

GENERAL NOTES:

GENERAL

1. THE CONTRACTOR SHALL VERIFY ALL DIMENSIONS BEFORE BEGINNING CONSTRUCTION. THE ARCHITECT OR ENGINEER SHALL BE NOTIFIED OF ANY DISCREPANCIES OR INCONSISTENCIES.
2. DO NOT SCALE THE DRAWINGS.
3. SPECIAL NOTES AND DETAILS ON THE DRAWINGS SHALL TAKE PRECEDENCE OVER GENERAL NOTES AND TYPICAL DETAILS.
4. ALL WORK SHALL CONFORM TO THE MINIMUM STANDARDS OF THE FOLLOWING CODES:

THE 1994 EDITION OF THE UNIFORM BUILDING CODE, AND OTHER CODES AND ORDINANCES OF OTHER REGULATING AGENCIES WHICH HAVE AUTHORITY OVER ANY PORTION OF THE WORK, AND THOSE CODES AND STANDARDS LISTED IN THESE NOTES.

5. SEE THE PROJECT ARCHITECTURAL DRAWINGS FOR THE FOLLOWING:

- a. SIZES AND LOCATIONS OF DOOR AND WINDOW OPENINGS.
- b. SIZES AND LOCATIONS OF INTERIOR AND EXTERIOR NON-BEARING PARTITIONS.
- c. SIZES AND LOCATIONS OF CONCRETE CURBS, FLOOR DRAINS, SLOPES, DEPRESSED AREAS, CHANGES IN LEVEL, CHAMFERS, GROOVES, INSERTS, ETC.,
- d. SIZES AND LOCATIONS OF FLOOR AND ROOF OPENINGS,
- e. FLOOR AND ROOF FINISHES,
- f. DIMENSIONS NOT SHOWN ON THE STRUCTURAL DRAWINGS, AND
- g. CEILING STRUCTURE, EXCEPT AS SHOWN.

6. SEE PROJECT MECHANICAL, PLUMBING, AND ELECTRICAL DRAWINGS FOR THE FOLLOWING:

- a. PIPES, SLEEVES, HANGERS, TRENCHES, WALL FLOOR AND ROOF OPENINGS, DUCT PENETRATION ETC., EXCEPT AS SHOWN OR NOTED,
- b. ELECTRICAL CONDUIT RUNS, BOXES, OUTLETS IN WALLS AND SLABS,
- c. CONCRETE INSERTS FOR ELECTRICAL, MECHANICAL OR PLUMBING FIXTURES,
- d. SIZE AND LOCATION OF MACHINE OR EQUIPMENT BASES, AND ANCHOR BOLTS FOR MOUNTS.

7. THE CONTRACT STRUCTURAL DRAWINGS REPRESENT THE FINISHED STRUCTURE. THEY DO NOT SHOW OR INTEND TO SHOW THE METHOD OF CONSTRUCTION.

8. THE CONTRACTOR SHALL PROVIDE ALL MEASURES NEEDED TO PROTECT THE STRUCTURE DURING CONSTRUCTION. SUCH MEASURES SHALL INCLUDE, BUT ARE NOT LIMITED TO, BRACING AND SHORING FOR LOADS DUE TO HYDROSTATIC, EARTH, WIND OR SEISMIC, CONSTRUCTION EQUIPMENT, ETC. OBSERVATION VISITS TO THE SITE BY THE STRUCTURAL ENGINEER SHALL NOT INCLUDE INSPECTION OF THE ABOVE ITEMS.

9. NOTIFY THE STRUCTURAL ENGINEER WHEN DRAWINGS BY OTHERS SHOW OPENINGS, POCKETS, ETC., NOT SHOWN ON THE STRUCTURAL DRAWINGS, BUT WHICH AFFECT THE STRUCTURAL MEMBERS.

10. CONTRACTOR SHALL INVESTIGATE THE SITE DURING CLEARING AND EARTH WORK OPERATIONS FOR FILLED EXCAVATIONS OR BURIED STRUCTURES SUCH AS CESSPOOLS, CISTERNS, FOUNDATIONS, UTILITIES, ETC. IF ANY SUCH STRUCTURES ARE FOUND, THE STRUCTURAL ENGINEER SHALL BE NOTIFIED IMMEDIATELY.

