	-5	3025.5
1	0300	13	60.	24.7	3029.6	*	2	2045	180	50.	-5	3025.5	**	4	1430	347	50.	-5	3025.5
1	0315	14	465.	80.4	3037.9	*	2	2100	181	50.	-5	3025.5	**	4	1445	348	50.	-5	3025.5
1	0330	15	5838.	140.1	3044.8	*	2	2115	182	50.	-5	3025.5	**	4	1460	349	50.	-5	3025.5
1	0345	16	6025.	141.7	3048.0	*	2	2130	183	50.	-5	3025.5	**	4	1475	350	50.	-5	3025.5
1	0400	17	4090.	174.8	3043.2	*	2	2200	184	50.	-5	3025.5	**	4	1500	351	50.	-5	3025.5
1	0415	18	2579.	109.3	3041.1	*	2	2215	185	50.	-5	3025.5	**	4	1515	352	50.	-5	3025.5
1	0430	19	1538.	98.4	3040.1	*	2	2230	186	50.	-5	3025.5	**	4	1530	353	50.	-5	3025.5
1	0445	20	1074.	91.0	3039.2	*	2	2245	187	50.	-5	3025.5	**	4	1545	354	50.	-5	3025.5
1	0500	21	776.	85.8	3038.5	*	2	2300	188	50.	-5	3025.5	**	4	1600	355	50.	-5	3025.5
1	0515	22	586.	82.5	3038.1	*	2	2315	189	50.	-5	3025.5	**	4	1615	356	50.	-5	3025.5
1	0530	23	4440.	79.9	3037.6	*	2	2330	191	50.	-5	3025.5	**	4	1630	357	50.	-5	3025.5
1	0545	24	334.	78.1	3037.6	*	2	2345	192	50.	-5	3025.5	**	4	1645	358	50.	-5	3025.5
1	0600	25	274.	77.0	3037.4	*	2	0000	193	50.	-5	3025.5	**	4	1700	357	50.	-5	3025.5
1	0615	26	219.	76.0	3037.3	*	3	0000	193	50.	-5	3025.5	**	4	1715	358	50.	-5	3025.5
1	0630	27	147.	74.8	3037.2	*	3	0015	194	50.	-5	3025.5	**	4	1730	359	50.	-5	3025.5
1	0645	28		73.6	3037.0	*	3	0030	195	50.	-5	3025.5	**	4	1745	360	50.	-5	3025.5
1	0700	29	84.	72.5	3036.9	*	3	0045	196	50.	-5	3025.5	**	4	1800	361	50.	-5	3025.5
1	0715	30	74.	71.2	3036.7	*	3	0100	197	50.	-5	3025.5	**	4	1815	362	50.	-5	3025.5
1	0730	31	74.	69.8	3036.5	*	3	0115	198	50.	-5	3025.5	**	4	1830	363	50.	-5	3025.5
1	0745	32	73.	68.3	3036.3	*	3	0130	199	50.	-5	3025.5	**	4	1845	364	50.	-5	3025.5
1	0800	33	75.	66.9	3036.1	*	3	0145	200	50.	-5	3025.5	**	4	1900	365	50.	-5	3025.5
1	0815	34	73.	65.4	3035.9	*	3	0200	201	50.	-5	3025.5	**	4	1915	366	50.	-5	3025.5
1	0830	35	72.	63.9	3035.7	*	3	0215	202	50.	-5	3025.5	**	4	1930	367	50.	-5	3025.5
1	0845	36	72.	62.4	3035.5	*	3	0230	203	50.	-5	3025.5	**	4	1945	368	50.	-5	3025.5
1	0900																		

CUMULATIVE AREA = .88 SQ MI

PEAK FLOW AND STAGE (END-OF-PERIOD) SUMMARY FOR MULTIPLE PLAN-RATIO ECONOMIC COMPUTATIONS
 FLOWS IN CUBIC FEET PER SECOND, AREA IN SQUARE MILES
 TIME TO PEAK IN HOURS

*** NORMAL END OF HEC-1 ***

APPENDIX D

Collector Channel WSPG Hydraulic Analysis

Summary of Collector Channel Hydraulic Characteristics								
Station	Q ₁₀₀ (cfs)	Flow Depth (ft)	Flow Velocity (ft/s)	Froude Number	Hydraulic Freeboard (ft) ¹	Sedimentation Freeboard (ft)	Required Channel Depth (ft)	Proposed Channel Depth (ft)
2489.05	475	0.69	27.66	5.88	1.61	1.0	3.3	4.5
2454.74	475	1.45	13.07	1.91	1.37	1.0	3.8	4.0
2422.47	475	2.15	11.99	1.68	1.39	1.0	4.5	5.0
2035.60	475	1.11	28.03	5.18	1.73	1.0	3.8	5.0
1825.49	475	1.50	19.18	3.11	1.55	1.0	4.0	5.0
1786.16	475	2.44	10.07	1.33	1.34	1.0	4.8	5.0
1674.49	475	2.44	10.07	1.33	1.34	1.0	4.8	5.0
1449.42	475	2.45	10.04	1.33	1.34	1.0	4.8	5.0
1304.15	475	2.49	9.82	1.19	1.29	1.0	4.8	5.0
1173.89	475	2.86	8.06	1.06	1.29	0.9	5.0	5.0
1 – Hydraulic Freeboard obtained from CCRFCD Manual Equations. If Froude Number <1, Equation 741; if Froude Number >1, Equation 747.								

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T1	SKYE CANYON DETENTION BASIN, OLY1401.004			
T2	TECHNICAL DRAINAGE STUDY UPDATE			
T3	COLLECTOR CHANNEL			3030.300
SO	-2489.0503025.900	2	.015	
R	-2454.7403036.900	2	.015	
TS	-2422.4703037.060	1	.015	
R	-2035.6003039.000	1	.015	
R	-1825.4903060.010	1	.015	
R	-1786.1603063.940	1	.015	
R	-1674.4903064.500	1	.015	
R	-1449.4203065.630	1	.015	
R	-1304.1503066.360	1	.015	
R	-1173.8903067.010	1	.015	
SH	-1173.8903067.010	1	.015	
CD	1 1 0 .000	5.000	12.000	3.000
CD	2 2 0 .000	5.000	25.000	0.000
Q	475.000 .0			

.000

7.511
21.327

18.374

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WATER SURFACE PROFILE - CHANNEL DEFINITION LISTING										PAGE 1										
W S F G W - EDIT LISTING - VERSION 14.00										DATE: 8-10-2010 TIME: 3:23:21										
CARD	SECT	CHN	NO OF	AVE	PIER	HEIGHT 1	BASE	ZL	ZR	INV	Y(1)	Y(2)	Y(3)	Y(4)	Y(5)	Y(6)	Y(7)	Y(8)	Y(9)	Y(10)
CODE	NO	TYPE	PIER/PIP	WIDTH	DIAMETER	WIDTH	DIAMETER	WIDTH	DIAMETER	WIDTH	DROP									

00	1	0	.000	5.000	12.000	3.000	.00
00	2	0	.000	5.000	25.000	3.000	.00

WATER SURFACE PROFILE - TITLE CARD LISTING

SKYE CANYON DETENTION BASIN, 0LY1401.004

TECHNICAL DRAINAGE STUDY UPDATE

COLLECTOR CHANNEL

W S P G W
WATER SURFACE PROFILE - ELEMENT CARD LISTING

ELEMENT NO	1 IS A	SYSTEM	OUTLET	STATION	INVERT	SECT	W S ELEV
			U/S DATA				
				-2489.050	3025.900	2	3030.300

STATION	INVERT	SECT	N	RADIUS	ANGLE	ANG PT	MAN
-2454.740	3036.900	2	.015	.000	.000	.000	0

STATION	INVERT	SECT	N	RADIUS	ANGLE
-2422.470	3037.060	1	.015	.000	.000

STATION	INVERT	SECT	N	RADIUS	ANGLE	ANG PT	MAN F
-2035.60	3039.000	1	.015	.000	.000	.000	0

ELEMENT NO	6	TS	A	BEACH	U/S DATA	STATION	INVERT	SECT	N	RADIUS	ANGLE	ANG PT	MAN H
						-1825.490	3060.010	1	.015	.000	.000	.000	0

ELEMENT NO	7	TS	A	REACH	U/S DATA	STATION	INVERT	SECT	N	RADIUS	ANGLE	ANG PT	MAN H
						-1786.160	3063.940	1	.015	300.019	7.511	.000	0

ELEMENT NO	8 IS A REACH	U/S DATA	STATION	INVERT	SECT	N	RADIUS	ANGLE	ANG PT	MAN
			-1674.490	3064.500	1	.015	300.006	21.327	.000	0

ELEMENT NO	9	IS A REACH	U/S DATA	STATION	INVERT	SECT	N	RADIUS	ANGLE	MAN PT
			-1449.420	3065.630	*	1	.015	.000	.000	0

ELEMENT NO	10 IS A REACH	U/S DATA	STATION	INVEKI	SECI	N	RADIUS	ANGLE	ANG PI	MAIN
						.015	452.997	18.374	.000	0
			-1304.150	3066.360	1	*				

ELEMENT NO	11 IS A SYSTEM HEADWORKS	*
C/S DATA	-1173.890	3067.010
STATION	INVERT	SECT
NADIOS	.000	* .015
ANGEL	.000	
ANG F1	.000	
MEAL	0	

	STATE	INVESTMENT	CURRENTLY
-1173.890	3067.010	1	.000

→ HYDRAULICS ENTELLING

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SKYE CANYON DETENTION BASIN, OLY1401.004
TECHNICAL DRAINAGE STUDY UPDATE

COLLECTOR CHANNEL

Station	Invert Elev	Depth (FT)	Water Elev	Q (CFS)	Vel (FPS)	Vel Head	Energy Grd.El.	Super Elev	Critical Depth	Flow Top Width	Height/ Dia.-FT	Base Wt or I.D.	ZL	No Wth Prs/Pip
L/Elem *****	Ch Slope *****	*****	*****	*****	*****	SF Ave *****	HF *****	SE Dpth *****	Froude N *****	"N" *****	X-Fall *****	*****	ZR *****	Type Ch *****
-2489.051 4.115	3025.900 .3206	.687	3026.587	475.00	27.66	11.88 .1307	3038.47 .54	.00	2.24 5.88	25.00 .53	5.000 .015	25.000 .00	.00	0 .0
-2484.935 4.713	3027.219 .3206	.711	3027.931	475.00	26.71	11.08 .1143	3039.01 .54	.00	2.24 5.58	25.00 .53	5.000 .015	25.000 .00	.00	0 .0
-2480.222 3.947	3028.730 .3206	.746	3029.477	475.00	25.46	10.07 .0979	3039.55 .39	.00	2.24 5.20	25.00 .53	5.000 .015	25.000 .00	.00	0 .0
-2476.275 3.353	3029.996 .3206	.783	3030.778	475.00	24.28	9.15 .0838	3039.93 .28	.00	2.24 4.84	25.00 .53	5.000 .015	25.000 .00	.00	0 .0
-2472.922 2.880	3031.071 .3206	.821	3031.892	475.00	23.15	8.32 .0718	3040.21 .21	.00	2.24 4.50	25.00 .53	5.000 .015	25.000 .00	.00	0 .0
-2470.042 2.492	3031.994 .3206	.861	3032.855	475.00	22.07	7.57 .0615	3040.42 .15	.00	2.24 4.19	25.00 .53	5.000 .015	25.000 .00	.00	0 .0
-2467.550 2.169	3032.793 .3206	.903	3033.696	475.00	21.05	6.88 .0527	3040.57 .11	.00	2.24 3.90	25.00 .53	5.000 .015	25.000 .00	.00	0 .0
-2465.381 1.896	3033.489 .3206	.947	3034.435	475.00	20.07	6.25 .0452	3040.69 .09	.00	2.24 3.63	25.00 .53	5.000 .015	25.000 .00	.00	0 .0
-2463.485 1.661	3034.096 .3206	.993	3035.089	475.00	19.13	5.68 .0387	3040.77 .06	.00	2.24 3.38	25.00 .53	5.000 .015	25.000 .00	.00	0 .0
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FILE: CCHNL.WSW

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Program Package Serial Number: 1704

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SKYE CANYON DETENTION BASIN, OLY1401.004
TECHNICAL DRAINAGE STUDY UPDATE

[illegible]

Prs/P7p

L/Elem	Ch Slope	Depth (FT)	Water Elev	Q (CFS)	Vel (FPS)	Vel Head	Energy Grd.El.	Super Elev	Critical Depth	Flow Top Width	Height/Dia.-FT or I.D.	X-Fall	ZR	Type Ch
-2461.824	3034.629	1.042	3035.670	475.00	18.24	5.17	3040.84	.00	2.24	25.00	5.000	25.000	.00	0 .0
1.458	.3206					.0332	.05	1.04	3.15	.53	.015	.00	.00	RECTANG
-2460.367	3035.096	1.092	3036.188	475.00	17.39	4.70	3040.89	.00	2.24	25.00	5.000	25.000	.00	0 .0
1.279	.3206					.0285	.04	1.09	2.93	.53	.015	.00	.00	RECTANG
-2459.088	3035.506	1.146	3036.652	475.00	16.58	4.27	3040.92	.00	2.24	25.00	5.000	25.000	.00	0 .0
1.122	.3206					.0244	.03	1.15	2.73	.53	.015	.00	.00	RECTANG
-2457.966	3035.866	1.202	3037.067	475.00	15.81	3.88	3040.95	.00	2.24	25.00	5.000	25.000	.00	0 .0
.982	.3206					.0209	.02	1.20	2.54	.53	.015	.00	.00	RECTANG
-2456.984	3036.181	1.260	3037.441	475.00	15.08	3.53	3040.97	.00	2.24	25.00	5.000	25.000	.00	0 .0
.857	.3206					.0180	.02	1.26	2.37	.53	.015	.00	.00	RECTANG
-2456.127	3036.455	1.322	3037.777	475.00	14.37	3.21	3040.99	.00	2.24	25.00	5.000	25.000	.00	0 .0
.744	.3206					.0154	.01	1.32	2.20	.53	.015	.00	.00	RECTANG
-2455.383	3036.694	1.386	3038.080	475.00	13.71	2.92	3041.00	.00	2.24	25.00	5.000	25.000	.00	0 .0
.643	.3206					.0132	.01	1.39	2.05	.53	.015	.00	.00	RECTANG
-2454.740	3036.900	1.454	3038.354	475.00	13.07	2.65	3041.01	.00	2.24	25.00	5.000	25.000	.00	0 .0
TRANS STR	.0050					.0102	.33	1.45	1.91		.015	.00	.00	RECTANG
-2422.469	3037.060	2.147	3039.207	475.00	11.99	2.23	3041.44	.00	2.86	24.88	5.000	12.000	3.00	0 .0
32.457	.0050					.0087	.28	2.15	1.68	2.44	.015	.00	3.00	TRAP

