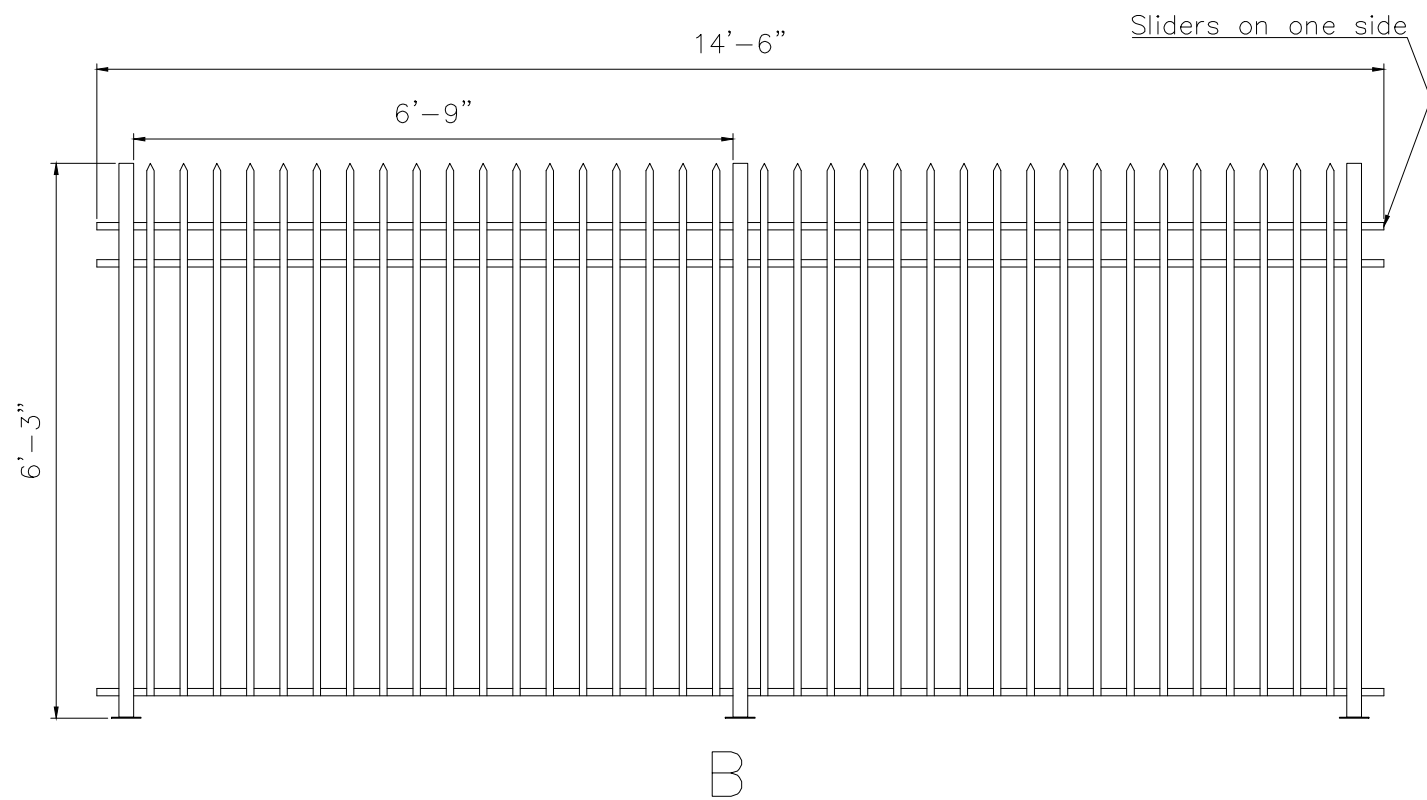
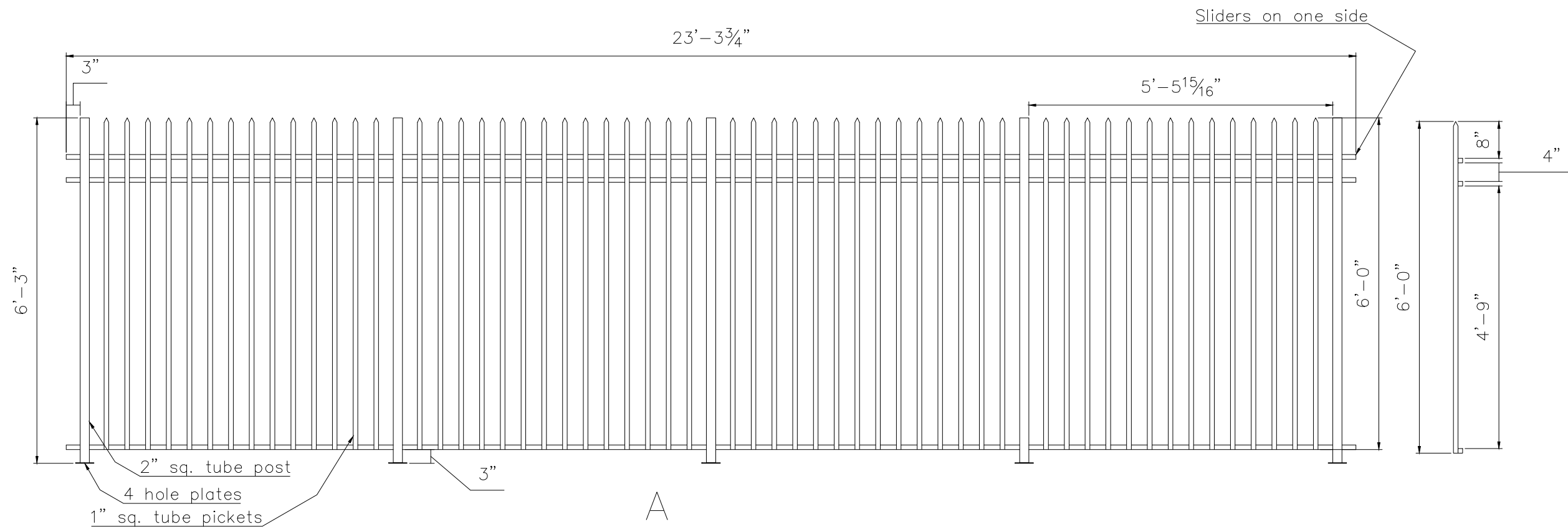
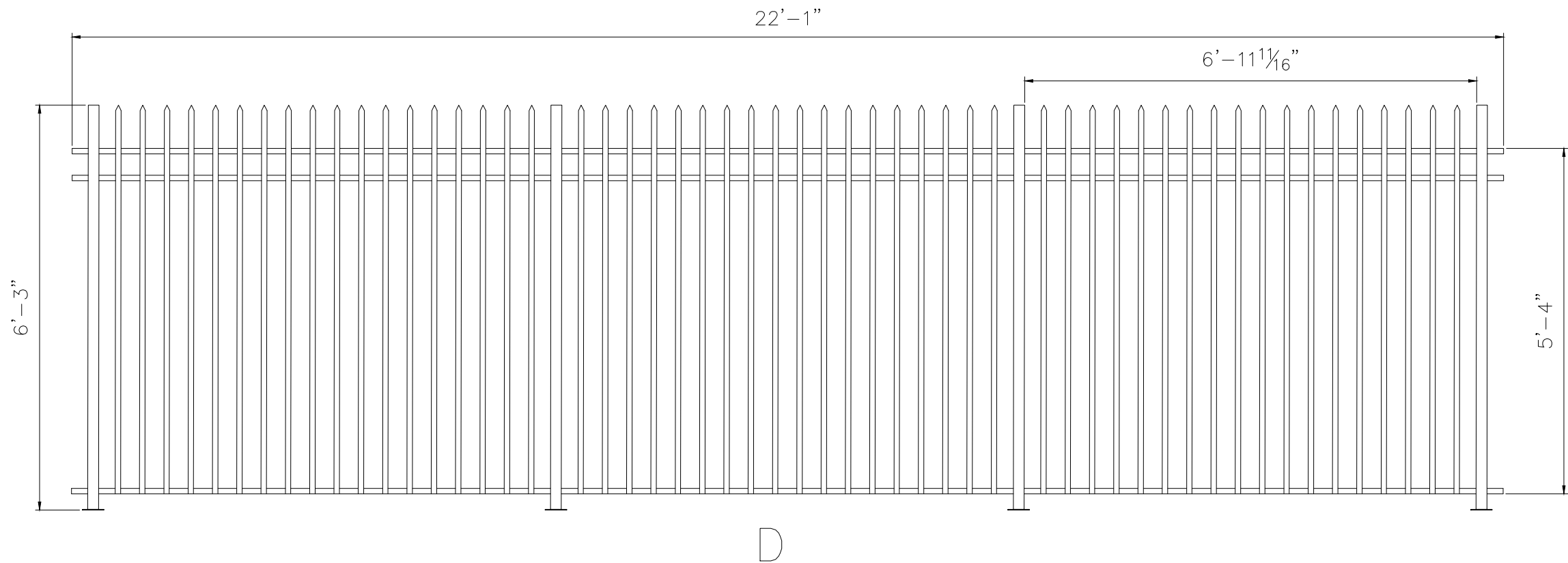
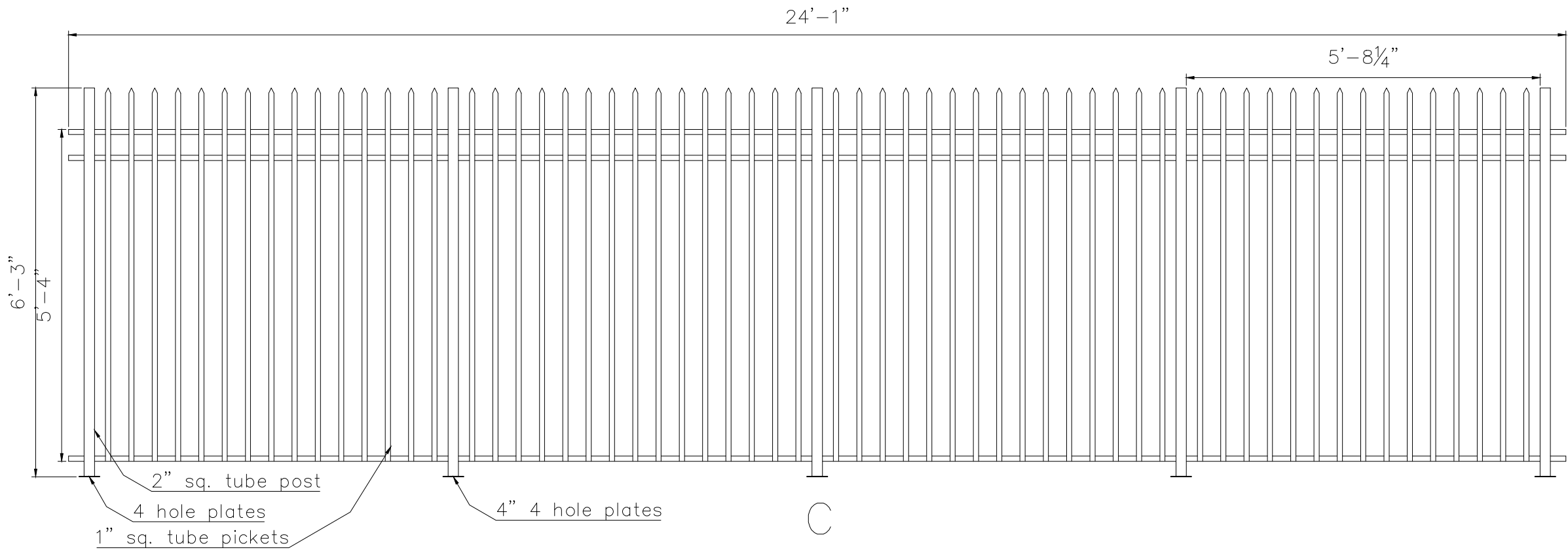