11. CONSTRUCTION MATERIALS SHALL BE SPREAD OUT WHEN PLACED ON FRAMED FLOORS OR ROOFS. THE CONSTRUCTION MATERIAL LOAD SHALL NOT EXCEED THE DESIGN LIVE LOAD PER SQUARE FOOT OF THAT AREA. PROVIDE ADEQUATE SHORING AND/OR BRACING WHERE THE STRUCTURE HAS NOT ATTAINED ITS DESIGN STRENGTH.

12. SHOP DRAWINGS WHICH ARE SUBMITTED TO THE STRUCTURAL ENGINEER FOR REVIEW SHALL CONSIST OF 1 BLUELINE SET AND 1 SEPIA SET. THE SEPIA SET WILL BE RETURNED TO THE ARCHITECT FOR DISTRIBUTION.

13. DESIGN LIVE LOADS:

ROOF 16 PSF (RISE < 6:12)
SEISMIC ZONE 2B
BASIC WIND SPEED 75 MPH, EXPOSURE "B"

FOUNDATION

1. FOUNDATION DESIGN HAS BEEN DESIGNED BASED ON THE GEOTECHNICAL REPORT WHICH WAS PREPARED BY:

GEOSOILS INC.-NEVADA
PROJECT NO. 899-LV
DATED, JUNE 18, 1997

2. FOOTINGS HAVE BEEN DESIGNED BASED UPON AN ALLOWABLE SOIL PRESSURE OF 2000 PSF.

3. CONTRACTOR SHALL PROVIDE FOR PROPER DEWATERING OF EXCAVATIONS WHICH FILL UP DUE TO SURFACE WATER, GROUND WATER, SEEPAGE, ETC.

4. CONTRACTOR SHALL PROVIDE FOR THE DESIGN AND THE INSTALLATION OF ALL CRIBBING, SHEATHING AND SHORING REQUIRED TO SAFELY AND ADEQUATELY RETAIN THE EARTH BANKS AND PROTECT ANY ADJACENT EXISTING STRUCTURES.

5. EXCAVATIONS FOR FOOTINGS SHALL BE APPROVED BY THE GEOTECHNICAL ENGINEER PRIOR TO PLACING THE CONCRETE AND REINFORCING. THE CONTRACTOR SHALL NOTIFY THE GEOTECHNICAL ENGINEER WHEN THE EXCAVATIONS ARE READY FOR INSPECTION. THE GEOTECHNICAL ENGINEER SHALL SUBMIT A LETTER OF COMPLIANCE TO THE OWNER.

6. ALL EXCAVATIONS SHALL BE PROPERLY BACKFILLED. DO NOT PLACE BACKFILL BEHIND RETAINING WALLS UNTILL CONCRETE OR MASONRY HAS ATTAINED ITS FULL DESIGN STRENGTH. CONTRACTOR SHALL BRACE OR PROTECT ALL BUILDING AND PIT WALLS BELOW GRADE FROM LATERAL LOADS UNTIL ATTACHING FLOORS ARE COMPLETELY IN PLACE AND HAVE ATTAINED THEIR FULL DESIGN STRENGTH. CONTRACTOR SHALL PROVIDE FOR DESIGN, PERMITS AND INSTALLATION OF SUCH BRACING AND PROTECTION.

7. FOOTINGS SHALL BE PLACED AND ESTIMATED BASED UPON DEPTHS SHOWN ON THE DRAWINGS. SHOULD SOIL ENCOUNTERED AT THESE DEPTHS NOT BE APPROVED BY THE GEOTECHNICAL ENGINEER, FOOTING ELEVATIONS OR FOOTING DESIGNS WILL BE ALTERED BY CHANGE ORDER.

8. FOOTING BACKFILL AND UTILITY TRENCH BACKFILL WITHIN THE BUILDING PERIMETER SHALL BE MECHANICALLY COMPACTED IN LAYERS, TO THE APPROVAL OF THE GEOTECHNICAL ENGINEER. FLOODING WILL NOT BE PERMITTED.

9. ALL ABANDONED FOOTINGS, UTILITIES, ETC., THAT INTERFERE WITH THE NEW CONSTRUCTION SHALL BE REMOVED.

CONCRETE

1. ALL WORK PERTAINING TO CONCRETE CONSTRUCTION SHALL CONFORM TO THE 'BUILDING CODE REQUIREMENTS FOR REINFORCED CONCRETE', ACI 318, AND THE 'SPECIFICATIONS FOR STRUCTURAL CONCRETE FOR BUILDINGS', ACI 301, LATEST EDITIONS, WITH MODIFICATIONS AS NOTED ON THE DESIGN DRAWINGS.