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SKYE CANYON DETENTION BASIN, OLY1401.004
TECHNICAL DRAINAGE STUDY UPDATE
COLLECTOR CHANNEL

station	Invert Elev	Depth (FT)	Water Elev	Q (CFS)	Vel (FPS)	Vel Head	Energy Grd.El.	Super Elev	Critical Depth	Flow Top Width	Height/Dia.-FT or I.D.	X-Fall	ZR	No Wth Prs/Pip
-2390.013	3037.223	2.087	3039.310	475.00	12.47	2.41	3041.72	.00	2.86	24.52	5.000	12.000	3.00	0 .0
35.338	.0050					.0098	.35	2.09	1.76	2.44	.015	.00	3.00	TRAP
-2354.675	3037.400	2.014	3039.414	475.00	13.07	2.65	3042.07	.00	2.86	24.08	5.000	12.000	3.00	0 .0

Station	Invert Elev	Depth (FT)	Water Elev	Q (CFS)	Vel (FPS)	Vel Head	Energy Grd.El.	Super Elev	Critical Depth	Flow Top Width	Height/Dia.	Base Wt or I.D.	No With Prs/Pip
31.542	.0050	-	-	-	-	.0112	-	2.01	1.88	2.44	.015	-	-
-2323.133	3037.558	1.943	3039.501	475.00	13.71	2.92	3042.42	.00	2.86	23.66	5.000	12.000	3.00
28.688	.0050	-	-	-	-	.0128	-	1.94	2.00	2.44	.015	-	-
-2294.444	3037.702	1.874	3039.576	475.00	14.38	3.21	3042.79	.00	2.86	23.24	5.000	12.000	3.00
26.426	.0050	-	-	-	-	.0146	-	1.87	2.13	2.44	.015	-	-
-2268.018	3037.835	1.807	3039.642	475.00	15.08	3.53	3043.18	.00	2.86	22.84	5.000	12.000	3.00
24.561	.0050	-	-	-	-	.0168	-	1.81	2.26	2.44	.015	-	-
-2243.457	3037.958	1.743	3039.700	475.00	15.82	3.89	3043.59	.00	2.86	22.46	5.000	12.000	3.00
22.976	.0050	-	-	-	-	.0192	-	1.74	2.41	2.44	.015	-	-
-2220.481	3038.073	1.680	3039.753	475.00	16.59	4.28	3044.03	.00	2.86	22.08	5.000	12.000	3.00
21.597	.0050	-	-	-	-	.0220	-	1.68	2.57	2.44	.015	-	-
-2198.884	3038.181	1.619	3039.801	475.00	17.40	4.70	3044.50	.00	2.86	21.71	5.000	12.000	3.00
20.376	.0050	-	-	-	-	.0252	-	1.62	2.74	2.44	.015	-	-
-2178.508	3038.284	1.560	3039.844	475.00	18.25	5.17	3045.02	.00	2.86	21.36	5.000	12.000	3.00
19.277	.0050	-	-	-	-	.0289	-	1.56	2.91	2.44	.015	-	-

W S P G W - CIVILDESIGN Version 14.06
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 SKYE CANYON DETENTION BASIN, OLY1401.004
 TECHNICAL DRAINAGE STUDY UPDATE
 COLLECTOR CHANNEL
 CCHNL.OUT
 FILE: CCHNL.WSW

Date: 8-10-2016 Time: 5:23:30

Station	Invert Elev	Depth (FT)	Water Elev	Q (CFS)	Vel (FPS)	Vel Head	Energy Grd.El.	Super Elev	Critical Depth	Flow Top Width	Height/Dia.	Base Wt or I.D.	No With Prs/Pip
-2159.231	3038.380	1.503	3039.883	475.00	19.14	5.69	3045.57	.00	2.86	21.02	5.000	12.000	3.00
18.276	.0050	-	-	-	-	.0331	-	1.50	3.10	2.44	.015	-	-
-2140.955	3038.472	1.448	3039.920	475.00	20.08	6.26	3046.18	.00	2.86	20.69	5.000	12.000	3.00
17.356	.0050	-	-	-	-	.0380	-	1.45	3.31	2.44	.015	-	-
-2123.599	3038.559	1.394	3039.953	475.00	21.06	6.89	3046.84	.00	2.86	20.36	5.000	12.000	3.00
16.504	.0050	-	-	-	-	.0436	-	1.39	3.53	2.44	.015	-	-
-2107.095	3038.642	1.342	3039.984	475.00	22.08	7.57	3047.56	.00	2.86	20.05	5.000	12.000	3.00

Station	Invert Elev	Depth (FT)	Water Elev	Q (CFS)	Vel (FPS)	Vel Head	Energy Grd. E.	Super Elev	Critical Depth	Flow Top Width	Height/Dia.-Fr	Base Wt or I.D.	ZL	No Wth Prs/pip
15.708	.0050													
-2091.387	3038.720	1.292	3040.012	475.00	23.16	8.33	3048.34	.00	2.86	19.75	5.000	12.000	3.00	0.0
14.963	.0050					.0574	.86	1.29	4.01	2.44	.015	.00	3.00	TRAP
-2076.424	3038.795	1.243	3040.039	475.00	24.29	9.16	3049.20	.00	2.86	19.46	5.000	12.000	3.00	0.0
14.260	.0050					.0660	.94	1.24	4.27	2.44	.015	.00	3.00	TRAP
-2062.163	3038.867	1.196	3040.063	475.00	25.48	10.08	3050.14	.00	2.86	19.18	5.000	12.000	3.00	0.0
13.596	.0050					.0758	1.03	1.20	4.55	2.44	.015	.00	3.00	TRAP
-2048.567	3038.935	1.150	3040.085	475.00	26.72	11.09	3051.17	.00	2.86	18.90	5.000	12.000	3.00	0.0
12.967	.0050					.0871	1.13	1.15	4.86	2.44	.015	.00	3.00	TRAP
-2035.600	3039.000	1.106	3040.106	475.00	28.03	12.20	3052.30	.00	2.86	18.64	5.000	12.000	3.00	0.0
82.005	.1000					.0874	7.17	1.11	5.18	1.08	.015	.00	3.00	TRAP

FILE: CCHNL.WSW

Program Package Serial Number: 1704
W S P G W - CIVILDESIGN Version 14.06

SKYE CANYON DETENTION BASIN, OLY1401.004
TECHNICAL DRAINAGE STUDY UPDATE

Date: 8-10-2016 Time: 5:23:30

Station	Invert Elev	Depth (FT)	Water Elev	Q (CFS)	Vel (FPS)	Vel Head	Energy Grd. E.	Super Elev	Critical Depth	Flow Top Width	Height/Dia.-Fr	Base Wt or I.D.	ZL	No Wth Prs/pip
-1953.595	3047.200	1.148	3048.348	475.00	26.78	11.13	3059.48	.00	2.86	18.89	5.000	12.000	3.00	0.0
40.734	.1000					.0763	3.11	1.15	4.87	1.08	.015	.00	3.00	TRAP
-1912.861	3051.273	1.194	3052.467	475.00	25.53	10.12	3062.59	.00	2.86	19.16	5.000	12.000	3.00	0.0
25.969	.1000					.0664	1.72	1.19	4.57	1.08	.015	.00	3.00	TRAP
-1886.892	3053.870	1.241	3055.111	475.00	24.34	9.20	3064.31	.00	2.86	19.45	5.000	12.000	3.00	0.0
18.667	.1000					.0578	1.08	1.24	4.28	1.08	.015	.00	3.00	TRAP
-1868.225	3055.737	1.290	3057.026	475.00	23.21	8.37	3065.39	.00	2.86	19.74	5.000	12.000	3.00	0.0
14.300	.1000					.0503	.72	1.29	4.02	1.08	.015	.00	3.00	TRAP
-1853.925	3057.167	1.340	3058.506	475.00	22.13	7.61	3066.11	.00	2.86	20.04	5.000	12.000	3.00	0.0
11.389	.1000					.0438	.50	1.34	3.77	1.08	.015	.00	3.00	TRAP
-1842.536	3058.305	1.392	3059.697	475.00	21.10	6.91	3066.61	.00	2.86	20.35	5.000	12.000	3.00	0.0
9.306	.1000					.0382	.36	1.39	3.54	1.08	.015	.00	3.00	TRAP

CCHNL-OUT														
-1833.230	3059.236	1.445	3060.681	475.00	20.12	6.29	3066.97	.00	2.86	20.67	5.000	12.000	3.00	0.0
7.740	.1000	-	-	-	-	.0333	.26	1.45	3.32	1.08	.015	.00	3.00	TRAP
-1825.490	3060.010	1.501	3061.511	475.00	19.18	5.71	3067.22	1.04	2.86	21.00	5.000	12.000	3.00	0.0
3.524	.0999	-	-	-	-	.0300	.11	2.54	3.11	1.08	.015	.00	3.00	TRAP
-1821.966	3060.362	1.530	3061.892	475.00	18.71	5.44	3067.33	1.00	2.86	21.18	5.000	12.000	3.00	0.0
5.990	.0999	-	-	-	-	.0271	.16	2.53	3.01	1.08	.015	.00	3.00	TRAP

‡ FILE: CCHNL.WSW
 W S P G W - CIVILDESIGN Version 14.06
 Program Package Serial Number: 1704
 SKYE CANYON DETENTION BASIN, OLY1401.004
 WATER SURFACE PROFILE LISTING
 TECHNICAL DRAINAGE STUDY UPDATE
 COLLECTOR CHANNEL
 Date: 8-10-2016 Time: 5:23:30
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Station	Invert Elev	Depth (FT)	Water Elev	Q (CFS)	Vel (FPS)	Vel Head	Energy Grd.El.	Super Elev	Critical Depth	Flow Top Width	Height/Dia.-FT	Base wt or I.D.	ZL	No With Prs/Pip
L/Elem	Ch Slope					SF Ave	HF	SE Dpth	Froude N	Norm Dp	"N"	X-Fall	ZR	Type Ch
-1815.976	3060.961	1.588	3062.549	475.00	17.84	4.94	3067.49	.92	2.86	21.53	5.000	12.000	3.00	0.0
5.106	.0999	-	-	-	-	.0236	.12	2.51	2.83	1.08	.015	.00	3.00	TRAP
-1810.870	3061.471	1.648	3063.119	475.00	17.01	4.49	3067.61	.85	2.86	21.89	5.000	12.000	3.00	0.0
4.373	.0999	-	-	-	-	.0206	.09	2.50	2.65	1.08	.015	.00	3.00	TRAP
-1806.497	3061.908	1.710	3063.617	475.00	16.22	4.09	3067.70	.79	2.86	22.26	5.000	12.000	3.00	0.0
3.757	.0999	-	-	-	-	.0180	.07	2.50	2.49	1.08	.015	.00	3.00	TRAP
-1802.740	3062.283	1.773	3064.057	475.00	15.47	3.71	3067.77	.73	2.86	22.64	5.000	12.000	3.00	0.0
3.231	.0999	-	-	-	-	.0157	.05	2.50	2.34	1.08	.015	.00	3.00	TRAP
-1799.509	3062.606	1.839	3064.445	475.00	14.75	3.38	3067.82	.67	2.86	23.03	5.000	12.000	3.00	0.0
2.777	.0999	-	-	-	-	.0137	.04	2.51	2.20	1.08	.015	.00	3.00	TRAP
-1796.733	3062.884	1.907	3064.790	475.00	14.06	3.07	3067.86	.62	2.86	23.44	5.000	12.000	3.00	0.0
2.381	.0999	-	-	-	-	.0120	.03	2.53	2.06	1.08	.015	.00	3.00	TRAP
-1794.352	3063.122	1.976	3065.098	475.00	13.41	2.79	3067.89	.58	2.86	23.86	5.000	12.000	3.00	0.0
2.034	.0999	-	-	-	-	.0105	.02	2.55	1.94	1.08	.015	.00	3.00	TRAP
-1792.318	3063.325	2.048	3065.373	475.00	12.78	2.54	3067.91	.53	2.86	24.29	5.000	12.000	3.00	0.0
1.726	.0999	-	-	-	-	.0092	.02	2.58	1.82	1.08	.015	.00	3.00	TRAP

-1790.592 3063.497 2.122 3065.619 475.00 12.19 2.31 3067.93 .49 2.86 24.73 5.000 12.000 3.00 0 .0
 1.452 .0999
 FILE: CCHNL.WSW

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 Program Package Serial Number: 1704
 WATER SURFACE PROFILE LISTING
 SKYE CANYON DETENTION BASIN, OLY1401.004
 TECHNICAL DRAINAGE STUDY UPDATE

Date: 8-10-2016 Time: 5:23:30

Station	Invert Elev	Depth (FT)	Water Elev	Q (CFS)	Vel (FPS)	Vel Head	Energy Grd.El.	Super Elev	Critical Depth	Flow Width	Flow Top Dia.-FT	Height/ Base wt or I.D.	ZL	No With Prs/Pip
L/Elem	Ch Slope					SF Ave	HF	SE Dpth	Froude N	Norm Dp	"N"	X-Fall	ZR	Type Ch
-1789.140	3063.642	2.198	3065.840	475.00	11.62	2.10	3067.94	.46	2.86	25.19	5.000	12.000	3.00	0 .0
1.207	.0999					.0070	.01		1.61	1.08	.015	.00	3.00	TRAP
-1787.933	3063.763	2.277	3066.040	475.00	11.08	1.91	3067.95	.42	2.86	25.66	5.000	12.000	3.00	0 .0
.986	.0999					.0061	.01		1.51	1.08	.015	.00	3.00	TRAP
-1786.947	3063.861	2.358	3066.219	475.00	10.56	1.73	3067.95	.39	2.86	26.15	5.000	12.000	3.00	0 .0
.787	.0999					.0054	.00		1.42	1.08	.015	.00	3.00	TRAP
-1786.160	3063.940	2.441	3066.381	475.00	10.07	1.58	3067.96	.36	2.86	26.64	5.000	12.000	3.00	0 .0
111.670	.0050					.0050	.56		1.33	2.44	.015	.00	3.00	TRAP
-1674.490	3064.500	2.441	3066.941	475.00	10.07	1.58	3068.52	.00	2.86	26.64	5.000	12.000	3.00	0 .0
28.344	.0050					.0050	.14		1.33	2.44	.015	.00	3.00	TRAP
-1646.146	3064.642	2.441	3067.083	475.00	10.07	1.58	3068.66	.00	2.86	26.64	5.000	12.000	3.00	0 .0
196.726	.0050					.0050	.98		1.33	2.44	.015	.00	3.00	TRAP
-1449.420	3065.630	2.446	3068.076	475.00	10.04	1.57	3069.64	.24	2.86	26.68	5.000	12.000	3.00	0 .0
145.270	.0050					.0048	.70		1.33	2.44	.015	.00	3.00	TRAP
-1304.150	3066.360	2.487	3068.847	475.00	9.82	1.50	3070.34	.00	2.86	26.92	5.000	12.000	3.00	0 .0
18.704	.0050					.0046	.09		1.29	2.44	.015	.00	3.00	TRAP
-1285.446	3066.453	2.497	3068.950	475.00	9.76	1.48	3070.43	.00	2.86	26.98	5.000	12.000	3.00	0 .0
70.323	.0050					.0043	.30		1.28	2.44	.015	.00	3.00	TRAP