Whiting Turner
Westside School
Powder coat
8-11-15
3765

Cut list
8 x 75" 2x2 .120
12 x 65¹⁵/₁₆" 1x1 .065
6 x 81" 1x1 .065
92 x 72" 1x1 .065
6 x 3" ³/₄"x³/₄" sliders

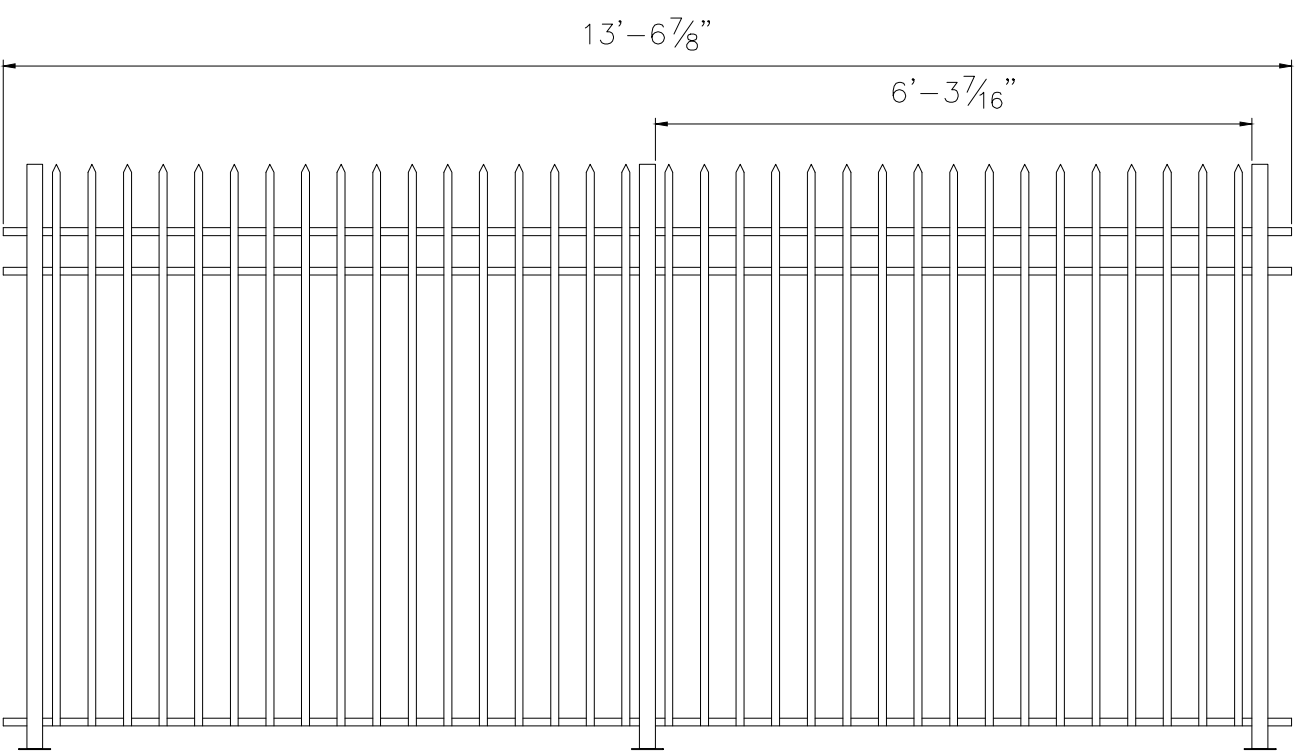
REV.	DATE	DESCRIPTION	NAME
A			
B			
C			
D			
P & L Fencing & Iron LLC. 2842 Marco St. Las Vegas, NV, 89115			
CONTRACTOR		Whiting Turner	DATE 10/15/15
			DRAWN JPH
PROJECT		Westside School	CHECKED PWH
			SCALE NTS
LOCATION		330 W. Washington	DWG #
			JOB #
DESCRIPTION		FENCE AS BUILTS	SHEET #
			3765
			A 1



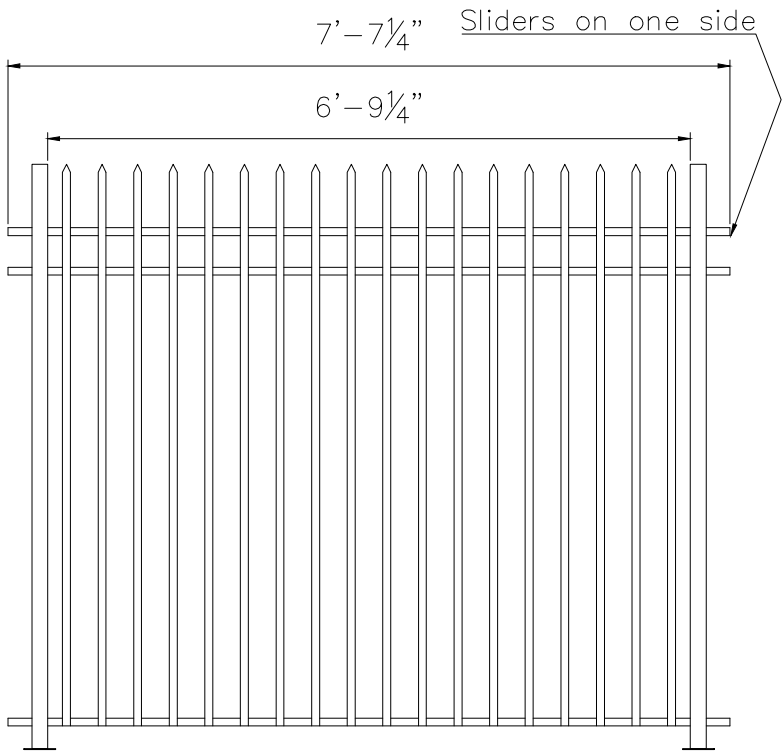
Whiting Turner
Westside School
Powder coat
8-11-15
3765