2. REINFORCED CONCRETE DESIGN IS BASED UPON THE ULTIMATE STRENGTH DESIGN METHOD.

3. CONCRETE MIXES SHALL BE DESIGNED BY A QUALIFIED TESTING LABORATORY AND SHALL BEAR THE WET SEAL OF A CIVIL ENGINEER LICENSED IN THE STATE OF NEVADA. THEN SHALL BE REVIEWED BY THE STRUCTURAL ENGINEER.

4. SCHEDULE OF STRUCTURAL CONCRETE 28-DAY STRENGTHS & TYPES:

LOCATIONS IN STRUCTURE	STRENGTH, PSI	TYPE
FOOTINGS	3000	HARD ROCK
SLAB ON GRADE	3000	HARD ROCK

5. PORTLAND CEMENT SHALL CONFORM TO THE REQUIREMENTS OF ASTM C-150, TYPE V IF IN CONTACT WITH SOIL, AND TYPE II ELSEWHERE.

6. CONCRETE MIXES MAY CONTAIN FLY ASH. THE FLY ASH SHALL CONFORM TO ASTM C-618 CLASS F AND THE IGNITION LOSS SHALL BE LIMITED TO 2%. THE ADDITION RATE SHALL NOT EXCEED 15% OF THE CEMENT WEIGHT. THE CONTRACTOR SHALL SUBMIT ALL CERTIFICATES SHOWING THAT FLY ASH CONFORMS TO THE ABOVE CRITERIA.

7. AGGREGATE FOR HARD ROCK CONCRETE SHALL CONFORM TO ALL REQUIREMENTS AND TESTS OF ASTM C-33 MAY BE USED ONLY WITH PERMISSION OF THE STRUCTURAL ENGINEER.

8. DRY PACK OR GROUT UNDER BASEPLATES, SILL PLATES, ETC.

9. CONCRETE MIXING OPERATIONS, ETC., SHALL CONFORM TO ASTM C-94.

10. PLACEMENT OF CONCRETE SHALL CONFORM TO ACI STANDARD 304. SANDBLAST ALL CONCRETE SURFACES AGAINST WHICH CONCRETE IS TO BE PLACED.

11. CLEAR COVERAGE OF CONCRETE OVER REINFORCING BARS SHALL BE AS FOLLOWS:

CONCRETE POURED DIRECTLY AGAINST EARTH, 3" CLR. TO REINFORCING.
SLABS ON GRADE, 1" CLR. TO TOP OF SLAB FOR 4" SLABS
SLABS ON GRADE, 2" CLR. TO TOP OF SLAB FOR 5" SLABS

12. ALL REINFORCING BARS, ANCHOR BOLTS AND OTHER CONCRETE INSERTS SHALL BE WELL SECURED IN PLACE PRIOR TO PLACING CONCRETE.

13. MECHANICAL PIPES AND ELECTRICAL CONDUITS WHICH PASS THROUGH SLAB ON GRADE WILL NOT REQUIRE SLEEVES. IF SLEEVES ARE REQUIRED, INSTALL SLEEVES BEFORE PLACING CONCRETE. DO NOT CUT ANY REINFORCING WHICH MAY INTERFERE WITH SLEEVE PLACEMENT. CORING OPENINGS IN CONCRETE IS NOT PERMITTED. NOTIFY THE STRUCTURAL ENGINEER IN ADVANCE OF CONDITIONS NOT SHOWN ON THE STRUCTURAL DRAWINGS. NO PIPES OR ELECTRICAL CONDUIT SHALL PASS THROUGH CONCRETE BEAMS OR COLUMNS, UNLESS SPECIFICALLY DETAILED.

14. FOR SLABS ON GRADE, UNLESS OTHERWISE DETAILED, NO PIPES OR CONDUITS SHALL BE PLACED WITHIN THE INDICATED CONCRETE SLAB THICKNESS AND SHALL BE LOCATED BELOW THE SLAB.

15. CURING COMPOUNDS USED OVER CONCRETE THAT IS TO RECEIVE A RESILIENT TILE FINISH SHALL BE APPROVED BY THE FINISH APPLICATOR BEFORE USE.

16. MODULUS OF ELASTICITY OF CONCRETE, WHEN TESTED IN ACCORDANCE WITH ASTM C-469, SHALL BE AT LEAST THE VALUE GIVEN BY THE EQUATIONS IN SECTION 8.5.1 OF ACI 318 FOR THE SPECIFIED CONCRETE 28-DAY STRENGTH.