W S P G W - CIVILDESIGN Version 14.06
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 WATER SURFACE PROFILE LISTING
 SKYE CANYON DETENTION BASIN, OLY1401.004
 TECHNICAL DRAINAGE STUDY UPDATE
 COLLECTOR CHANNEL

Date: 8-10-2016 Time: 5:23:30

CCHNL.OUT														
Station	Invert Elev	Depth (FT)	Water Elev	Q (CFS)	Vel (FPS)	Vel Head	Energy Grd.El.	Super Elev	Critical Depth	Flow Top width	Height/ Dia.-FT or I.D.	Base Wt	ZL	No With Prs/Pip
L/Elem	Ch slope					SF Ave	HF	SE Dpth	Froude N	Norm Dp	"N"	X-Fall	ZR	Type Ch
-1215.123	3066.804	2.584	3069.388	475.00	9.31	1.35	3070.73	.00	2.86	27.50	5.000	12.000	3.00	0 .0
26.975	.0050					.0038	.10	2.58	1.20	2.44	.015	.00	3.00	TRAP
-1188.148	3066.939	2.673	3069.612	475.00	8.87	1.22	3070.84	.00	2.86	28.04	5.000	12.000	3.00	0 .0
11.286	.0050					.0033	.04	2.67	1.13	2.44	.015	.00	3.00	TRAP
-1176.862	3066.995	2.766	3069.761	475.00	8.46	1.11	3070.87	.00	2.86	28.59	5.000	12.000	3.00	0 .0
2.972	.0050					.0029	.01	2.77	1.06	2.44	.015	.00	3.00	TRAP
-1173.890	3067.010	2.862	3069.872	475.00	8.06	1.01	3070.88	.00	2.86	29.17	5.000	12.000	3.00	0 .0

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SKYE CANYON DETENTION BASIN, OLY1401.004

TECHNICAL DRAINAGE STUDY UPDATE

COLLECTOR CHANNEL

-2489.05	.IW C H	E												R
-2479.16	. I W C H	E												R
-2469.27	. IW C H	E												R
-2459.39	. I W C H	E												R
-2449.50	. IW C H	E												R
-2439.61	. I W C H	E												R
-2429.72	. I W C H	E												R

CCHNL.OUT

-2419.83	.	I W C	H	E	.	R
-2409.94	.	I W C	H	E	.	R
-2400.06	.	I W C	H	E	.	R
-2390.17	.	I W C	H	E	.	R
-2380.28	.	I W C	HE		.	R
-2370.39	.	I W C	H		.	R
-2360.50	.	I W C	EH		.	R
-2350.61	.	I W C	EH		.	R
-2340.72	.	I W C	EH		.	R
-2330.84	.	I W C	EH		.	TX
-2320.95	.	I W C	EH		.	R
-2311.06	.	I W C	EH		.	R
-2301.17	.	I W C	H		.	R
-2291.28	.	I W C	EH		.	R
-2281.39	.	I W C	H		.	R
-2271.51	.				.	
-2261.62	.	I W C	HE		.	R
-2251.73	.				.	
-2241.84	.	I W C	HE		.	R

-2231.95	.						CCHNL.OUT	.
-2222.06	.							.
-2212.18	.	I	W	C	H	E		R
-2202.29	.							.
-2192.40	.	I	W	C	H	E		R
-2182.51	.							.
-2172.62	.	I	W	C	H	E		R
-2162.73	.							.
-2152.84	.	I	W	C	H	E		R
-2142.96	.							.
-2133.07	.	I	W	C	H	E		R
-2123.18	.	I	W	C	H	E		R
-2113.29	.							.
-2103.40	.	I	W	C	H	E		R
-2093.51	.							.
-2083.63	.	I	W	C	H	E		R
-2073.74	.	I	W	C	H	E		R
-2063.85	.							.
-2053.96	.	I	W	C	H	E		R

-2044.07	.	I	W	C	H	CCHNL.OUT E	.	R
-2034.18	.	I	W	C	H	E	.	R
-2024.29	.						.	
-2014.41	.						.	
-2004.52	.						.	
-1994.63	.						.	
-1984.74	.						.	
-1974.85	.						.	
-1964.96	.						.	
-1955.08	.						.	
-1945.19	.	I	W	C	H	E	.	R
-1935.30	.						.	
-1925.41	.						.	
-1915.52	.						.	
-1905.63	.	I	W	C	H	E	.	R
-1895.75	.						.	
-1885.86	.	I	W	C	H	E	.	R
-1875.97	.						.	
-1866.08	.	I	W	C	H	E	.	R

CCHNL. OUT

-1668.31	.	I	W	E	H	.	R
-1658.42	.					.	
-1648.53	.					.	
-1638.65	.	I	W	C	E	H	.
-1628.76	.					.	R
-1618.87	.					.	
-1608.98	.					.	
-1599.09	.					.	
-1589.20	.					.	
-1579.32	.					.	
-1569.43	.					.	
-1559.54	.					.	
-1549.65	.					.	
-1539.76	.					.	
-1529.87	.					.	
-1519.98	.					.	
-1510.10	.					.	
-1500.21	.					.	
-1490.32	.					.	

-1292.55	.									CCHNL.001	.
-1282.66	.	I	W C E H . R								.
-1272.77	.										.
-1262.89	.										.
-1253.00	.										.
-1243.11	.										.
-1233.22	.										.
-1223.33	.										.
-1213.44	.	I	W E H . R								.
-1203.56	.										.
-1193.67	.										.
-1183.78	.	I	W C E H . R								.
-1173.89	.	I	W E H . R								.
3025.900	3030.511	3035.122	3039.733	3044.344	3048.955	3053.566	3058.177	3062.789	3067.400	3072.010	

1. GLOSSARY

- GLOSSARY
- | | | |
|---|---|-------------------------|
| I | = | INVERT ELEVATION |
| C | = | CRITICAL DEPTH |
| W | = | WATER SURFACE ELEVATION |
| S | = | SUPER-ELEVATION |
| H | = | HEIGHT OF CHANNEL |
| E | = | ENERGY GRADE LINE |
| X | = | CURVES CROSSING OVER |
| B | = | BRIDGE ENTRANCE OR EXIT |
| Y | = | WALL ENTRANCE OR EXIT |

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APPENDIX E

Outlet Pipe WSPG Hydraulic Analysis

WSPG Summary							
Outlet Storm Drain							
WSPG Station	Storm Drain Diameter (in)	Q ₁₀₀ (cfs)	Invert Elevation (ft)	WSE (ft)	Velocity (ft/s)	FG Elevation	Description
1747.32	60	113	2995.40	2997.0	21.73	3001.4	End Model
1641.84	60	113	3000.79	3002.5	19.24	3028.2	SDMH#5
1641.84	60	106	3000.79	3002.3	20.79	3028.2	SM3 24" RCP Lateral
1567.32	60	106	3004.60	3006.3	18.59	3028.8	SDMH#4
1480.00	60	106	3009.06	3011.7	10.13	3027.4	SDMH#3
1480.00	60	71	3009.06	3010.4	16.26	3027.4	SM1 & SM2 36" RCP Lateral
1415.65	60	71	3012.35	3014.7	8.04	3028.0	SDMH#2
1294.16	60	71	3012.83	3015.1	8.08	3035.5	SDMH#1
1052.15	60	71	3013.80	3016.2	7.71	3025.5	Detention Basin Outlet Structure

1 - 'HGL Check' reflects the minimum 1-ft below finished grade.

60-OUT.WSW

TT1 SKYE CANYON DETENTION BASIN, OLY1401.004
TT2 TECHNICAL DRAINAGE STUDY UPDATE

TT2 TECHNICAL DRAINAGE STUDY UPDATE

T3 OUTLET PIPE

[illegible]

W S ELEV
.000

60-OUT. EDT

SECT
60

INVERT
3013.800

STATION
-1052.150

U/S DATA

Station	Invert Elev	Depth (FT)	Water Elev	Q (CFS)	Vel (FPS)	Vel Head	Energy Grd.El.	Super Elev	Critical Depth	Flow Width	Height/ Dia.-FT	Base Wt or I.D.	No Wth Prs/Pip
L/Elem	Ch Slope					SF Ave	HF	SE Dpth	Froude N	Norm Dp	"N"	X-Fall	ZL
-1747.320	2995.400	1.554	2996.953	113.00	21.73	7.33	3004.28	.00	3.03	4.63	5.000	.00	1.0
35.256	.0511					.0414	1.46	1.55	3.61	1.48	.013	.00	PIPE
-1712.063	2997.202	1.583	2998.785	113.00	21.17	6.96	3005.74	.00	3.03	4.65	5.000	.00	1.0
42.098	.0511					.0374	1.57	1.58	3.48	1.48	.013	.00	PIPE
-1669.966	2999.353	1.638	3000.991	113.00	20.18	6.33	3007.32	.00	3.03	4.69	5.000	.00	1.0
28.126	.0511					.0327	.92	1.64	3.26	1.48	.013	.00	PIPE
-1641.840	3000.790	1.697	3002.487	113.00	19.24	5.75	3008.24	.00	3.03	4.73	5.000	.00	1.0
JUNCT STR	.0000					.0352	.00	1.70	3.05		.013	.00	PIPE
-1641.840	3000.790	1.532	3002.321	106.00	20.79	6.71	3009.03	.00	2.93	4.61	5.000	.00	1.0
15.738	.0511					.0389	.61	1.53	3.48	1.44	.013	.00	PIPE
-1626.102	3001.594	1.549	3003.144	106.00	20.45	6.49	3009.64	.00	2.93	4.62	5.000	.00	1.0
34.646	.0511					.0357	1.24	1.55	3.40	1.44	.013	.00	PIPE
-1591.457	3003.366	1.604	3004.970	106.00	19.49	5.90	3010.87	.00	2.93	4.67	5.000	.00	1.0
24.137	.0511					.0313	.75	1.60	3.18	1.44	.013	.00	PIPE
-1567.320	3004.601	1.661	3006.262	106.00	18.59	5.36	3011.63	.00	2.93	4.71	5.000	.00	1.0
13.852	.0511					.0278	.39	1.66	2.98	1.44	.013	.00	PIPE
-1553.468	3005.308	1.704	3007.012	106.00	17.94	5.00	3012.01	.00	2.93	4.74	5.000	.00	1.0
15.033	.0511					.0248	.37	1.70	2.83	1.44	.013	.00	PIPE

Station	Invert Elev	Depth (FT)	Water Elev	Q (CFS)	Vel (FPS)	Vel Head	Energy Grd.El.	Super Elev	Critical Depth	Flow Width	Height/ Dia.-FT	Base Wt or I.D.	No Wth Prs/Pip
L/Elem	Ch Slope					SF Ave	HF	SE Dpth	Froude N	Norm Dp	"N"	X-Fall	ZL
-1747.320	2995.400	1.554	2996.953	113.00	21.73	7.33	3004.28	.00	3.03	4.63	5.000	.00	1.0
35.256	.0511					.0414	1.46	1.55	3.61	1.48	.013	.00	PIPE
-1712.063	2997.202	1.583	2998.785	113.00	21.17	6.96	3005.74	.00	3.03	4.65	5.000	.00	1.0
42.098	.0511					.0374	1.57	1.58	3.48	1.48	.013	.00	PIPE
-1669.966	2999.353	1.638	3000.991	113.00	20.18	6.33	3007.32	.00	3.03	4.69	5.000	.00	1.0
28.126	.0511					.0327	.92	1.64	3.26	1.48	.013	.00	PIPE
-1641.840	3000.790	1.697	3002.487	113.00	19.24	5.75	3008.24	.00	3.03	4.73	5.000	.00	1.0
JUNCT STR	.0000					.0352	.00	1.70	3.05		.013	.00	PIPE
-1641.840	3000.790	1.532	3002.321	106.00	20.79	6.71	3009.03	.00	2.93	4.61	5.000	.00	1.0
15.738	.0511					.0389	.61	1.53	3.48	1.44	.013	.00	PIPE
-1626.102	3001.594	1.549	3003.144	106.00	20.45	6.49	3009.64	.00	2.93	4.62	5.000	.00	1.0
34.646	.0511					.0357	1.24	1.55	3.40	1.44	.013	.00	PIPE
-1591.457	3003.366	1.604	3004.970	106.00	19.49	5.90	3010.87	.00	2.93	4.67	5.000	.00	1.0
24.137	.0511					.0313	.75	1.60	3.18	1.44	.013	.00	PIPE
-1567.320	3004.601	1.661	3006.262	106.00	18.59	5.36	3011.63	.00	2.93	4.71	5.000	.00	1.0
13.852	.0511					.0278	.39	1.66	2.98	1.44	.013	.00	PIPE
-1553.468	3005.308	1.704	3007.012	106.00	17.94	5.00	3012.01	.00	2.93	4.74	5.000	.00	1.0
15.033	.0511					.0248	.37	1.70	2.83	1.44	.013	.00	PIPE