Cut list
9 x 75" 2x2 .120
12 x 68 1/4" 1x1 .065
9 x 83 1/16" 1x1 .065
114 x 72" 1x1 .065
6 x 3" 3/4"x3/4" sliders

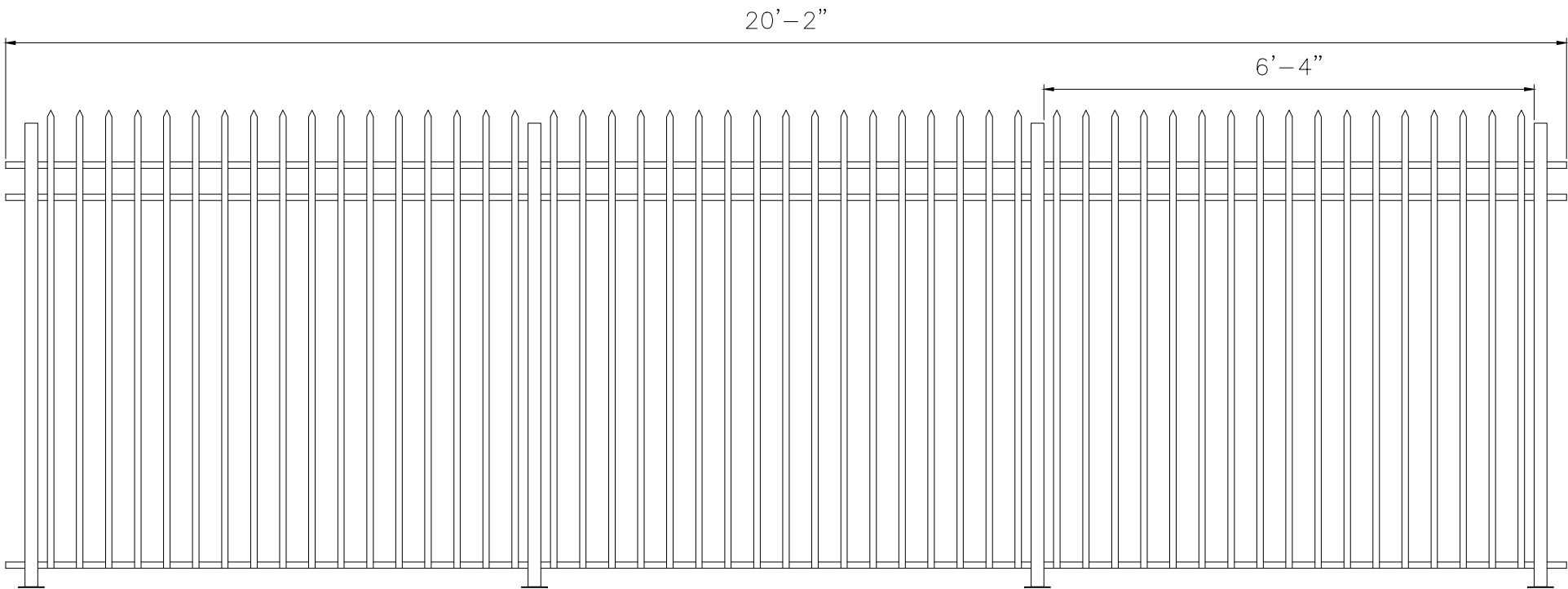
REV.	DATE	DESCRIPTION	NAME
A			
B			
C			
D			
P & L Fencing & Iron LLC. 2842 Marco St. Las Vegas, NV, 89115			
CONTRACTOR		Whiting Turner	DATE 10/15/15
			DRAWN JPH
PROJECT		Westside School	CHECKED PWH
			SCALE NTS
LOCATION		330 W. Washington	DWG #
			JOB #
DESCRIPTION		FENCE AS BUILTS	SHEET #
			A2



E



F



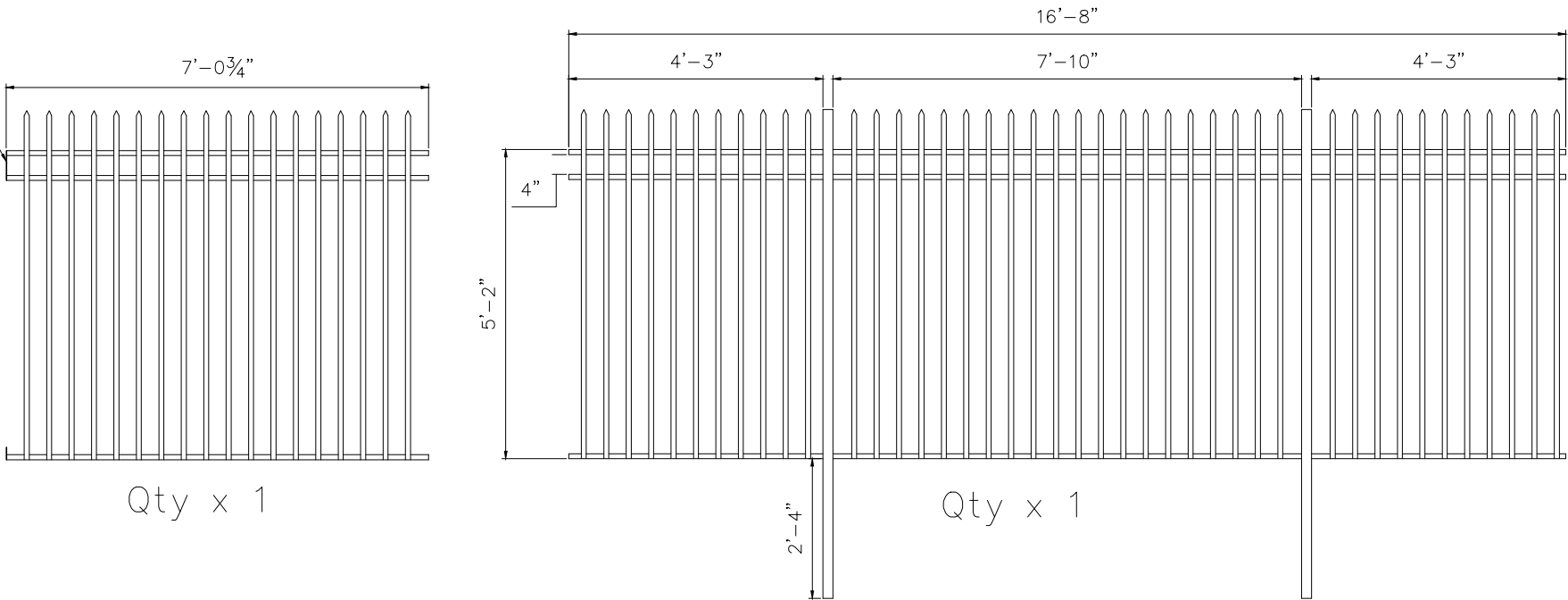
G

Whiting Turner
Westside School
Powder coat
8-11-15
3765

- Cut list
- 9 x 75" 2x2 .120
 - 6 x 75 7/16" 1x1 .065
 - 3 x 81 1/4" 1x1 .065
 - 9 x 76" 1x1 .065
 - 103 x 72" 1x1 .065
 - 6 x 3" 3/4"x3/4" sliders