MASONRY

1. ALL MASONRY UNITS SHALL CONFORM TO ASTM C90 GRADE N-1 AND SHALL HAVE A MINIMUM 28 DAY COMPRESSIVE STRENGTH ON THE NET AREA OF 2000 PSI FOR A DESIGN STRENGTH OF $f_m = 1500$ PSI. UNLESS NOTED OTHERWISE.

2. CEMENT SHALL BE THE SAME AS THAT SPECIFIED FOR CONCRETE.

3. REINFORCING BARS - SEE NOTES UNDER 'REINFORCING STEEL' FOR REQUIREMENTS. ALL REINFORCING SHALL MEET ASTM A615, GRADE 60, UNLESS NOTED OTHERWISE.

4. MORTAR MIX SHALL CONFORM TO REQUIREMENTS OF UBC TABLE 21-A, TYPE S. MORTAR SHALL ATTAIN A MINIMUM COMPRESSIVE STRENGTH OF 1800 PSI AT 28 DAYS. UNLESS NOTED OTHERWISE.

5. GROUT SHALL CONFORM TO THE REQUIREMENTS OF SECTION 2103.4 OF THE UBC FOR COARSE GROUT. USE SUFFICIENT WATER FOR GROUT TO FLOW INTO ALL CELLS OF THE MASONRY WITHOUT SEGREGATION. GROUT SHALL ATTAIN A MINIMUM COMPRESSIVE STRENGTH OF 2000 PSI AT 28 DAYS, UNLESS NOTED OTHERWISE. FLY ASH WILL NOT BE PERMITTED IN GROUT.

6. GROUT AND MORTAR MIXES SHALL BE DESIGNED BY A QUALIFIED TESTING LABORATORY AND SHALL BEAR THE WET SEAL OF A CIVIL ENGINEER LICENSED IN THE STATE OF NEVADA. THE MIXES WILL BE REVIEWED BY THE STRUCTURAL ENGINEER.

7. PROVIDE A MINIMUM OF 1/2" OF GROUT BETWEEN MAIN REINFORCING AND MASONRY UNITS.

8. FOR GROUT LIFT CONSTRUCTION, REFER TO UBC SECTION 2104.6.

9. ALL CELLS IN CONCRETE BLOCKS SHALL BE FILLED SOLID WITH GROUT, UNLESS NOTED OTHERWISE IN THE DRAWINGS.

10. CELLS SHALL BE IN VERTICAL ALIGNMENT. DOWELS FROM FOOTINGS SHALL BE SET TO ALIGN WITH CORES CONTAINING REINFORCING STEEL.

11. REFER TO ARCHITECTURAL DRAWINGS FOR SURFACE AND HEIGHT OF MASONRY UNITS, LAYING PATTERN, AND JOINT TYPE.

12. ALL VERTICAL REINFORCING SHALL HAVE A MINIMUM LAP SPLICE OF 48-BAR DIAMETERS. WHEN TWO BARS OCCUR IN ONE CELL, THE SPLICE SHALL BE INCREASED TO 62-BAR DIAMETERS. ALL HORIZONTAL REINFORCING SHALL HAVE A MINIMUM LAP SPLICE OF 48-BAR DIAMETERS, STAGGER ALL HORIZONTAL REBAR SPLICES.

13. MECHANICAL PIPES AND ELECTRICAL CONDUITS WHICH PASS THROUGH MASONRY WALLS DO NOT REQUIRE SLEEVES. MECHANICAL OR ELECTRICAL DRAWINGS. IF SLEEVES ARE REQUIRED, INSTALL SLEEVES BEFORE GROUTING. DO NOT CUT ANY REINFORCING WHICH MAY INTERFERE WITH SLEEVE PLACEMENT. CORING OPENINGS IN GROUTED MASONRY IS NOT PERMITTED. NOTIFY THE STRUCTURAL ENGINEER IN ADVANCE OF CONDITIONS NOT SHOWN ON THE STRUCTURAL DRAWINGS. NO PIPES OR ELECTRICAL CONDUIT SHALL PASS THROUGH MASONRY LINTELS, UNLESS SPECIFICALLY DETAILED.