-1538.434	3006.076	1.764	3007.840	106.00	17.11	4.54	3012.39	60-OUT	0.00	2.93	4.78	5.000	.000	.00	1	.0
11.948	.0511	-	-	-	-	.0218	.26		1.76	2.65	1.44	.013	.00	.00	PIPE	
-1526.487	3006.686	1.827	3008.513	106.00	16.31	4.13	3012.65		.00	2.93	4.82	5.000	.000	.00	1	.0
9.679	.0511	-	-	-	-	.0191	.18		1.83	2.47	1.44	.013	.00	.00	PIPE	
-1516.808	3007.180	1.893	3009.073	106.00	15.55	3.76	3012.83		.00	2.93	4.85	5.000	.000	.00	1	.0
7.965	.0511	-	-	-	-	.0167	.13		1.89	2.31	1.44	.013	.00	.00	PIPE	
-1508.843	3007.587	1.961	3009.548	106.00	14.83	3.41	3012.96		.00	2.93	4.88	5.000	.000	.00	1	.0
6.553	.0511	-	-	-	-	.0147	.10		1.96	2.16	1.44	.013	.00	.00	PIPE	
-1502.290	3007.922	2.033	3009.955	106.00	14.14	3.10	3013.06		.00	2.93	4.91	5.000	.000	.00	1	.0
5.454	.0511	-	-	-	-	.0129	.07		2.03	2.02	1.44	.013	.00	.00	PIPE	
-1496.836	3008.200	2.107	3010.308	106.00	13.48	2.82	3013.13		.00	2.93	4.94	5.000	.000	.00	1	.0
4.517	.0511	-	-	-	-	.0113	.05		2.11	1.88	1.44	.013	.00	.00	PIPE	
-1492.319	3008.431	2.184	3010.615	106.00	12.85	2.57	3013.18		.00	2.93	4.96	5.000	.000	.00	1	.0
3.702	.0511	-	-	-	-	.0100	.04		2.18	1.76	1.44	.013	.00	.00	PIPE	
-1488.618	3008.620	2.265	3010.885	106.00	12.26	2.33	3013.22		.00	2.93	4.98	5.000	.000	.00	1	.0
3.001	.0511	-	-	-	-	.0087	.03		2.27	1.64	1.44	.013	.00	.00	PIPE	
-1485.617	3008.773	2.350	3011.124	106.00	11.69	2.12	3013.24		.00	2.93	4.99	5.000	.000	.00	1	.0
2.392	.0511	-	-	-	-	.0077	.02		2.35	1.53	1.44	.013	.00	.00	PIPE	

W S P G W - CIVILDESIGN Version 14.06
 Program Package Serial Number: 1704
 SKYE CANYON DETENTION BASIN, OLY1401.004
 TECHNICAL DRAINAGE STUDY UPDATE
 OUTLET PIPE

Date: 8-10-2016 Time: 3:46:53

60-OUT
 3012.39
 3012.65
 3012.83
 3012.96
 3013.06
 3013.13
 3013.18
 3013.22
 3013.24
 3013.26
 3013.27
 3013.28
 3014.53

***** OUTLET PIPE *****															
Station	Invert Elev	Depth (FT)	Water Elev	Q (cfs)	Vel (FPS)	Vel Head	Energy Grd.El.	Super Elev	Critical Depth	Flow Top width	Height/ Dia.-FT	Base wt or I.D.	ZL	No with Prs/pip	
L/Elem	Ch Slope					SF Ave	HF	SE Dpth	Froude N	Norm Dp	"N"	X-Fall	ZR	Type Ch	
*****	*****	*****	*****	*****	*****	*****	*****	*****	*****	*****	*****	*****	*****	*****	
-1483.225	3008.896	2.439	3011.335	106.00	11.14	1.93	3013.26	.00	2.93	5.00	5.000	.000	.00	1 .0	
1.856	.0511	-	-	-	-	.0068	.01	2.44	-	1.44	.013	.00	.00	PIPE	
-1481.369	3008.990	2.532	3011.522	106.00	10.62	1.75	3013.27	.00	2.93	5.00	5.000	.000	.00	1 .0	
1.369	.0511	-	-	-	-	.0060	.01	2.53	-	1.44	.013	.00	.00	PIPE	
-1480.000	3009.060	2.630	3011.690	106.00	10.13	1.59	3013.28	.00	2.93	4.99	5.000	.000	.00	1 .0	
JUNCT STR	.0000	-	-	-	-	.0166	.00	2.63	-	-	.013	.00	.00	PIPE	
-1480.000	3009.060	1.370	3010.431	71.00	16.26	4.10	3014.53	.00	2.38	4.46	5.000	.000	.00	1 .0	

* FILE: 60-OUT.WSW
 *

60-OUT-OUT									
Station	Invert Elev	Depth (FT)	Water Elev	Q (CFS)	Vel (FPS)	Vel Head	Energy Grd.EI.	Super Elev	Critical Depth
-1416.697	3012.297	2.140	3014.437	71.00	8.85	1.21	3015.65	.00	2.38
.679	.0511	-	-	-	-	.0048	.00	2.14	1.22
-1416.018	3012.331	2.219	3014.550	71.00	8.43	1.10	3015.65	.00	2.38
.368	.0511	-	-	-	-	.0042	.00	2.22	1.14

W S P G W - CIVILDESIGN Version 14.06

Program Package Serial Number: 1704

SKYE CANYON DETENTION BASIN, OLY1401.004

TECHNICAL DRAINAGE STUDY UPDATE

OUTLET PIPE

Station	Invert Elev	Depth (FT)	Water Elev	Q (CFS)	Vel (FPS)	Vel Head	Energy Grd.EI.	Super Elev	Critical Depth	Flow Top Width	Height/ Base Wt Dia.-FT	No Wth Prs/Pip
-1415.650	3012.350	2.302	3014.652	71.00	8.04	1.00	3015.66	.00	2.38	4.98	5.000	.00
75.123	.0040	-	-	-	-	.0040	.30	2.30	1.06	2.30	.013	.00
-1340.527	3012.647	2.302	3014.949	71.00	8.04	1.00	3015.95	.00	2.38	4.98	5.000	.00
46.367	.0040	-	-	-	-	.0040	.18	2.30	1.06	2.30	.013	.00
-1294.160	3012.830	2.293	3015.123	71.00	8.08	1.01	3016.14	.00	2.38	4.98	5.000	.00
215.432	.0040	-	-	-	-	.0040	.86	2.29	1.07	2.29	.013	.00
-1078.728	3013.694	2.293	3015.986	71.00	8.08	1.01	3017.00	.00	2.38	4.98	5.000	.00
26.578	.0040	-	-	-	-	.0038	.10	2.29	1.07	2.29	.013	.00
-1052.150	3013.800	2.379	3016.178	71.00	7.71	.92	3017.10	.00	2.38	4.99	5.000	.00

of 4

SKYE CANYON DETENTION BASIN, OLY1401.004

TECHNICAL DRAINAGE STUDY UPDATE

OUTLET PIPE

-1747.32 .I W C H E R

-1738.74 .

60-OUT.OUT

-1730.16

-1721.57

-1712.99

-1704.41

-1695.83

-1687.24

-1678.66

-1670.08

-1661.50

-1652.91

~1644.33

-1635.75

-1627.17

-1618.58

-1610.00

-1601.42

-1592.84

-1584.26

-1575.67

-1567.09

-1558.51

R

α

XC

R

2

R

2

60-OUT.OUT

-1549.93	.	I	W	C	H	E	.	R
-1541.34	.						.	
-1532.76	.	I	W	C	H	E	.	R
-1524.18	.	I	W	C	H	E	.	R
-1515.60	.	I	W	C	H	E	.	R
-1507.01	.	I	W	C	H	E	.	R
-1498.43	.	I	W	C	HE		.	R
-1489.85	.	I	W	C	EH		.	R
-1481.27	.	I	W	C	EH		.	R
-1472.68	.	I	W	C	EH		.	R
-1464.10	.	I	W	C	EH		.	R
-1455.52	.	I	W	C	EH		.	R
-1446.94	.	I	W	C	EH		.	R
-1438.36	.	I	WC	EH			.	JX
-1429.77	.	I	W	C	HE		.	R
-1421.19	.	I	W	C	HE		.	R
-1412.61	.	I	W	C	EH		.	R
-1404.03	.	I	W	C	EH		.	R
-1395.44	.	I	W	C	EH		.	R
-1386.86	.	I	W	C	EH		.	R

60-OUT.OUT

-1378.28	.	I	W	C	E	H	.	R
-1369.70	.	I	W	C	E	H	.	R
-1361.11	.	I	W	C	E	H	.	R
-1352.53	.	I	W	C	E	H	.	R
-1343.95	.	I	W	C	E	H	.	R
-1335.37	.	I	W	C	E	H	.	R
-1326.79	.	I	W	C	E	H	.	R
-1318.20	.	I	W	C	E	H	.	R
-1309.62	.	I	W	C	E	H	.	R
-1301.04	.	I	W	C	E	H	.	R
-1292.46	.	I	W	C	E	H	.	R
-1283.87	.	I	W	C	E	H	.	R
-1275.29	.						.	
-1266.71	.						.	
-1258.13	.						.	
-1249.54	.						.	
-1240.96	.						.	
-1232.38	.						.	
-1223.80	.						.	
-1215.21	.						.	

60-OUT.OUT

-1206.63	.
-1198.05	.
-1189.47	.
-1180.89	.
-1172.30	.
-1163.72	.
-1155.14	.
-1146.56	.
-1137.97	.
-1129.39	.
-1120.81	.
-1112.23	.
-1103.64	.
-1095.06	.
-1086.48	.
-1077.90	.
-1069.31	.
-1060.73	.
-1052.15	.

I	WC	E	H	R
			.	
			.	
I	W	E	H	R

2995.400	2997.740	3000.080	3002.420	3004.760	3007.100	3009.440	3011.781	3014.121	3016.461	3018.800
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60-OUT.OUT

N O T E S

1. GLOSSARY

I = INVERT ELEVATION
C = CRITICAL DEPTH
W = WATER SURFACE ELEVATION
S = SUPER-ELEVATION
H = HEIGHT OF CHANNEL
E = ENERGY GRADE LINE
X = CURVES CROSSING OVER
B = BRIDGE ENTRANCE OR EXIT
Y = WALL ENTRANCE OR EXIT

2. STATIONS FOR POINTS AT A JUMP MAY NOT BE PLOTTED EXACTLY

Section 1-b

Worksheet for Trapezoidal Channel

Project Description

Worksheet	Section 1-b Natural Wash @ End of Outlet SD
Flow Element	Trapezoidal Channel
Method	Manning's Formula
Solve For	Channel Depth

Input Data

Mannings Coefficient	0.035
Channel Slope	4.00 %
Left Side Slope	0.20 H : V
Right Side Slope	0.20 H : V
Bottom Width	8.00 ft
Discharge	113.00 cfs

Results

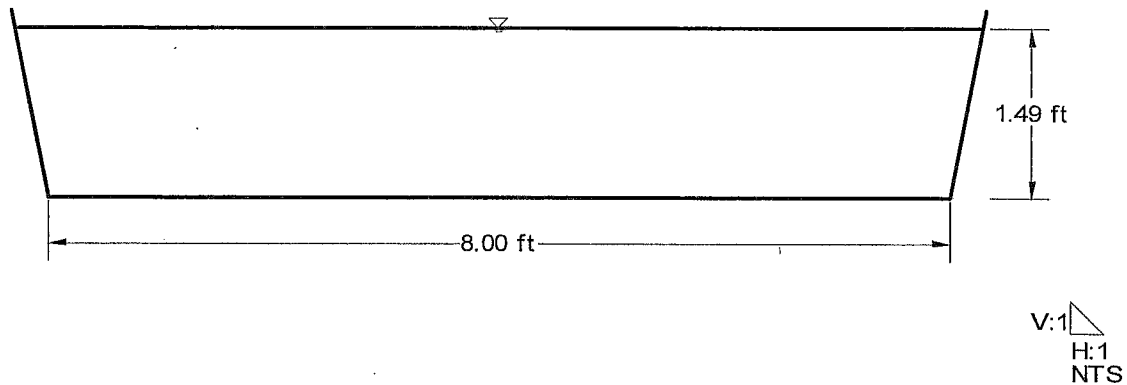
Depth	1.49 ft
Flow Area	12.3 ft ²
Wetted Perimeter	11.04 ft
Top Width	8.60 ft
Critical Depth	1.81 ft
Critical Slope	2.19 %
Velocity	9.15 ft/s
Velocity Head	1.30 ft
Specific Energy	2.79 ft
Froude Number	1.35
Flow Type	Supercritical

→ UTILIZED FOR STARTING WSE IN WSPG
& OUTLET PROTECTION GRAPH

Section 1-b **Cross Section for Trapezoidal Channel**

Project Description	
Worksheet	Section 1-b Natural Wash @ End of Outlet SD
Flow Element	Trapezoidal Channel
Method	Manning's Formula
Solve For	Channel Depth