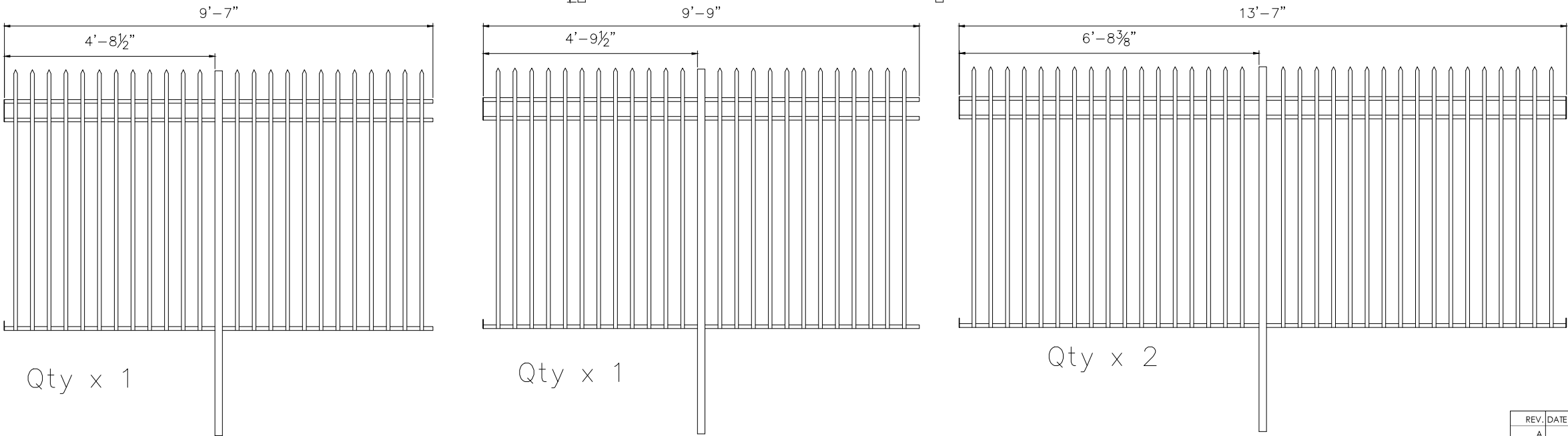
REV.	DATE	DESCRIPTION	NAME
A			
B			
C			
D			
P & L Fencing & Iron LLC. 2842 Marco St. Las Vegas, NV, 89115			
CONTRACTOR		Whiting Turner	DATE 10/15/15
			DRAWN JPH
PROJECT		Westside School	CHECKED PWH
			SCALE NTS
LOCATION		330 W. Washington	DWG #
			JOB #
DESCRIPTION		FENCE AS BUILTS	SHEET #
			3765
			A3

Tab one side
slider the other

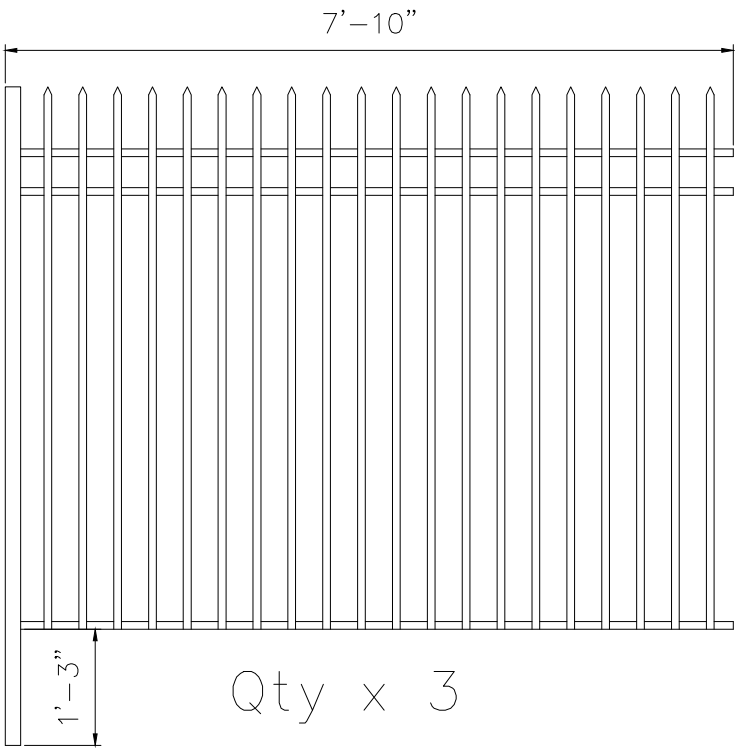
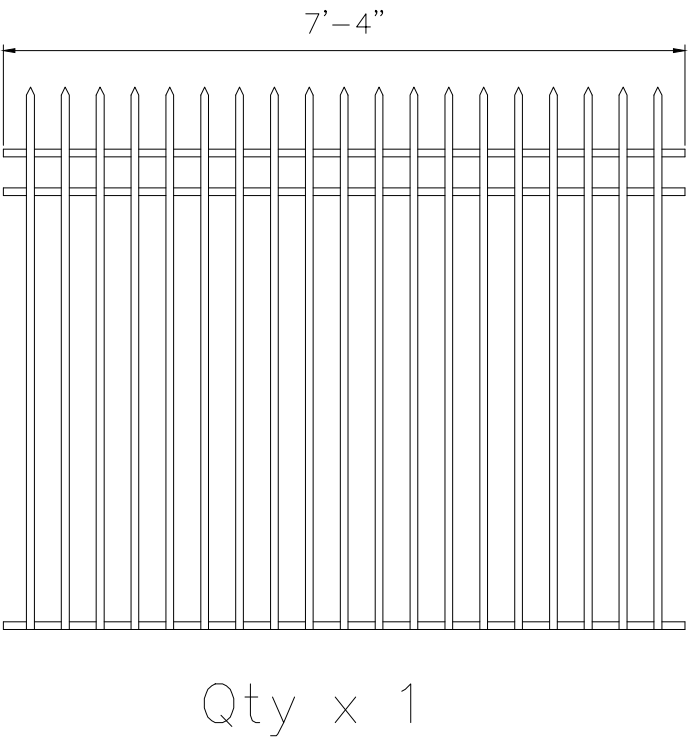
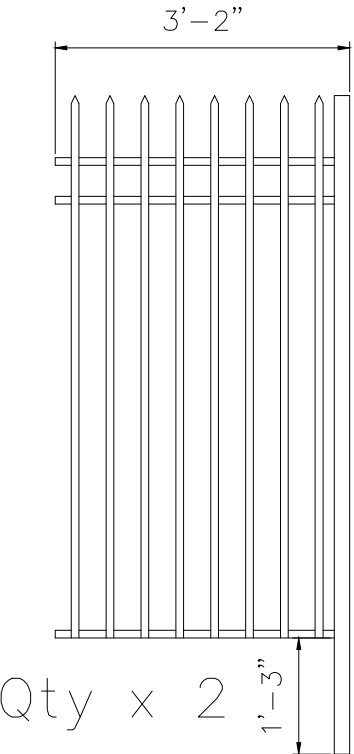
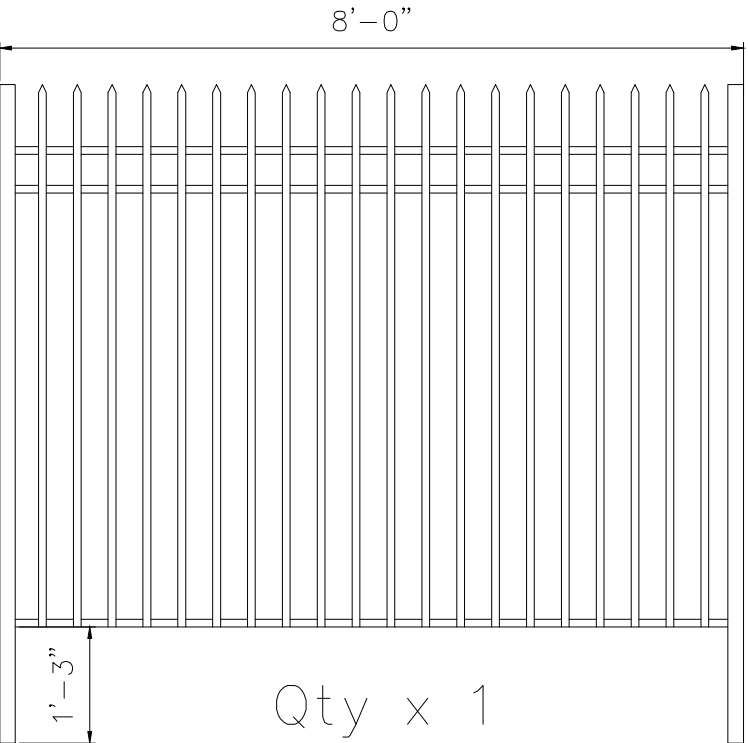
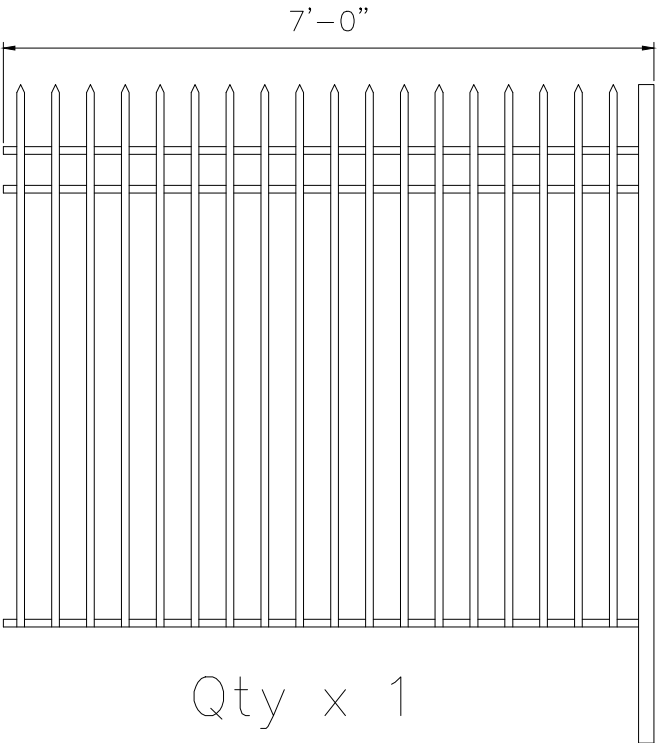