REINFORCING STEEL (FOR CONCRETE AND MASONRY)

1. ALL REINFORCING STEEL SHALL BE DETAILED AND PLACED SO AS TO CONFORM TO THE 'BUILDING CODE REQUIREMENTS FOR REINFORCED CONCRETE' (ACI 318 LATEST APPROVED EDITION), AND THE 'MANUAL OF STANDARD PRACTICE FOR REINFORCED CONCRETE CONSTRUCTION' (LATEST APPROVED EDITION) BY THE C.R.S.I. AND THE W.C.R.S.I., AS MODIFIED BY THE PROJECT DRAWINGS.

2. DEFORMED REINFORCING BARS SHALL CONFORM TO THE REQUIREMENTS OF ASTM A-615 GRADE 60, EXCEPT FOR TIES, STIRRUPS AND REINFORCING BARS IN NON-STRUCTURAL CONCRETE SUCH AS SLABS ON GRADE, WHICH MAY BE GRADE 40, UNLESS NOTED OTHERWISE.

3. WELDING OF REINFORCING SHALL BE WITH LOW HYDROGEN ELECTRODES IN CONFORMANCE WITH 'RECOMMENDED PRACTICES FOR WELDING REINFORCING STEEL, ETC.', AMERICAN WELDING SOCIETY, AWS-D1.4.

4. ALL REINFORCING BAR BENDS SHALL BE MADE COLD.

5. WELDED WIRE FABRIC SHALL CONFORM TO ASTM A185.

6. MINIMUM LAP OF WELDED WIRE FABRIC SHALL BE 6" OR ONE FULL MESH AND ONE HALF, WHICH EVER IS GREATER.

7. REINFORCING SPLICES SHALL BE MADE ONLY WHERE INDICATED ON THE DRAWINGS.

8. DOWELS BETWEEN FOOTINGS AND WALLS OR COLUMNS SHALL BE THE SAME GRADE, SIZE, SPACING AND NUMBER AS THE SPECIFIED, VERTICAL REINFORCING, U.N.O..

9. ALL BARS SHALL BE MARKED SO IDENTIFICATION CAN BE MADE WHEN THE FINAL IN-PLACE INSPECTION OCCURS.

STRUCTURAL STEEL

1. STRUCTURAL STEEL SHALL BE DESIGNED, DETAILED, FABRICATED AND ERECTED IN COMPLIANCE WITH THE AISC "SPECIFICATION FOR STRUCTURAL STEEL BUILDINGS" AND "CODE OF STANDARD PRACTICE FOR STEEL BUILDINGS AND BRIDGES" (LATEST EDITION AND SUPPLEMENTS).

- a. STEEL JOISTS SHALL CONFORM TO THE CURRENT EDITION OF THE STANDARD SPECIFICATIONS FOR STEEL JOISTS (K SERIES), LONG SPAN STEEL JOISTS (LH SERIES), OR DEEP LONG SPAN STEEL JOISTS (DLH SERIES), AS ADOPTED BY THE STEEL JOIST INSTITUTE AND THE AISC.

- b. STEEL JOIST FABRICATOR SHALL SUBMIT SHOP DRAWINGS AND CALCULATIONS SEALED BY A CIVIL ENGINEER LICENSED IN THE STATE OF NEVADA. JOIST DESIGNS AND DRAWINGS SHALL INCLUDE BRIDGING AND OTHER MISCELLANEOUS ITEMS AS REQUIRED.

- c. STEEL JOIST DESIGN CRITERIA:

2. STRUCTURAL STEEL SHALL CONFORM TO THE ASTM DESIGNATION A-572 Fy=50 ksi UNLESS NOTED OTHERWISE.

3. BOLTS SHALL CONFORM TO ASTM A325-N, EXCEPT ANCHOR BOLTS WHICH SHALL CONFORM TO ASTM A307, GRADE "A", UNLESS NOTED OTHERWISE.

4. THE STRUCTURAL STEEL FABRICATOR SHALL FURNISH SHOP DRAWINGS OF ALL STRUCTURAL STEEL FOR ARCHITECT'S REVIEW BEFORE FABRICATION.

5. BOLT HOLES IN STEEL SHALL BE 1/16" LARGER THAN NOMINAL SIZE OF BOLT USED EXCEPT ANCHOR BOLT HOLES WHICH MAY BE 3/16" LARGER.

6. ALL STEEL BAR JOISTS WILL BE PAINTED TO MATCH THE STANDING SEAM METAL ROOF. ALL ANGLES AND EXPOSED STRUCTURAL STEEL WILL BE PAINTED TO MATCH STEEL DECK. SEE ARCHITECTURAL DRAWINGS FOR ANY ADDITIONAL REQUIREMENTS.