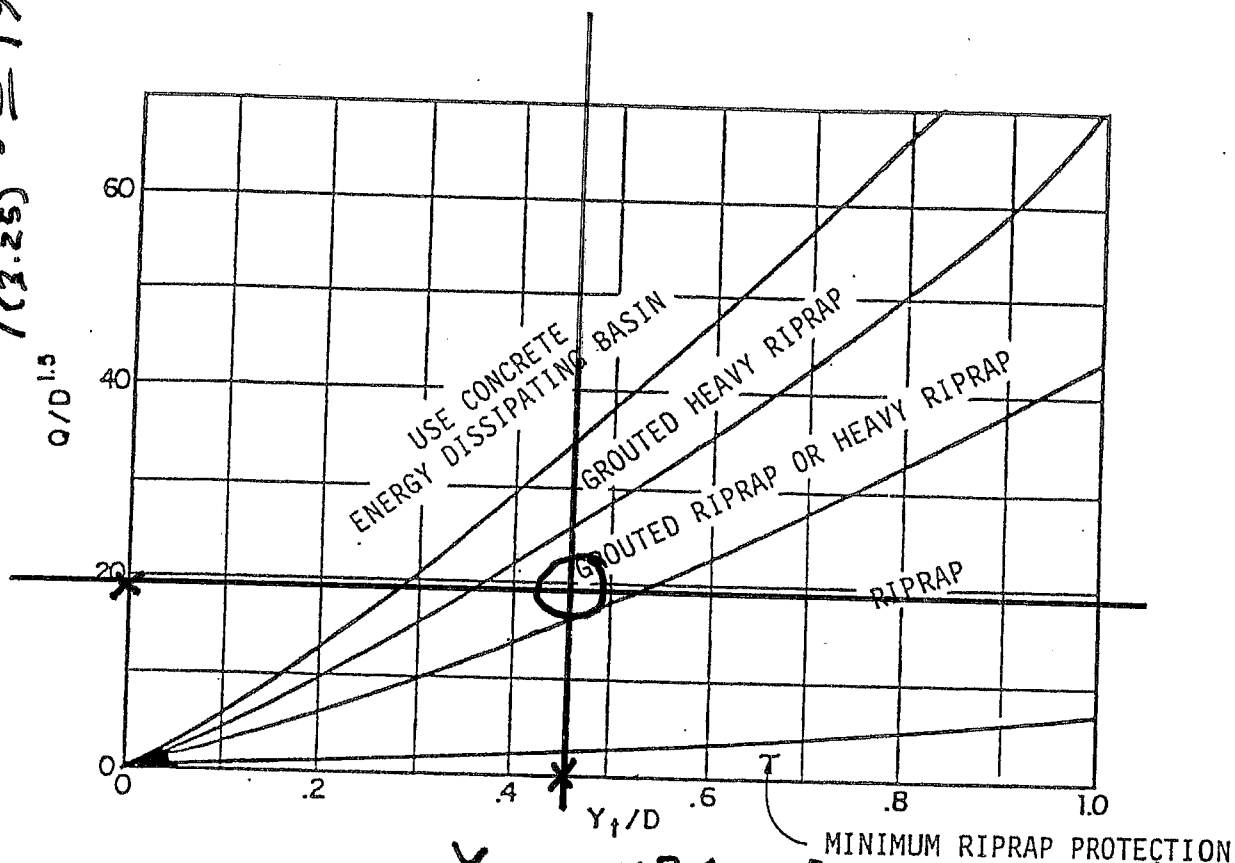
Section Data	
Mannings Coefficient	0.035
Channel Slope	4.00 %
Depth	1.49 ft
Left Side Slope	0.20 H : V
Right Side Slope	0.20 H : V
Bottom Width	8.00 ft
Discharge	113.00 cfs



RIPRAP EROSION PROTECTION AT CIRCULAR CONDUIT OUTLET

60" RCP @ 113 cfs

$$= 113 / (3.25)^{1.5} = 19.3$$



Notes:

$$0.46 = \frac{Y_t}{D_a} = \frac{1.49 \text{ (SEE FLOWMASTER 1-6)}}{3.25}$$

1. Use H_a instead of H whenever culvert has supercritical flow in the barrel.
2. Minimum Riprap Protection: Use Riprap for a distance of $3H$ downstream for 18" to 36" equivalent round pipe size and $4D$ downstream for 42" and larger equivalent round pipe size.

$$D_a = \frac{1}{2} (D + Y_n) = \frac{1}{2} (5 + 1.48) = 3.25'$$

$$\frac{Q}{D_a^{2.5}} = \frac{113}{(3.25)^{2.5}} = 5.9 < 6.0 \Rightarrow \text{FIG. 712 VALID}$$

CONCRETE UTILIZED IN LIEU; OK

*MAX. 50' EXTENT (10D) PER 707.4.3

Revision	Date

WRC
ENGINEERING

REFERENCE:

USDCM, DRCOG, March 1969 (with modifications)

FIGURE 712

APPENDIX F

Emergency Spillway Channel Normal Depth Computations

Section 1-a
Worksheet for Trapezoidal Channel

Project Description	
Worksheet	Section 1-a Spillway U/S
Flow Element	Trapezoidal Channel
Method	Manning's Formula
Solve For	Channel Depth

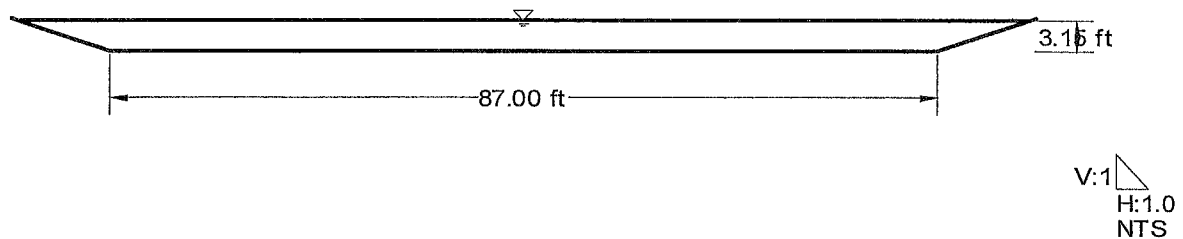
Input Data	
Mannings Coefficient	0.015
Channel Slope	1.00 %
Left Side Slope	3.00 H : V
Right Side Slope	3.00 H : V
Bottom Width	87.00 ft
Discharge	6,025.00 cfs

Results	
Depth	3.15 ft
Flow Area	303.4 ft ²
Wetted Perimeter	106.90 ft
Top Width	105.88 ft
Critical Depth	4.99 ft
Critical Slope	0.20 %
Velocity	19.86 ft/s
Velocity Head	6.13 ft
Specific Energy	9.27 ft
Froude Number	2.07
Flow Type	Supercritical

Section 1-a **Cross Section for Trapezoidal Channel**

Project Description	
Worksheet	Section 1-a Spillway U/S
Flow Element	Trapezoidal Channel
Method	Manning's Formula
Solve For	Channel Depth

Section Data	
Mannings Coefficient	0.015
Channel Slope	1.00 %
Depth	3.15 ft
Left Side Slope	3.00 H : V
Right Side Slope	3.00 H : V
Bottom Width	87.00 ft
Discharge	6,025.00 cfs



Section 1-f
Worksheet for Trapezoidal Channel

Project Description	
Worksheet	Section 1-f Spillway D/S
Flow Element	Trapezoidal Channel
Method	Manning's Formula
Solve For	Channel Depth

Input Data	
Mannings Coefficient	0.025
Channel Slope	3.20 %
Left Side Slope	100.00 H : V
Right Side Slope	100.00 H : V
Bottom Width	87.00 ft
Discharge	6,025.00 cfs

Results	
Depth	1.90 ft
Flow Area	524.0 ft ²
Wetted Perimeter	466.05 ft
Top Width	466.03 ft
Critical Depth	2.56 ft
Critical Slope	0.80 %
Velocity	11.50 ft/s
Velocity Head	2.05 ft
Specific Energy	3.95 ft
Froude Number	1.91
Flow Type	Supercritical

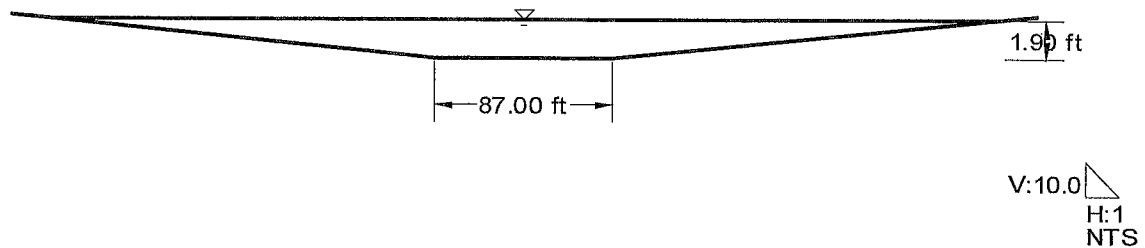
→ ENERGY GRADIENT CONTAINED BY 3034.0'
(3028.0 + 3.95) < 3034.0'

- 3034.0' PERPENDICULAR TO FL; NORTH OF
THE 34' CONTOUR, FLOW HAS BEGUN TO
TURN & ENERGY GRADIENT IS LESS.

Section 1-f **Cross Section for Trapezoidal Channel**

Project Description	
Worksheet	Section 1-f Spillway D/S
Flow Element	Trapezoidal Channel
Method	Manning's Formula
Solve For	Channel Depth

Section Data	
Mannings Coefficient	0.025
Channel Slope	3.20 %
Depth	1.90 ft
Left Side Slope	100.00 H : V
Right Side Slope	100.00 H : V
Bottom Width -	87.00 ft
Discharge	6,025.00 cfs



APPENDIX G

Sheep Mountain H&H Calculations

Hydrology Data & HEC-1 Output



0 0.5 1 1.5 2 2.5 3 3.5 4 4.5 5 5.5 6 6.5 7 7.5 8 8.5 9 9.5 10

Date: 8/9/2016

[1]	$K = 0.0132 (CN) - 0.39$	[5]	For Existing Conditions: $V2 = 29.4 * (S/100)^{0.5}$	[9]	$Tc = (L/180) + 10$
[2]	$A = Area/640$		For Developed Conditions: $V2 = 30.6 * (S/100)^{0.5}$	[10]	$Flag = (0.6 * Tc)/60$
[3]	$Tl = (1.8 (1.1-K) L^{0.5})/S^{0.33}$	[6]	$Tt = L/(60 * V)$	[11]	Min Flag = 0.05
[4]	For Existing Conditions: $V1 = 14.8 * (S/100)^{0.5}$	[7]	$Tc = Tl + Tt$	Note: For Travel Time Calcs: V1 applies to first 500 feet and V2 applies to remaining distance.	
	For Developed Conditions: $V1 = 20.2 * (S/100)^{0.5}$	[8]	Total L = Initial L + Travel L		

STANDARD FORM 4

STB.OUT

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*****
* FLOOD HYDROGRAPH PACKAGE (HEC-1)
* JUN 1998
* VERSION 4.1
* RUN DATE 08AUG16 TIME 14:37:33
*****

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*****
* U.S. ARMY CORPS OF ENGINEERS
* HYDROLOGIC ENGINEERING CENTER
* 609 SECOND STREET
* DAVIS, CALIFORNIA 95616
* (916) 756-1104
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THIS PROGRAM REPLACES ALL PREVIOUS VERSIONS OF HEC-1 KNOWN AS HEC1 (JAN 73), HEC1G5, HEC1D8, AND HEC1KW.

THE DEFINITIONS OF VARIABLES -RTIMP- AND -RTIOR- HAVE CHANGED FROM THOSE USED WITH THE 1973-STYLE INPUT STRUCTURE.
 THE DEFINITION OF -AMSK- ON RM-CARD WAS CHANGED WITH REVISIONS DATED 28 SEP 81. THIS IS THE FORTRAN77 VERSION
 NEW OPTIONS: DAMBREAK OUTFLOW SUBMERGENCE, SINGLE EVENT DAMAGE CALCULATION, DSS:WRITE STAGE FREQUENCY,
 DSS:READ TIME SERIES AT DESIRED CALCULATION INTERVAL, LOSS RATE:GREEN AND AMPT INFILTRATION
 KINEMATIC WAVE: NEW FINITE DIFFERENCE ALGORITHM

1 HEC-1 INPUT PAGE 1

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

```

*DIAGRAM
1 ID SLATER HANIFAN GROUP
2 ID NINETY-FIVE MANAGEMENT, LLC.
3 ID SKYE CANYON - DETENTION BASIN 1
4 ID TDS UPDATE - SHEEP MOUNTAIN
5 ID GRADED PARKWAY
6 ID
7 ID RETURN PERIOD ----- 100-YEAR
8 ID DISTRIBUTION ----- 6-HOUR SDN3
9 ID PROJECT NO. ----- OLY1401.004
10 ID FILENAME ----- STB.H1
11 ID DATE ----- 8/3/2016
12 ID MODELED BY ----- JSR
13 ID
14 ID SIX HOUR DEPTH-AREA REDUCTION FACTORS
15 ID DRAINAGE AREA (MI^2) RATIO
16 ID 0.0-0.49 1.00
17 ID
18 ID 5 0 0 300
19 ID 5 0 0
20 ID 5 0 0
21 ID JR PREC 1.00
22 ID *
23 ID KK SM1
24 ID KM DISCHARGES INTO OUTLET CHANNEL
25 ID PB 3.0
26 ID PC 0 0.020 0.057 0.070 0.087 0.108 0.124 0.130 0.130 0.130
27 ID PC 0.130 0.130 0.130 0.133 0.140 0.142 0.148 0.158 0.172 0.181
28 ID PC 0.190 0.197 0.199 0.200 0.201 0.204 0.214 0.229 0.241 0.249
29 ID PC 0.251 0.256 0.270 0.278 0.281 0.283 0.295 0.322 0.352 0.409
30 ID PC 0.499 0.590 0.710 0.744 0.781 0.812 0.819 0.835 0.851 0.856
31 ID PC 0.860 0.868 0.876 0.888 0.910 0.926 0.937 0.950 0.970 0.976
32 ID PC 0.982 0.985 0.987 0.989 0.990 0.993 0.993 0.994 0.995 0.998
33 ID PC 0.998 0.999 1
34 ID BA .0090
35 ID LS 0 94.0
36 ID UD 0.066
37 ID *
38 ID KK SM2
39 ID KM DISCHARGES INTO OUTLET CHANNEL
40 ID BA .0077
41 ID LS 0 94.0
42 ID UD 0.109
43 ID *
44 ID KK SM3
45 ID KM SWALE FLOW
46 ID BA .0032
47 ID LS 0 94.0
48 ID UD 0.077
49 ID *
50 ID ZZ

```

```

1 SCHEMATIC DIAGRAM OF STREAM NETWORK
INPUT LINE (V) ROUTING (--->) DIVERSION OR PUMP FLOW
NO. (.) CONNECTOR (<---) RETURN OF DIVERTED OR PUMPED FLOW
22 SM1
.
.
36 SM2
.
.
41 SM3
.
.