Whiting Turner
Westside School
Powder coat
10-5-15
3765

Cut list
6 x 85" 2x2 .120
3 x 84 3/4" 1x1 .095
3 x 94" 1x1 .095
6 x 51" 1x1 .095
6 x 56 1/2" 1x1 .095
6 x 57 1/2" 1x1 .095
6 x 80 3/8" 1x1 .095
142 x 70" 1x1 .065



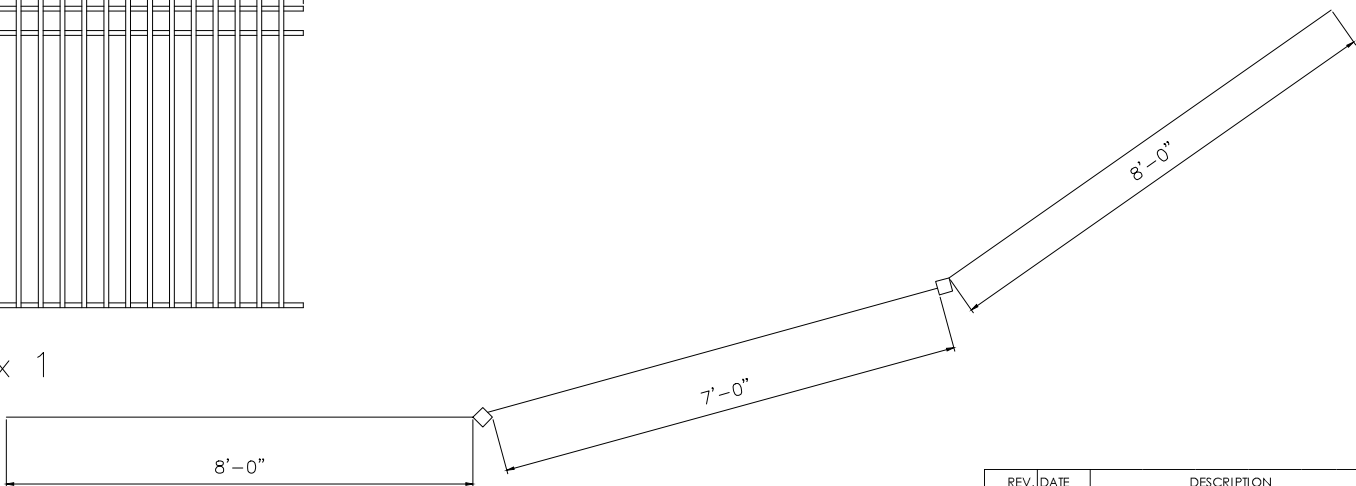
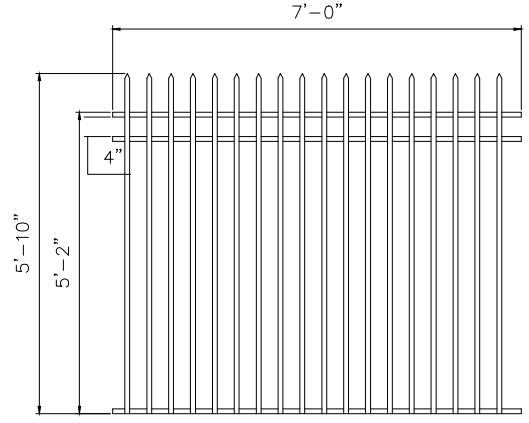
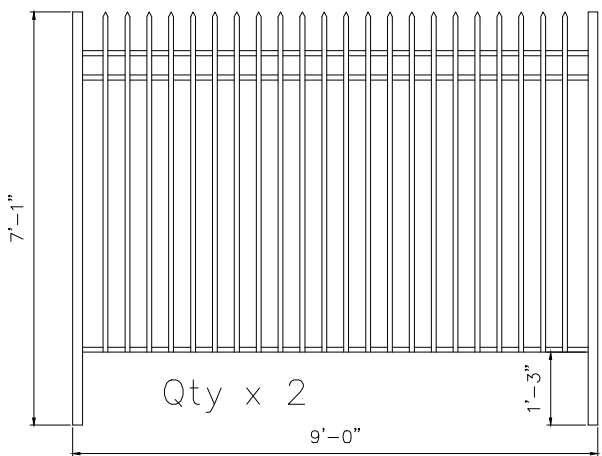
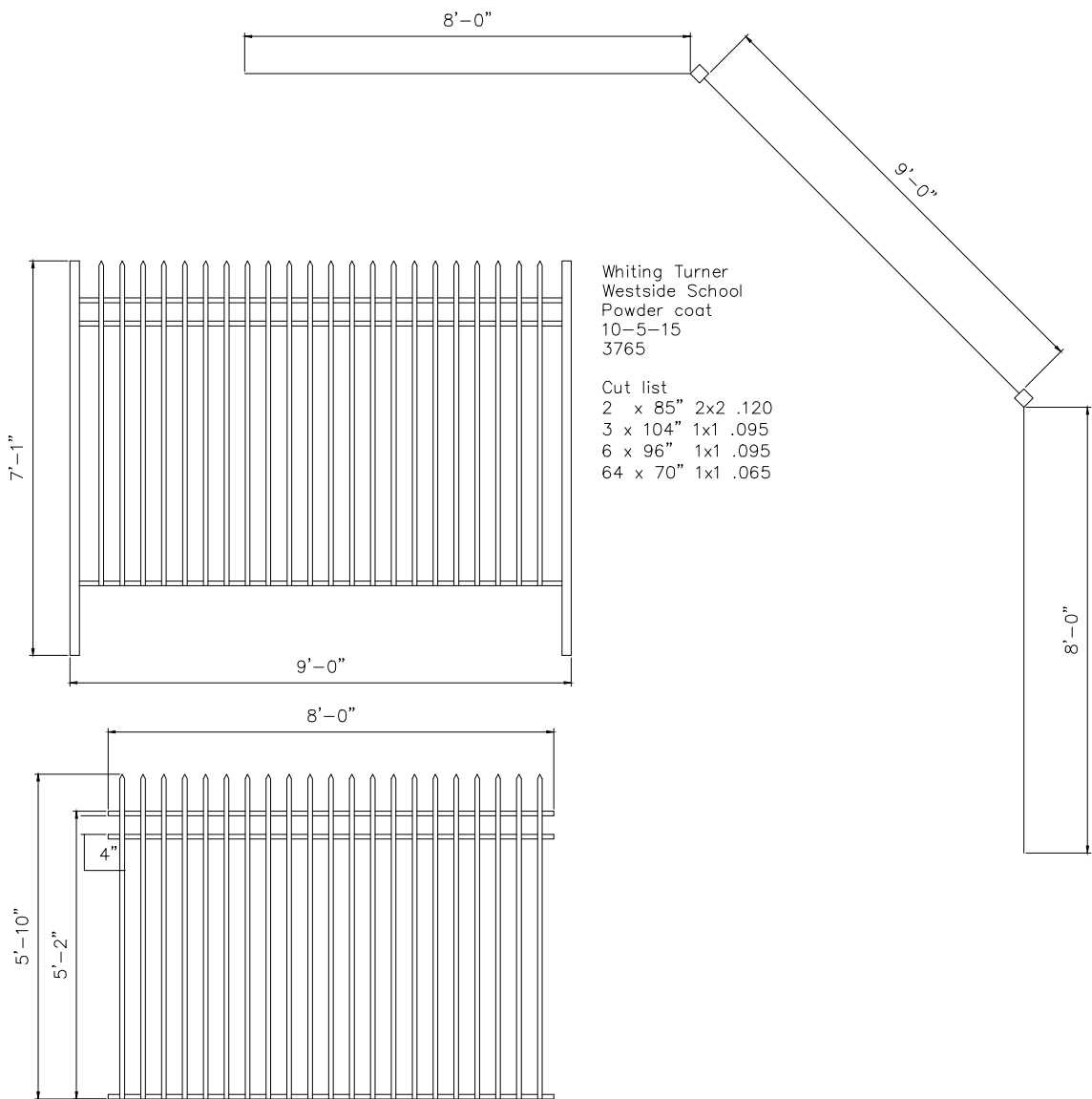
REV.	DATE	DESCRIPTION	NAME
A			
B			
C			
D			
P & L Fencing & Iron LLC. 2842 Marco St. Las Vegas, NV, 89115			
CONTRACTOR		Whiting Turner	DATE 10/15/15
			DRAWN JPH
PROJECT		Westside School	CHECKED PWH
			SCALE NTS
LOCATION		330 W. Washington	DWG #
			JOB #
DESCRIPTION		FENCE AS BUILTS	SHEET # A4
			3765