7. WELDED JOINTS SHALL CONFORM TO THE PREQUALIFIED JOINT DETAILS AS INDICATED IN THE STRUCTURAL WELDING CODE (AWS D1.1) BY THE AMERICAN WELDING SOCIETY.

8. WELD LENGTHS CALLED FOR ON PLANS ARE THE NET EFFECTIVE LENGTH REQUIRED. WELD SIZE SHALL BE AISC MINIMUM, UNLESS A LARGER SIZE IS NOTED.

STEEL DECK

1. THE STEEL DECKING SHALL BE OF THE TYPE AND GAUGE AS CALLED FOR ON THE DRAWINGS. DECKING AND ALL ACCESSORIES SHALL BE FORMED FROM STEEL SHEETS CONFORMING TO ASTM STANDARDS AS SHOWN BELOW. GALVANIZED DECK SHALL BE ZINC COATED PER ASTM A525, G80. ALL ROOF DECK SHALL BE GALVANIZED. THE DECK UNITS SHALL BE SHORED, U.N.O. STEEL ROOF DECK UNITS SHALL BE CONTINUOUS OVER TWO OR MORE SPANS, U.N.O.

GALVANIZED DECK ASTM A446 GRADE A
EXPOSED UNDERSIDE OF DECK TO BE
PAINTED AS SHOWN ON ARCH. DWGS.

2. MINIMUM BEARING OF THE STEEL DECK ON SUPPORTS SHALL BE 2". ALL 3" DEEP METAL DECK SHALL HAVE MINIMUM BEARING OF 3". SHEETS SHALL BE ATTACHED TO ALL SUPPORTING STEEL MEMBERS BY WELDING AS INDICATED ON DRAWINGS AND IN ACCORDANCE WITH MANUFACTURER'S RECOMMENDATIONS. UPON COMPLETION OF ERECTION, ALL WELDS ON STEEL DECK AREAS EXPOSED TO WEATHER SHALL BE DE-SLAGGED, CLEANED AND TOUCHED-UP WITH A ZINC RICH PRIMER.

3. THE STEEL DECK FABRICATOR SHALL FURNISH SHOP DRAWINGS OF ALL STEEL DECK, FOR ARCHITECT'S REVIEW BEFORE FABRICATION.

4. SEE ARCHITECTURAL, MECHANICAL, ELECTRICAL, ETC., DRAWINGS FOR SIZES AND LOCATIONS OF DECK OPENINGS AND FOR DECK OPENINGS NOT SHOWN ON THE STRUCTURAL DRAWINGS. SEE TYPICAL DETAILS FOR FRAMING REQUIREMENTS AT DECK OPENINGS. OPENINGS LARGER THAN 12" SHALL NOT BE PLACED IN DECK UNLESS SPECIFICALLY SHOWN ON THE STRUCTURAL DRAWINGS, NOTIFY THE STRUCTURAL ENGINEER.

SPECIAL INSPECTIONS REQUIREMENTS

1. SPECIAL INSPECTION SHALL BE PROVIDED FOR THE FOLLOWING TYPES OF WORK:

- a. CONCRETE WORK WITH STRENGTHS GREATER THAN 2500 PSI, EXCEPT FOR NONSTRUCTURAL CONCRETE SUCH AS SLABS ON GRADE.
- b. FIELD WELDING (EXCEPT STEEL STUDS, FURRING CHANNELS, ETC.).
- c. SHOP WELDING PROCEDURES AND MULTIPLE PASS WELDS.
- d. HIGH STRENGTH BOLTING.
- e. REBAR OR BOLTS ANCHORED WITH EPOXY'S OR GROUTS.
- f. ANCHOR BOLTS AND EMBEDDED PLATES.

Alternate No. 1

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DRAWING NO.

54

53

SHEET NO.

S1

SHEET TITLE:
PARK-GENERAL NOTES

DEPARTMENT OF PUBLIC WORKS

Torrey Pines Park
& Ball Field



ARCHITECTURAL SERVICES

DESIGN BY: JFJ

DRAWN BY: DUANE

REVISIONS:

DATE:

PLOTTED: 4/2/01

SCALE FACTOR: 1=1

TIME: 15:14

DATE: 07/30/97

PLOTTED BY: DUANE

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