```

(***) RUNOFF ALSO COMPUTED AT THIS LOCATION

```

*****
* FLOOD HYDROGRAPH PACKAGE (HEC-1)
* JUN 1998
* VERSION 4.1
* RUN DATE 08AUG16 TIME 14:37:33
*****

```

```

*****
* U.S. ARMY CORPS OF ENGINEERS
* HYDROLOGIC ENGINEERING CENTER
* 609 SECOND STREET
* DAVIS, CALIFORNIA 95616
* (916) 756-1104
*****

```

SLATER HANIFAN GROUP
 NINETY-FIVE MANAGEMENT, LLC.
 SKYE CANYON - DETENTION BASIN 1
 TDS UPDATE - SHEEP MOUNTAIN
 GRADED PARKWAY
 RETURN PERIOD ----- 100-YEAR

DISTRIBUTION ----- 6-HOUR SDN3
 PROJECT NO. ----- OLY1401.004
 FILENAME ----- STB.H1
 DATE ----- 8/3/2016
 MODELED BY ----- JSR

STB.OUT

SIX HOUR DEPTH-AREA REDUCTION FACTORS
 DRAINAGE AREA (MIA2) RATIO
 0.0-0.49 1.00

19 IO

OUTPUT CONTROL VARIABLES

IPRNT 5 PRINT CONTROL
 IPLOT 0 PLOT CONTROL
 QSCAL 0. HYDROGRAPH PLOT SCALE

IT

HYDROGRAPH TIME DATA

NMIN 5 MINUTES IN COMPUTATION INTERVAL
 IDATE 1 0 STARTING DATE
 ITIME 0000 STARTING TIME
 NQ 300 NUMBER OF HYDROGRAPH ORDINATES
 NDDATE 2 0 ENDING DATE
 NUTIME 0055 ENDING TIME
 ICENT 19 CENTURY MARK

COMPUTATION INTERVAL .08 HOURS
 TOTAL TIME BASE 24.92 HOURS

ENGLISH UNITS

DRAINAGE AREA SQUARE MILES
 PRECIPITATION DEPTH INCHES
 LENGTH, ELEVATION FEET
 FLOW CUBIC FEET PER SECOND
 STORAGE VOLUME ACRE-Feet
 SURFACE AREA ACRES
 TEMPERATURE DEGREES FAHRENHEIT

JP

MULTI-PLAN OPTION

NPLAN 1 NUMBER OF PLANS

JR

MULTI-RATIO OPTION

RATIOS OF PRECIPITATION
 1.00

1

PEAK FLOW AND STAGE (END-OF-PERIOD) SUMMARY FOR MULTIPLE PLAN-RATIO ECONOMIC COMPUTATIONS
 FLOWS IN CUBIC FEET PER SECOND, AREA IN SQUARE MILES
 TIME TO PEAK IN HOURS

OPERATION	STATION	AREA	PLAN	RATIOS APPLIED TO PRECIPITATION
				RATIO 1 1.00
HYDROGRAPH AT				
+	SM1	.01	1 FLOW TIME	20. 3.50
HYDROGRAPH AT				
+	SM2	.01	1 FLOW TIME	15. 3.50
HYDROGRAPH AT				
+	SM3	.00	1 FLOW TIME	7. 3.50

*** NORMAL END OF HEC-1 ***

Ditch Hydraulic Calculations

Section 1-c
Worksheet for Triangular Channel

Project Description	
Worksheet	Section 1-c Swale @ Basin SM1
Flow Element	Triangular Channel
Method	Manning's Formula
Solve For	Channel Depth

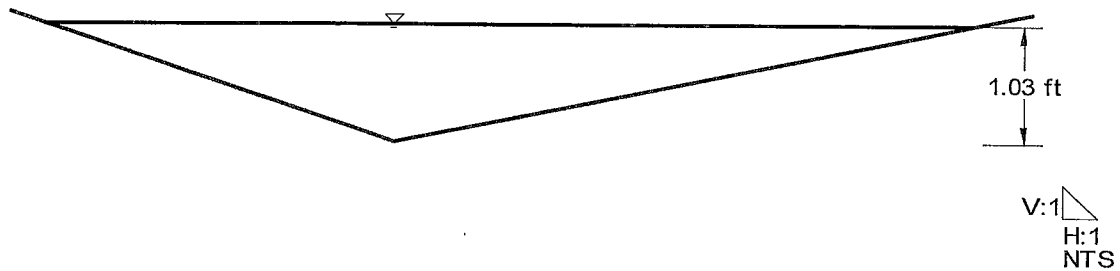
Input Data	
Mannings Coefficient	0.025
Channel Slope	1.60 %
Left Side Slope	3.00 H : V
Right Side Slope	5.00 H : V
Discharge	20.00 cfs

Results	
Depth	1.03 ft
Flow Area	4.2 ft ²
Wetted Perimeter	8.50 ft
Top Width	8.23 ft
Critical Depth	1.09 ft
Critical Slope	1.16 %
Velocity	4.73 ft/s
Velocity Head	0.35 ft
Specific Energy	1.38 ft
Froude Number	1.16
Flow Type	Supercritical

Section 1-c **Cross Section for Triangular Channel**

Project Description	
Worksheet	Section 1-c Swale @ Basin SM1
Flow Element	Triangular Channel
Method	Manning's Formula
Solve For	Channel Depth

Section Data	
Mannings Coefficient	0.025
Channel Slope	1.60 %
Depth	1.03 ft
Left Side Slope	3.00 H : V
Right Side Slope	5.00 H : V
Discharge	20.00 cfs



Section 1-d
Worksheet for Triangular Channel

Project Description	
Worksheet	Section 1-d Swale @ Basin SM2
Flow Element	Triangular Channel
Method	Manning's Formula
Solve For	Channel Depth

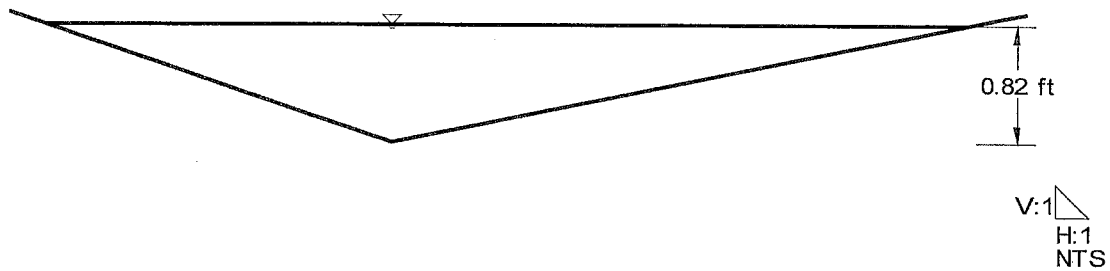
Input Data	
Mannings Coefficient	0.025
Channel Slope	3.00 %
Left Side Slope	3.00 H : V
Right Side Slope	5.00 H : V
Discharge	15.00 cfs

Results	
Depth	0.82 ft
Flow Area	2.7 ft ²
Wetted Perimeter	6.78 ft
Top Width	6.57 ft
Critical Depth	0.97 ft
Critical Slope	1.21 %
Velocity	5.57 ft/s
Velocity Head	0.48 ft
Specific Energy	1.30 ft
Froude Number	1.53
Flow Type	Supercritical

Section 1-d
Cross Section for Triangular Channel

Project Description	
Worksheet	Section 1-d Swale @ Basin SM2
Flow Element	Triangular Channel
Method	Manning's Formula
Solve For	Channel Depth

Section Data	
Mannings Coefficient	0.025
Channel Slope	3.00 %
Depth	0.82 ft
Left Side Slope	3.00 H : V
Right Side Slope	5.00 H : V
Discharge	15.00 cfs



Section 1-e
Worksheet for Triangular Channel

Project Description

Worksheet	Section 1-e Swale @ Basin SM3
Flow Element	Triangular Channel
Method	Manning's Formula
Solve For	Channel Depth

Input Data

Mannings Coefficient	0.025
Channel Slope	3.25 %
Left Side Slope	6.00 H : V
Right Side Slope	3.00 H : V
Discharge	7.00 cfs

Results

Depth	0.58 ft
Flow Area	1.5 ft ²
Wetted Perimeter	5.37 ft
Top Width	5.22 ft
Critical Depth	0.68 ft
Critical Slope	1.35 %
Velocity	4.62 ft/s
Velocity Head	0.33 ft
Specific Energy	0.91 ft
Froude Number	1.51
Flow Type	Supercritical

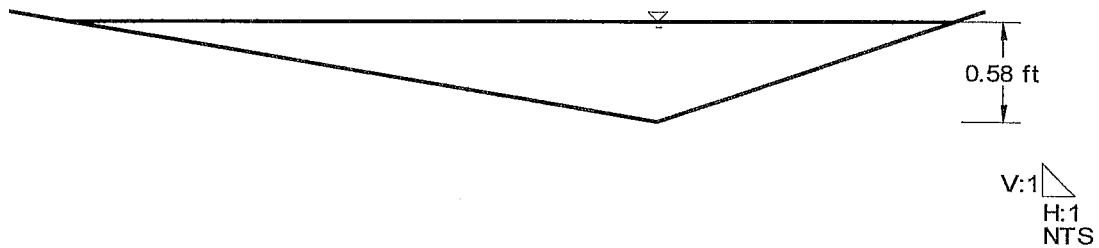
Section 1-e
Cross Section for Triangular Channel

Project Description

Worksheet	Section 1-e Swale @ Basin SM3
Flow Element	Triangular Channel
Method	Manning's Formula
Solve For	Channel Depth

Section Data

Mannings Coefficient	0.025
Channel Slope	3.25 %
Depth	0.58 ft
Left Side Slope	6.00 H : V
Right Side Slope	3.00 H : V
Discharge	7.00 cfs



Drop Inlet Calculations

Project Name: Skye Canyon Detention Basin 1
By: SMJ
Date: 8/8/2016

Grate Inlet Headwater Depth Computation

Notes:

Facility ID: Drop Inlets in Sheep Mountain Parkway
Drainage Condition: Graded Sheep Mountain Parkway
Source of Flow: 100-yr Flow produced by SM1
Inlet Type: NDOT Type 2B
Inlet Size: 2-ft x 4-ft (36-inch RCP out)

Known Information:

Q = Flow (cfs)	20.0
W = Width of Grate (ft)	4.00
L = Length of Grate (ft)	2.00
D = Diameter of Circle (in)	
Cf = Clogging Factor (%)	50

Weir Conditions:

$$HW = (Q/(Cw \cdot Pc))^{2/3}$$

Cw = Weir Coefficient	3.00
P = Perimeter of grate (ft)	12.0
Pc = Perimeter (w/clogging) (ft)	6.0
HW = Headwater depth (ft)	1.07

Orifice Conditions:

$$HW = (Q/(Co \cdot Ac))^2 / 2 \cdot g$$

Co = Orifice Coefficient	0.65
Ag = Grate Area (ft ²)	8.00
Gf = Grate Opening Factor	0.67
Ac = Grate Open Area (w/ clogging) (ft ²)	2.68
g = Gravity (fps)	32.20
HW = Headwater depth (ft)	2.05

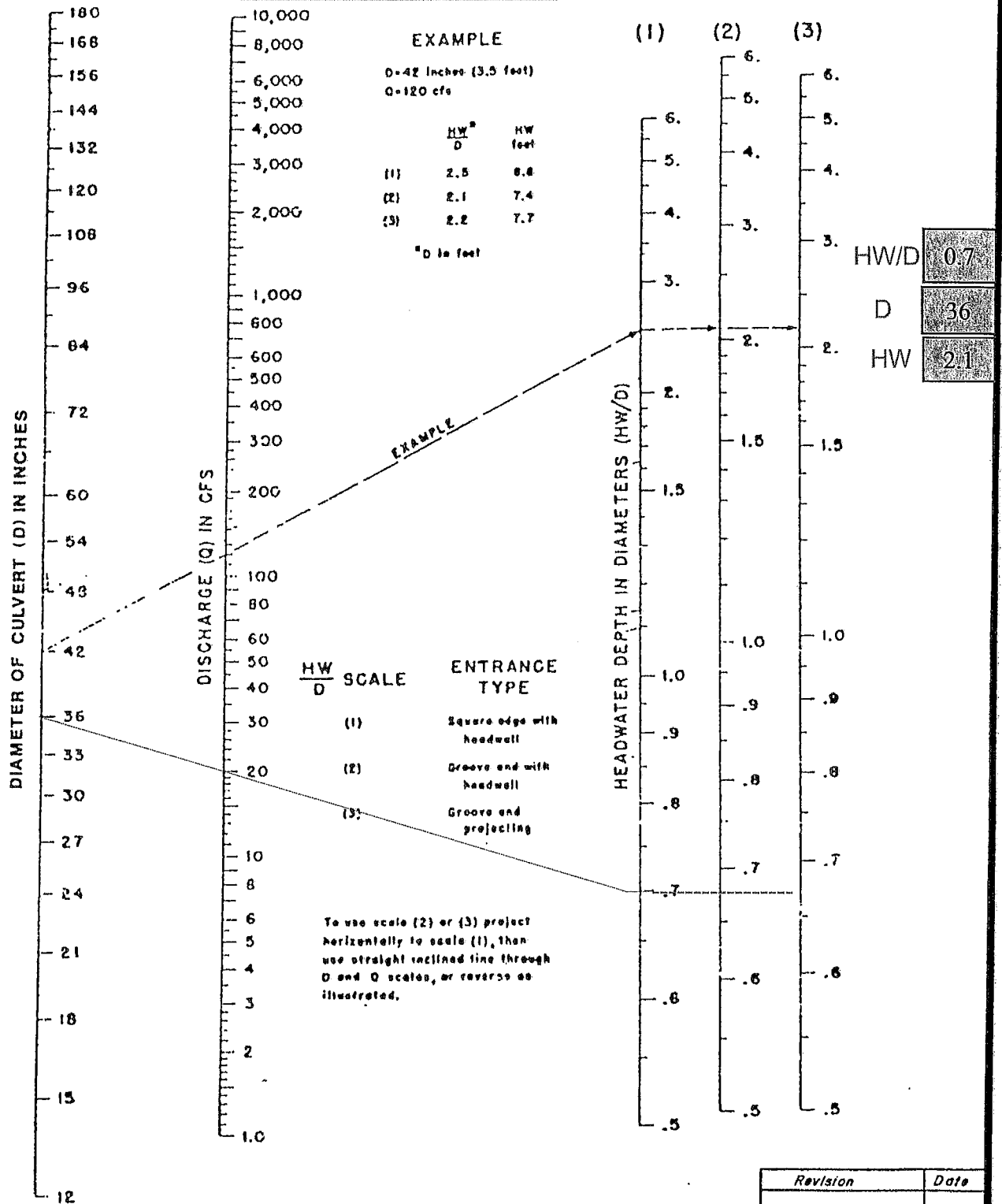
Output:

Worst Case Scenario Occurs Under Orifice Conditions

Headwater Depth = 2.0 ft with grate

NOMOGRAPH - INLET CONTROL RCP

SM1 LATERAL Q=20 CFS



Revision	Date

Project Name: Skye Canyon Detention Basin 1
By: SMJ
Date: 8/8/2016



Grate Inlet Headwater Depth Computation

Notes:

Facility ID: Drop Inlets in Sheep Mountain Parkway
Drainage Condition: Graded Sheep Mountain Parkway
Source of Flow: 100-yr Flow produced by SM2
Inlet Type: NDOT Type 2B
Inlet Size: 2-ft x 3.5-ft (30-inch RCP out)

Known Information:

Q = Flow (cfs)	15.0
W = Width of Grate (ft)	3.50
L = Length of Grate (ft)	2.00
D = Diameter of Circle (in)	
Cf = Clogging Factor (%)	50

Weir Conditions:

$$HW = (Q/(Cw*Pc))^{2/3}$$

Cw = Weir Coefficient	3.00
P = Perimeter of grate (ft)	11.0
Pc = Perimeter (w/clogging) (ft)	5.5
HW = Headwater depth (ft)	0.94

Orifice Conditions:

$$HW = (Q/(Co*Ac))^2/2*g$$

Co = Orifice Coefficient	0.65
Ag = Grate Area (ft ²)	7.00
Gf = Grate Opening Factor	0.67
Ac = Grate Open Area (w/ clogging) (ft ²)	2.35
g = Gravity (fps)	32.20
HW = Headwater depth (ft)	1.50

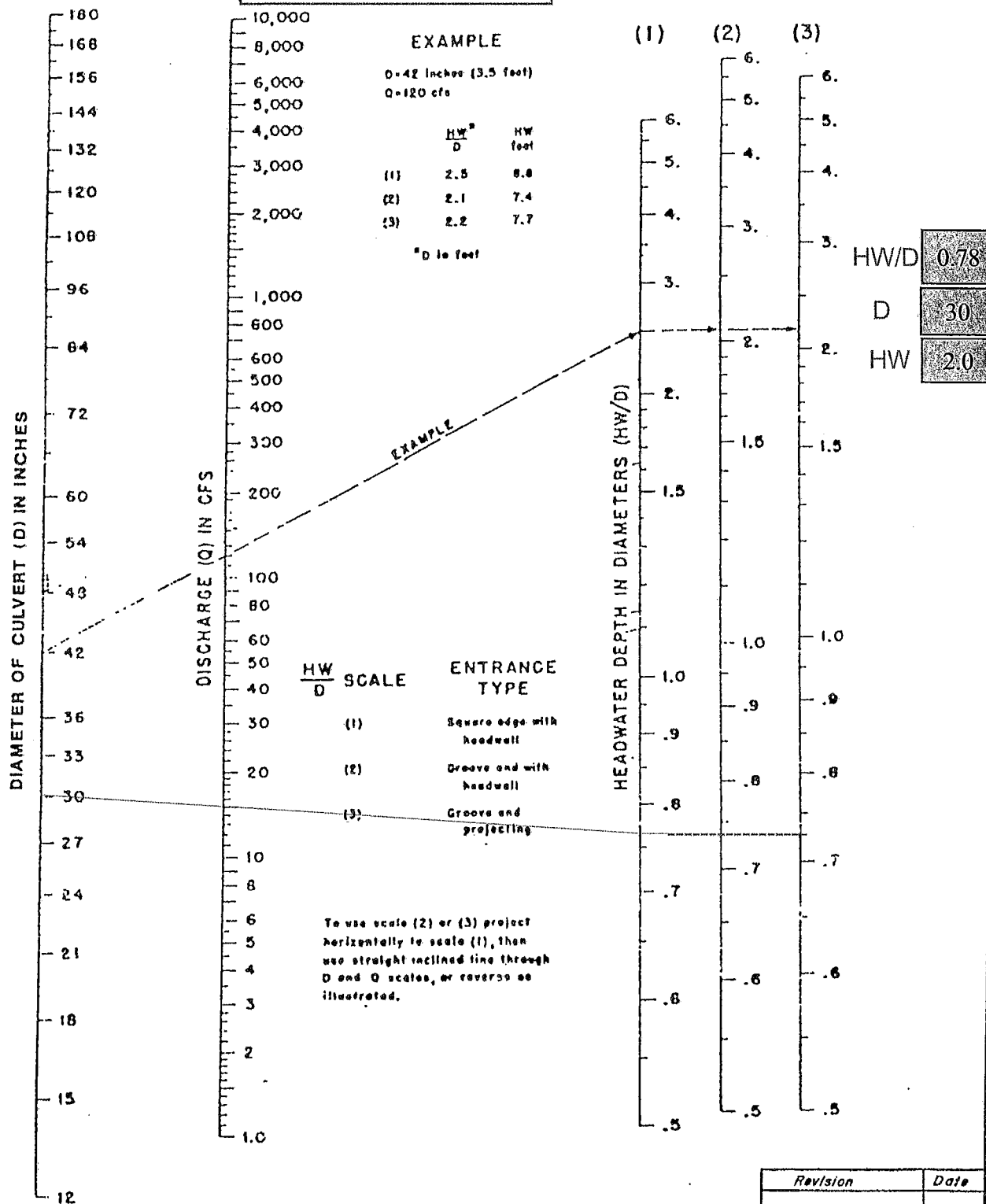
Output:

Worst Case Scenario Occurs Under Orifice Conditions

Headwater Depth = 1.5 ft with grate

NOMOGRAPH - INLET CONTROL RCP

SM2 LATERAL: Q=15 CFS



Revision	Date

Project Name: Skye Canyon Detention Basin 1
By: SMJ
Date: 8/8/2016



Grate Inlet Headwater Depth Computation

Notes:

Facility ID: Drop Inlets in Sheep Mountain Parkway
Drainage Condition: Graded Sheep Mountain Parkway
Source of Flow: 100-yr Flow produced by SM3
Inlet Type: NDOT Type 2B
Inlet Size: 2-ft x 3-ft (24-inch RCP out)

Known Information:

Q = Flow (cfs)	7.0
W = Width of Grate (ft)	3.00
L = Length of Grate (ft)	2.00
D = Diameter of Circle (in)	
Cf = Clogging Factor (%)	50

Weir Conditions:

$$HW = (Q / (C_w * P_c))^{2/3}$$

Cw = Weir Coefficient	3.00
P = Perimeter of grate (ft)	10.0
Pc = Perimeter (w/clogging) (ft)	5.0
HW = Headwater depth (ft)	0.60

Orifice Conditions:

$$HW = (Q / (C_o * A_c))^{2/2 * g}$$

Co = Orifice Coefficient	0.65
Ag = Grate Area (ft ²)	6.00
Gf = Grate Opening Factor	0.67
Ac = Grate Open Area (w/ clogging) (ft ²)	2.01
g = Gravity (fps)	32.20
HW = Headwater depth (ft)	0.45

Output:

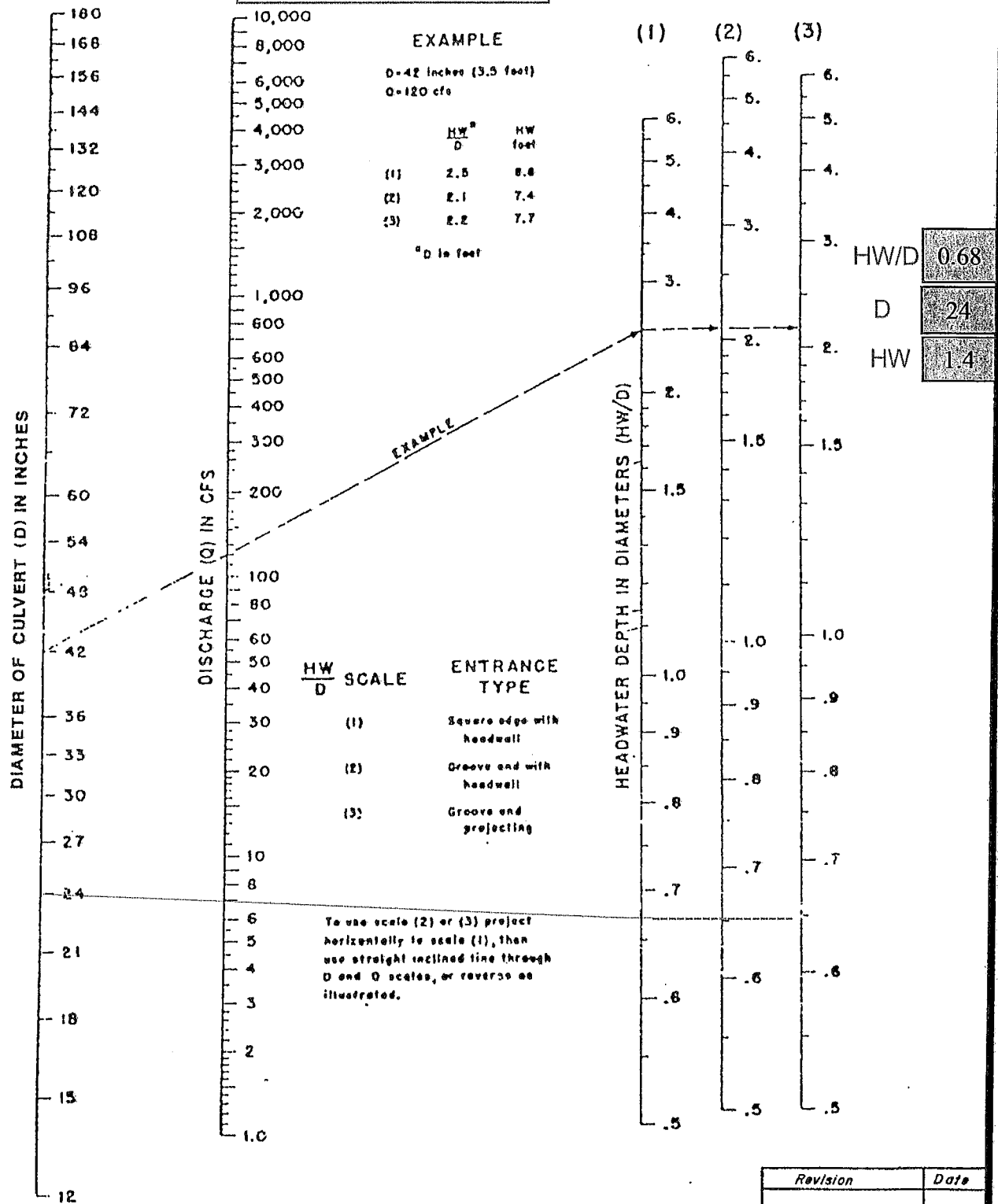
Worst Case Scenario Occurs Under Weir Conditions

Headwater Depth = 0.6 ft with grate

HYDROLOGIC CRITERIA AND DRAINAGE DESIGN MANUAL

NOMOGRAPH - INLET CONTROL RCP

SM3 LATERAL: Q=7 CFS



Revision	Date

WRC
ENGINEERING

REFERENCE:

USDOT, FHWA, HDS, No. 5, 1985

FIGURE 1005

Storm Drain WSPG Hydraulic Analyses
(Laterals to Trunk Outlet Pipe)

SM1.WSW

T1 SKYE CANYON DETENTION BASIN, OLY1401.004
T2 TECHNICAL DRAINAGE STUDY UPDATE
T3 LATERAL FOR SM1

SO	-1074.3503011.060	36				3014.060
R	-1050.3503011.260	36	.013			
JX	-1050.3503011.260	36	.013	15.0		3011.76
R	-1038.3503011.460	36	.013			90.
SH	-1038.3503011.460	36	.013			
CD	30	4	1	.000	.000	.00
Q	36	4	1	.000	.000	.00
			20.000	.0		

* STARTING WSE (3014.06') ASSUMED TOP OF PIPE, DUE TO
TRUNK HGL NOT PRODUCING TAILWATER.

sm1.OUT

-1071.41 .
-1070.68 .
-1069.94 .
-1069.21 .
-1068.47 .
-1067.74 .
-1067.00 .
-1066.27 .
-1065.53 .
-1064.80 .
-1064.06 .
-1063.33 .
-1062.59 .
-1061.86 .
-1061.13 .
-1060.39 .
-1059.66 .
-1058.92 .
-1058.19 .

sm1.out

-1057.45	.
-1056.72	.
-1055.98	.
-1055.25	.
-1054.51	.
-1053.78	.
-1053.04	.
-1052.31	.
-1051.57	.
-1050.84	.
-1050.11	.
-1049.37	.
-1048.64	.
-1047.90	.
-1047.17	.
-1046.43	.
-1045.70	.
-1044.96	.
-1044.23	.

I	C	W	H	E	.	JX
I	C	H	H	E	.	R

[illegible]

1. GLOSSARY

2. STATIONS FOR POINTS AT A JUMP MAY NOT BE PLOTTED EXACTLY

SM2. WSW

T1	SKYE CANYON DETENTION BASIN, OLY1401.004			
T2	TECHNICAL DRAINAGE STUDY UPDATE			
T3	LATERAL FOR SM2			3014.11
SO	-1037.0003011.760 30			
R	-1015.0003011.880 30	.013		
SH	-1015.0003011.880 30	.013		
CD	30 4 1 .000 2.5	.000	.000	.00
Q		15.000	.0	

FILE: SM2.WSW

SM2.EDT

Date: 8-10-2016 Time: 1:31: 6

Version 14.06

W S P G W

WATER SURFACE PROFILE - TITLE CARD LISTING

W S P G W

W S P G W

Card Sect Chn No of Ave Pier Height I Base ZL ZR INV Y(1) Y(2) Y(3) Y(4) Y(5) Y(6) Y(7) Y(8) Y(9) Y(10)

Channel Definition Listing

Water Surface Profile - Title Card Listing

Water Surface Profile - Title Card Listing

Water Surface Profile - Title Card Listing

CD 30 4 1 2.500

SKYE CANYON DETENTION BASIN, OLY1401.004

TECHNICAL DRAINAGE STUDY UPDATE

LATERAL FOR SM2

LATERAL FOR SM2

Element No 1 IS A SYSTEM OUTLET

Element No 2 IS A REACH

Element No 3 IS A SYSTEM HEADWORKS

Element No 1 IS A SYSTEM OUTLET

Element No 2 IS A REACH

Element No 3 IS A SYSTEM HEADWORKS

Station Invert Sect

Station Invert Sect

Station Invert Sect

Station Invert Sect

Station Invert Sect

W S Elev 3014.110

Radius .000

Angle .000

Ang Pt Man H

Ang Pt Man H

W S Elev .000

W S Elev .000

W S Elev .000

W S Elev .000

W S Elev .000

W S Elev .000

W S Elev .000

W S Elev .000

W S Elev .000

W S Elev .000

W S Elev .000

W S Elev .000

W S Elev .000

W S Elev .000

W S Elev .000

SKYE CANYON DETENTION BASIN, OLY1401.004

TECHNICAL DRAINAGE STUDY UPDATE

LATERAL FOR SM2

Station	Invert Elev	Depth (FT)	Water Elev	Q (CFS)	Vel (FPS)	Vel Head	Energy Grd.El.	Super Elev	Critical Depth	Flow Top Width	Height/Dia.-FT	Base Wt or I.D.	ZL	No With Pys/Pip
L/Elem	Ch Slope					SF Ave	HF	SE Dpth	Froude N	Norm Dp	"N"	X-Fall	ZR	Type Ch
-1037.000	3011.760	2.350	3014.110	15.00	3.13	.15	3014.26	.00	1.31	1.19	2.500	.000	.00	1 .0
22.000	.0054					.0012	.03	2.35	.27	1.24	.013	.00	.00	PIPE
-1015.000	3011.880	2.247	3014.126	15.00	3.23	.16	3014.29	.00	1.31	1.51	2.500	.000	.00	1 .0

** WARNING NO. 22 ** - NO PLOT GENERATED, BAD DATA OR NOT ENOUGH POINTS, 3 OR LESS

SM3.WSW

T1 SKYE CANYON DETENTION BASIN, OLY1401.004

T2 TECHNICAL DRAINAGE STUDY UPDATE

T3 LATERAL FOR SM2

SO -1050.0003003.790 24 3005.79

R -1030.0003009.790 24 .013

SH -1030.0003009.790 24 .013

CD 24 4 1 .000 2.0 .000 .000 .000 .00

Q 7.000 .0

* STARTING WSE (3005.79') ASSUMED TOP OF PIPE, DUE TO
TRUNK HGL NOT PRODUCING TAILWATER.