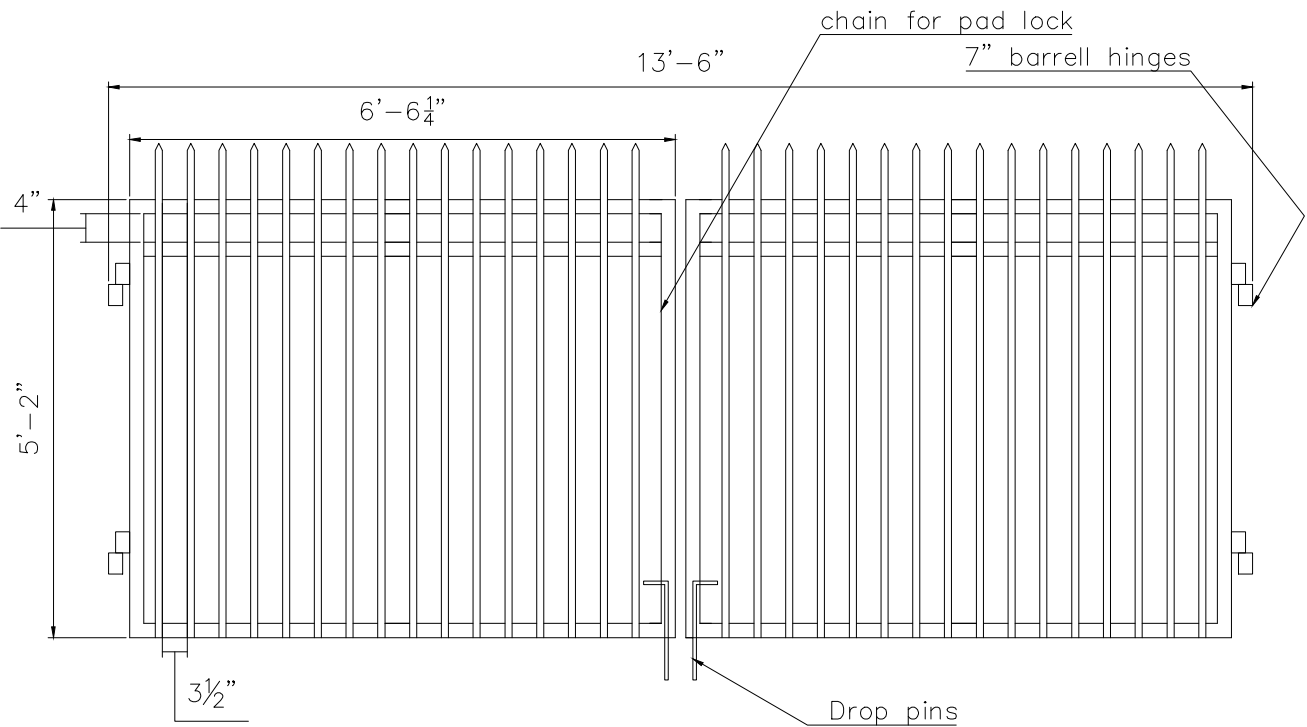
Whiting Turner
Westside School
Powder coat
12-18-15
3765

Cut list
8 x 85" 2x2 .120
3 x 82" 1x1 .095
12 x 92" 1x1 .095
6 x 36" 1x1 .095
3 x 88" 1x1 .095
132 x 70" 1x1 .065

REV.	DATE	DESCRIPTION	NAME
A			
B			
C			
D			
P & L Fencing & Iron LLC. 2842 Marco St. Las Vegas, NV, 89115			
CONTRACTOR		Whiting Turner	DATE 10/15/15
			DRAWN JPH
PROJECT		Westside School	CHECKED PWH
			SCALE NTS
LOCATION		330 W. Washington	DWG #
			JOB #
DESCRIPTION		FENCE AS BUILTS	SHEET #
			3765 A6



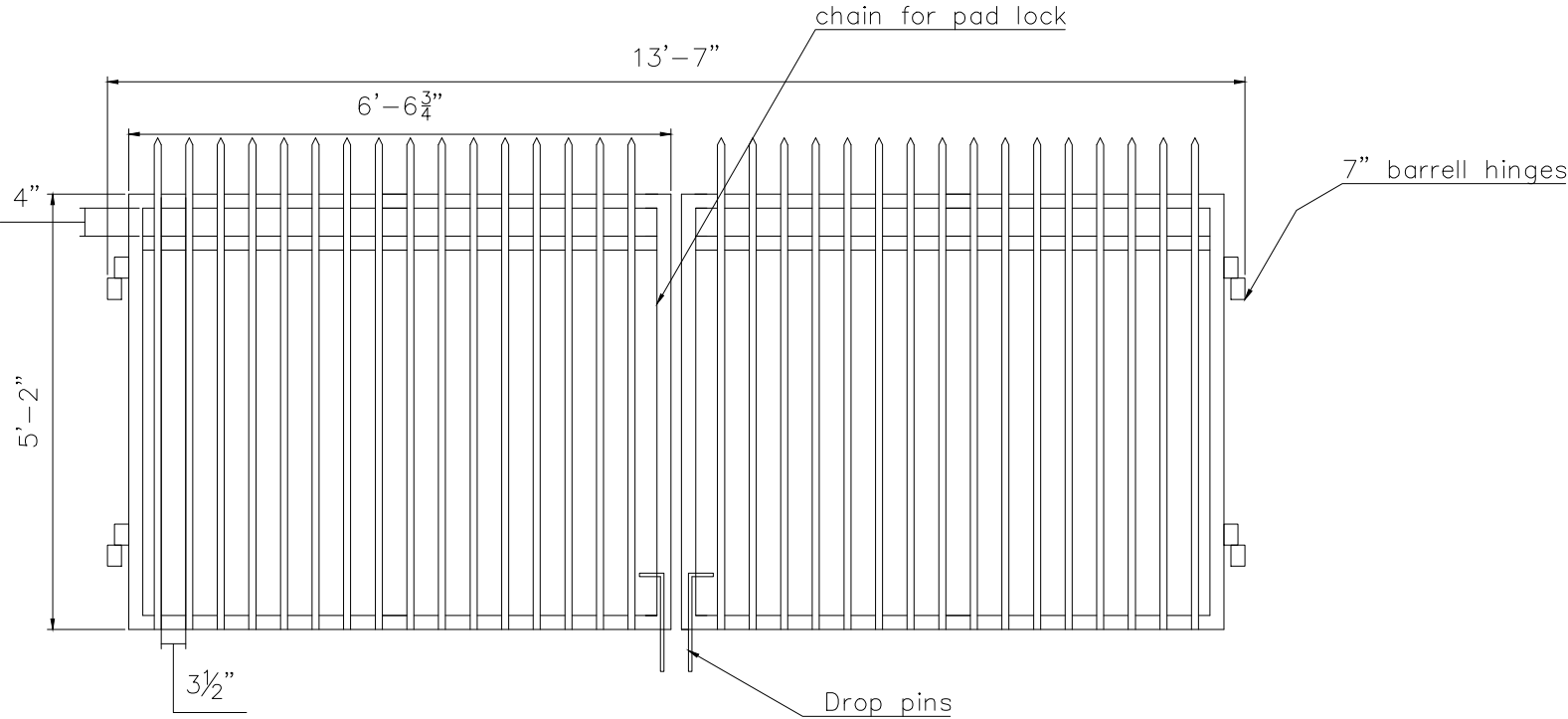
REV.	DATE	DESCRIPTION	NAME
A			
B			
C			
D			
P & L Fencing & Iron LLC. 2842 Marco St. Las Vegas, NV, 89115			
CONTRACTOR		Whiting Turner	DATE 10/15/15
		DRAWN	JPH
PROJECT		Westside School	CHECKED PWH
		SCALE	NTS
LOCATION		330 W. Washington	DWG #
		JOB #	SHEET #
DESCRIPTION		FENCE AS BUILTS	3765 A5



west side Gate entry

Whiting Turner
Westside School
Powder coat
1-25-15
3765

Cut list
4 x 62" 2 x 2 x .095 w/ 45 1 side
2 x 78 1/4" 2 x 2 x .095 w/ 45 2 sides
4 x 74 1/4" 2 x 2 x .095
32 x 70" 1 x 1 x .065

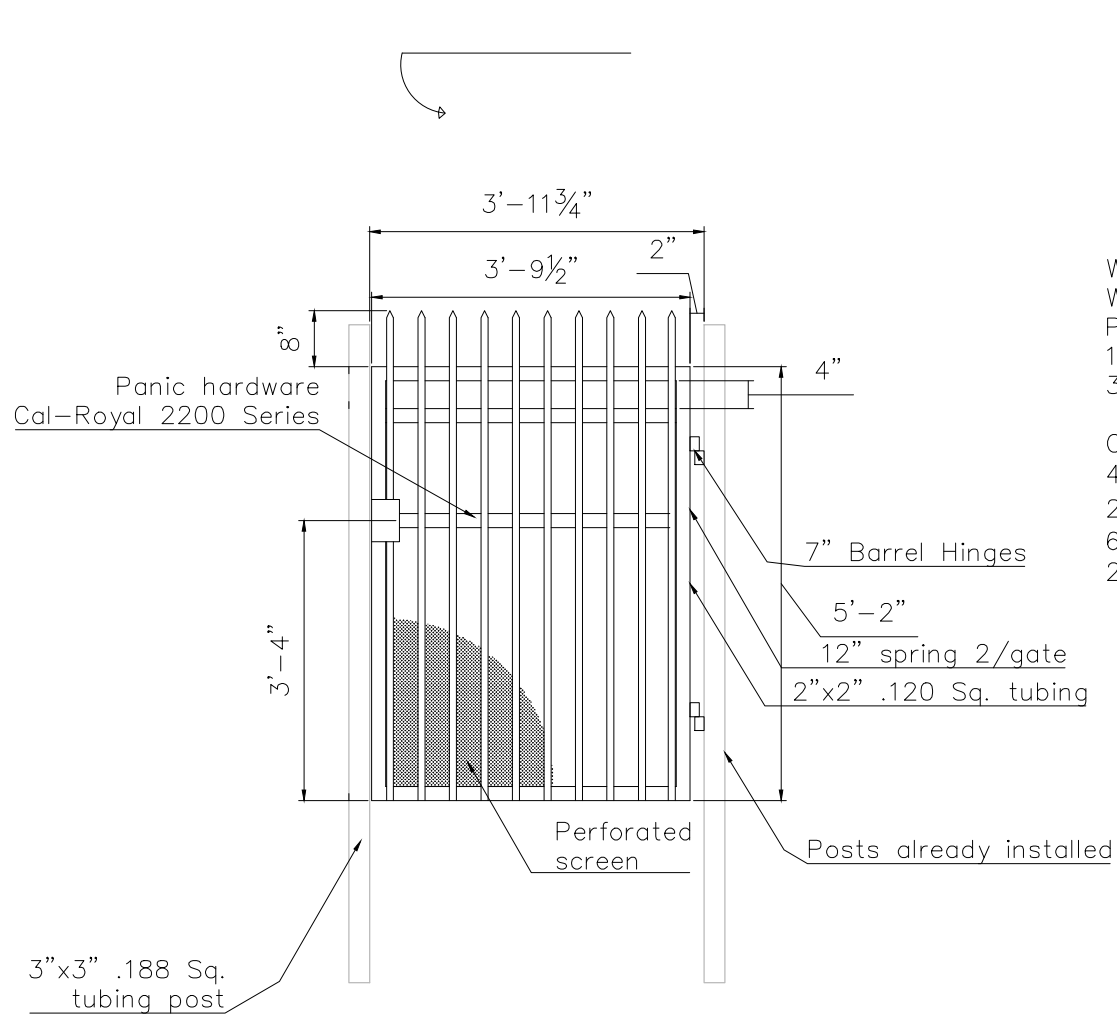


East side Gate entry

Whiting Turner
Westside School
Powder coat
1-25-15
3765

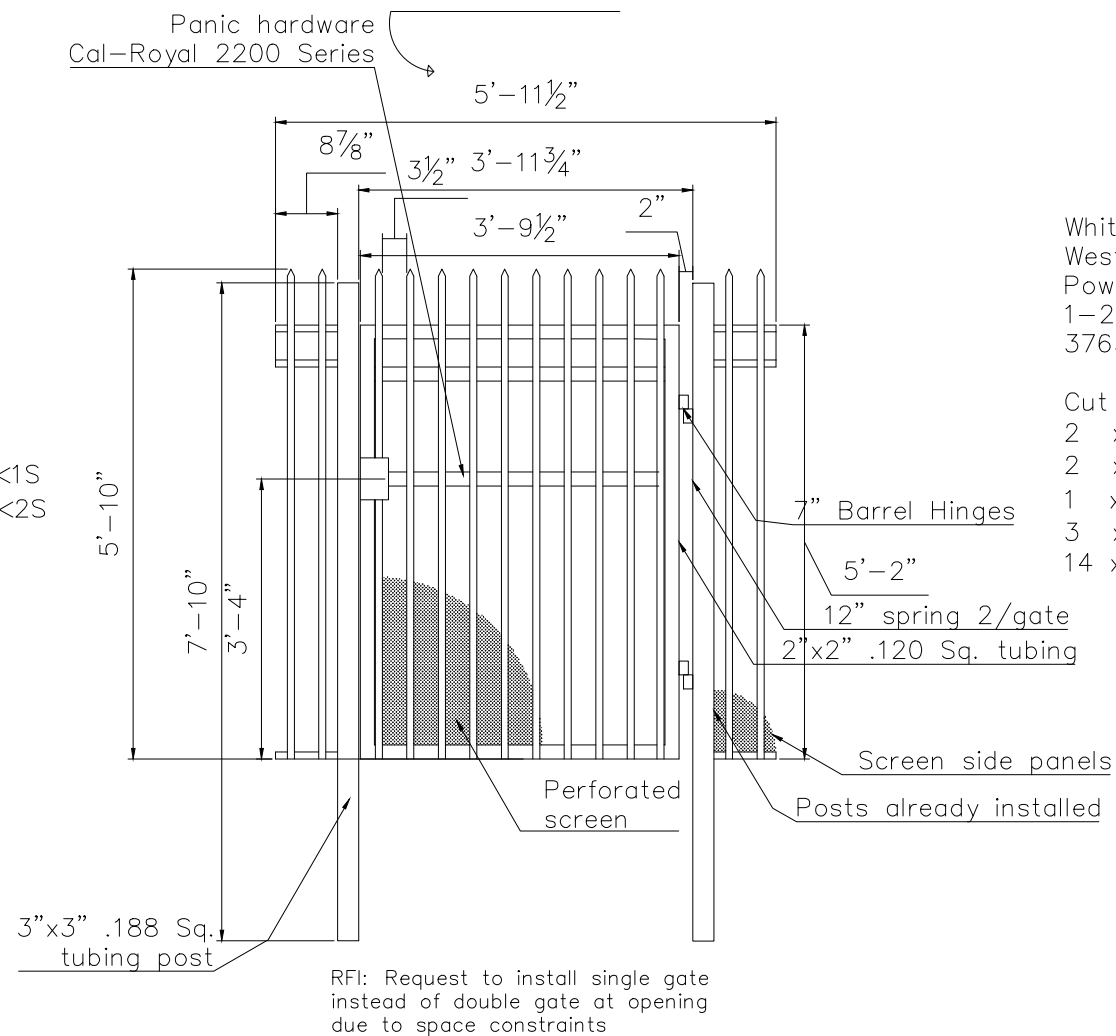
Cut list
4 x 62" 2 x 2 x .095 w/ 45 1 side
2 x 78 3/4" 2 x 2 x .095 w/ 45 2 sides
4 x 74 3/4" 2 x 2 x .095
32 x 70" 1 x 1 x .065