FILE: SM3.WSW

SM3.EDT
W S P G W - EDIT LISTING - Version 14.06
WATER SURFACE PROFILE - CHANNEL DEFINITION LISTING
CARD SECT CHN NO OF AVE PIER HEIGHT 1 BASE ZL ZR INV Y(1) Y(2) Y(3) Y(4) Y(5) Y(6) Y(7) Y(8) Y(9) Y(10)
CODE NO TYPE PIER/PIP WIDTH DIAMETER WIDTH DROP
Date: 8-10-2016 Time: 1:31:0
PAGE 1

CD 24 4 1

2.000

W S P G W

WATER SURFACE PROFILE - TITLE CARD LISTING

SKYE CANYON DETENTION BASIN, OLY1401.004

TECHNICAL DRAINAGE STUDY UPDATE

LATERAL FOR SM2

W S P G W

WATER SURFACE PROFILE - ELEMENT CARD LISTING

ELEMENT NO 1 IS A SYSTEM OUTLET *
U/S DATA STATION INVERT SECT

-1050.000 3003.790 24 *

ELEMENT NO 2 IS A REACH
U/S DATA STATION INVERT SECT

-1030.000 3009.790 24 *

ELEMENT NO 3 IS A SYSTEM HEADWORKS
U/S DATA STATION INVERT SECT

-1030.000 3009.790 24 *

W S ELEV
3005.790

RADIUS
.000

ANGLE
.000

ANG PT MAN H
.000 0

W S ELEV
.000

*

N
.013

Program Package Serial Number: 1704
WATER SURFACE PROFILE LISTING

Date: 8-10-2016 Time: 1:31: 2

SKYE CANYON DETENTION BASIN, OLY1401.004
TECHNICAL DRAINAGE STUDY UPDATE

***** LATERAL FOR SM2 *****														*****			
Station	Invert Elev	Depth (FT)	Water Elev	Q (CFS)	Vel (FPS)	Vel Head	Energy Grd.El.	Super Elev	Critical Depth	Flow Width	Height/Dia.-FT	Base Wt or I.D.	ZL	No Wth Prs/Pip	Type Ch	*****	
L/Elem	Ch Slope					SF Ave	HF	SE Dpth	Froude N	Norm Dp	"N"	X-Fall	ZR			*****	
-1050.000	3003.790	.365	3004.155	7.00	17.85	4.95	3009.10	.00	.94	1.54	2.000	.000	.00	1.0		*****	
1.325	.3000					.1766	.23		6.24	.32	.013	.00	.00	PIPE		*****	
-1048.676	3004.187	.369	3004.556	7.00	17.53	4.77	3009.33	.00	.94	1.55	2.000	.000	.00	1.0		*****	
3.029	.3000					.1611	.49		6.09	.32	.013	.00	.00	PIPE		*****	
-1045.646	3005.096	.382	3005.478	7.00	16.71	4.34	3009.81	.00	.94	1.57	2.000	.000	.00	1.0		*****	
2.396	.3000					.1409	.34		5.71	.32	.013	.00	.00	PIPE		*****	
-1043.250	3005.815	.395	3006.210	7.00	15.93	3.94	3010.15	.00	.94	1.59	2.000	.000	.00	1.0		*****	
1.952	.3000					.1231	.24		5.35	.32	.013	.00	.00	PIPE		*****	
-1041.298	3006.400	.408	3006.809	7.00	15.19	3.58	3010.39	.00	.94	1.61	2.000	.000	.00	1.0		*****	
1.620	.3000					.1075	.17		5.01	.32	.013	.00	.00	PIPE		*****	
-1039.678	3006.886	.422	3007.308	7.00	14.48	3.26	3010.57	.00	.94	1.63	2.000	.000	.00	1.0		*****	
1.370	.3000					.0939	.13		4.69	.32	.013	.00	.00	PIPE		*****	
-1038.309	3007.297	.436	3007.733	7.00	13.81	2.96	3010.70	.00	.94	1.65	2.000	.000	.00	1.0		*****	
1.167	.3000					.0821	.10		4.39	.32	.013	.00	.00	PIPE		*****	
-1037.142	3007.647	.451	3008.098	7.00	13.17	2.69	3010.79	.00	.94	1.67	2.000	.000	.00	1.0		*****	
1.007	.3000					.0718	.07		4.12	.32	.013	.00	.00	PIPE		*****	
-1036.135	3007.949	.466	3008.415	7.00	12.56	2.45	3010.86	.00	.94	1.69	2.000	.000	.00	1.0		*****	
.870	.3000					.0627	.05		3.85	.32	.013	.00	.00	PIPE		*****	

Program Package Serial Number: 1704
WATER SURFACE PROFILE LISTING

Date: 8-10-2016 Time: 1:31: 2

SKYE CANYON DETENTION BASIN, OLY1401.004
TECHNICAL DRAINAGE STUDY UPDATE

***** LATERAL FOR SM2 *****														*****			
Station	Invert Elev	Depth (FT)	Water Elev	Q (CFS)	Vel (FPS)	Vel Head	Energy Grd.El.	Super Elev	Critical Depth	Flow Width	Height/Dia.-FT	Base Wt or I.D.	ZL	No Wth Prs/Pip	Type Ch	*****	
L/Elem	Ch Slope					SF Ave	HF	SE Dpth	Froude N	Norm Dp	"N"	X-Fall	ZR			*****	

-1035.265	3008.210	.482	3008.693	7.00	11.97	2.23	3010.92	.00	.94	1.71	2.000	.000	.00	1	.0
.756	.3000	-	-	-	-	.0549	.04	.48	3.61	.32	.013	.00	.00	PIPE	
-1034.509	3008.437	.499	3008.936	7.00	11.41	2.02	3010.96	.00	.94	1.73	2.000	.000	.00	1	.0
.662	.3000	-	-	-	-	.0480	.03	.50	3.38	.32	.013	.00	.00	PIPE	
-1033.847	3008.636	.516	3009.152	7.00	10.88	1.84	3010.99	.00	.94	1.75	2.000	.000	.00	1	.0
.578	.3000	-	-	-	-	.0420	.02	.52	3.16	.32	.013	.00	.00	PIPE	
-1033.269	3008.809	.534	3009.344	7.00	10.38	1.67	3011.02	.00	.94	1.77	2.000	.000	.00	1	.0
.505	.3000	-	-	-	-	.0367	.02	.53	2.96	.32	.013	.00	.00	PIPE	
-1032.763	3008.961	.553	3009.514	7.00	9.89	1.52	3011.03	.00	.94	1.79	2.000	.000	.00	1	.0
.445	.3000	-	-	-	-	.0322	.01	.55	2.77	.32	.013	.00	.00	PIPE	
-1032.318	3009.094	.572	3009.667	7.00	9.43	1.38	3011.05	.00	.94	1.81	2.000	.000	.00	1	.0
.388	.3000	-	-	-	-	.0281	.01	.57	2.59	.32	.013	.00	.00	PIPE	
-1031.930	3009.211	.592	3009.803	7.00	8.99	1.26	3011.06	.00	.94	1.83	2.000	.000	.00	1	.0
.342	.3000	-	-	-	-	.0246	.01	.59	2.43	.32	.013	.00	.00	PIPE	
-1031.588	3009.313	.612	3009.926	7.00	8.58	1.14	3011.07	.00	.94	1.84	2.000	.000	.00	1	.0
.294	.3000	-	-	-	-	.0216	.01	.61	2.27	.32	.013	.00	.00	PIPE	
-1031.294	3009.402	.634	3010.036	7.00	8.18	1.04	3011.07	.00	.94	1.86	2.000	.000	.00	1	.0
.257	.3000	-	-	-	-	.0189	.00	.63	2.12	.32	.013	.00	.00	PIPE	

9 FILE: SM3.WSW

W S P G W - CIVILDESIGN Version 14.06
Program Package Serial Number: 1704

SKYE CANYON DETENTION BASIN, OLY1401.004

WATER SURFACE PROFILE LISTING

TECHNICAL DRAINAGE STUDY UPDATE

LATERAL FOR SM2

Date: 8-10-2016 Time: 1:31: 2

***** LATERAL FOR SMZ *****															*****	
Station	Invert Elev	Depth (FT)	Water Elev	Q (CFS)	Vel (FPS)	Vel Head	Energy Grd.El.	Super Elev	Critical Depth	Flow Top Width	Height/ Dia.-FT	Base Wt or I.D.	ZL	No Wth Prs/Pip		
L/Elem *****	Ch slope *****	*****	*****	*****	*****	SF Ave *****	HF *****	SE Dpth *****	Froude N *****	Norm Dp *****	"N" *****	X-Fall *****	ZR *****	Type Ch *****		
-1031.037	3009.479	.656	3010.135	7.00	7.80	.94	3011.08	.00	.94	1.88	2.000	.000	.00	1 .0		
.222	.3000	-	-	-	-	.0165	.00	.66	1.99	.32	.013	.00	.00	PIPE		
-1030.815	3009.545	.679	3010.224	7.00	7.43	.86	3011.08	.00	.94	1.89	2.000	.000	.00	1 .0		
.186	.3000	-	-	-	-	.0145	.00	.68	1.86	.32	.013	.00	.00	PIPE		
-1030.630	3009.601	.704	3010.305	7.00	7.09	.78	3011.08	.00	.94	1.91	2.000	.000	.00	1 .0		
.160	.3000	-	-	-	-	.0127	.00	.70	1.74	.32	.013	.00	.00	PIPE		
-1030.470	3009.649	.729	3010.378	7.00	6.76	.71	3011.09	.00	.94	1.93	2.000	.000	.00	1 .0		

SM3.OUT									
-133	.3000	-	-	-	-	.0111	-	-	-
-1030.337	3009.689	.755	3010.444	7.00	6.44	.64	3011.09	.00	.00
.109	.3000	-	-	-	-	.0098	.00	.00	.00
-1030.228	3009.721	.782	3010.503	7.00	6.14	.59	3011.09	.00	.00
.083	.3000	-	-	-	-	.0086	.00	.00	.00
-1030.144	3009.746	.811	3010.557	7.00	5.86	.53	3011.09	.00	.00
.066	.3000	-	-	-	-	.0075	.00	.00	.00
-1030.078	3009.766	.840	3010.606	7.00	5.58	.48	3011.09	.00	.00
.044	.3000	-	-	-	-	.0066	.00	.00	.00
-1030.034	3009.780	.871	3010.651	7.00	5.32	.44	3011.09	.00	.00
.027	.3000	-	-	-	-	.0058	.00	.00	.00
FILE: SM3.WSW									

W S P G W - CIVILDESIGN Version 14.06
 Program Package Serial Number: 1704
 SKYE CANYON DETENTION BASIN, OLY1401.004
 TECHNICAL DRAINAGE STUDY UPDATE
 LATERAL FOR SM2

Date: 8-10-2016 Time: 1:31: 2

Station	Invert Elev	Depth (FT)	Water Elev	Q (CFS)	Vel (FPS)	Vel Head	Energy Grd.El.	Super Elev	Critical Depth	Flow Top width	Height/ Base wt Dia.-FT	I.D.	ZL	No Wth Pps/Pip
L/Elem	Ch Slope					SF Ave	HF	SE Dpth	Froude N	Norm Dp	"N"	X-Fall	ZR	Type Ch
-1030.006	3009.788	.903	3010.691	7.00	5.08	.40	3011.09	.00	.94	1.99	2.000	.000	.00	1 .0
.006	.3000	-	-	-	-	.0051	.00	.90	1.08	.32	.013	.00	.00	PIPE
-1030.000	3009.790	.939	3010.729	7.00	4.83	.36	3011.09	.00	.94	2.00	2.000	.000	.00	1 .0
		-	-	-	-	-	-	-	-	-	-	-	-	

SKYE CANYON DETENTION BASIN, OLY1401.004

TECHNICAL DRAINAGE STUDY UPDATE

LATERAL FOR SM2

-1050.00 .I W C H E R
 -1049.65 .

-1042.28

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Year	2003	2004	2005	2006	2007	2008	2009	2010	2011
3003.790	3004.590	3005.390	3006.190	3007.790	3008.590	3009.390	3010.190	3010.990	3011.790

1. GLOSSARY
I = INVERT ELEVATION
C = CRITICAL DEPTH
W = WATER SURFACE ELEVATION
S = SUPER-ELEVATION
H = HEIGHT OF CHANNEL
E = ENERGY GRADE LINE
X = CURVES CROSSING OVER

SM3. OUT

B = BRIDGE ENTRANCE OR EXIT
Y = WALL ENTRANCE OR EXIT

2. STATIONS FOR POINTS AT A JUMP MAY NOT BE PLOTTED EXACTLY

APPENDIX H

Updated Drainage Exhibits