REV.	DATE	DESCRIPTION	NAME
A			
B			
C			
D			
P & L Fencing & Iron LLC. 2842 Marco St. Las Vegas, NV, 89115			
CONTRACTOR		Whiting Turner	DATE 10/15/15
		DRAWN	JPH
PROJECT		Westside School	CHECKED PWH
		SCALE	NTS
LOCATION		330 W. Washington	DWG #
		JOB #	SHEET #
DESCRIPTION		GATE AS BUILTS	3765 A1



Whiting Turner
Westside School
Powder coat
1-22-15
3765

Cut list
4 x 64" 2x2 .120 45<1S
2 x 45 1/2" 2x2 .120 45<2S
6 x 41 1/2" 2x2 .120
20 x 70" 1x1 .065



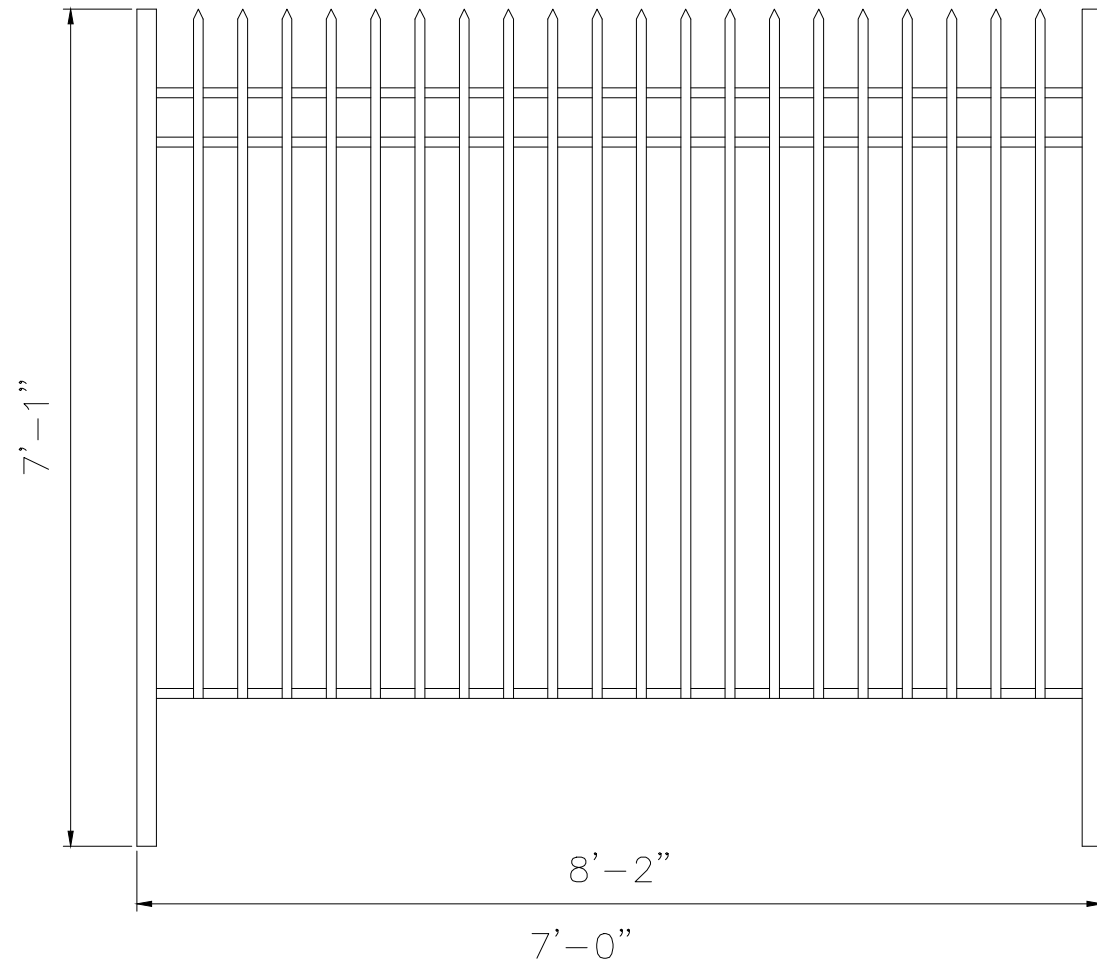
Whiting Turner
Westside School
Powder coat Pale Brown
1-25-15
3765

Cut list
2 x 96" 3x3 .188
2 x 64" 2x2 .120 45<1S
1 x 45 1/2" 2x2 .120 45<2S
3 x 41 1/2" 2x2 .120
14 x 70" 1x1 .065

RFI: Request to install single gate
instead of double gate at opening
due to space constraints

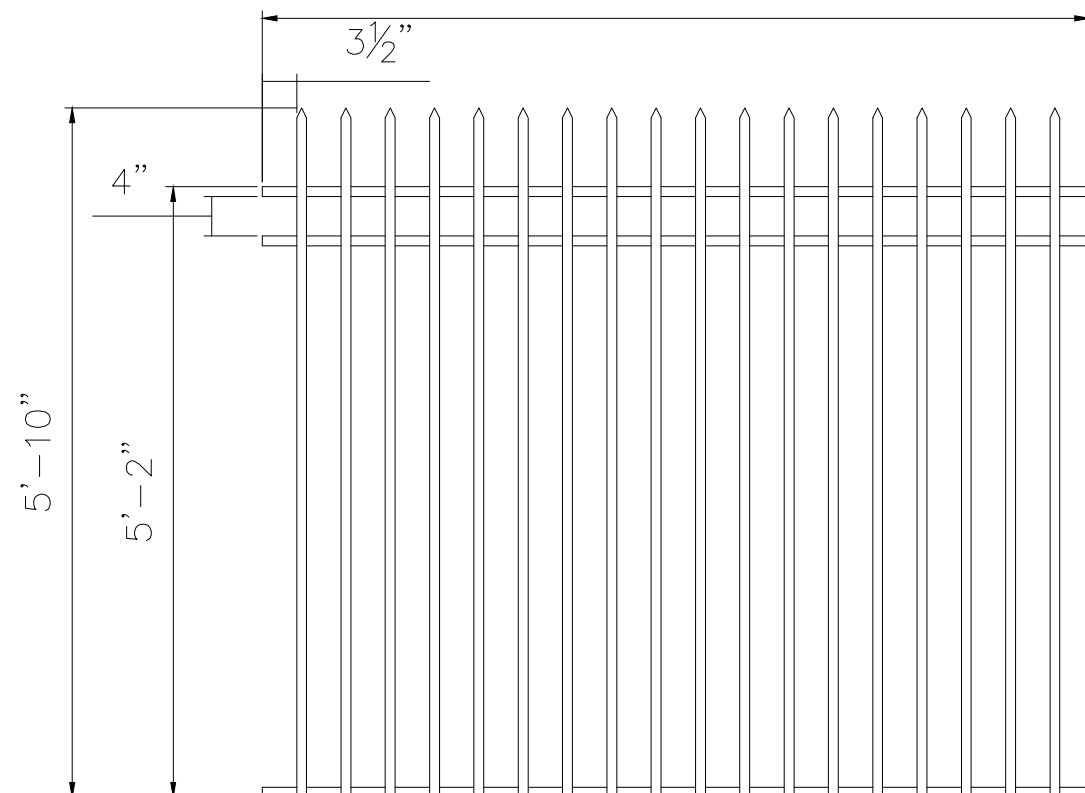
REV.	DATE	DESCRIPTION	NAME
A			
B			
C			
D			

P & L Fencing & Iron LLC. 2842 Marco St. Las Vegas, NV, 89115			
CONTRACTOR	Whiting Turner	DATE	10/15/15
		DRAWN	JPH
PROJECT	Westside School	CHECKED	PWH
		SCALE	NTS
LOCATION	330 W. Washington	DWG #	
		JOB #	
DESCRIPTION	GATE AS BUILTS	SHEET #	G2



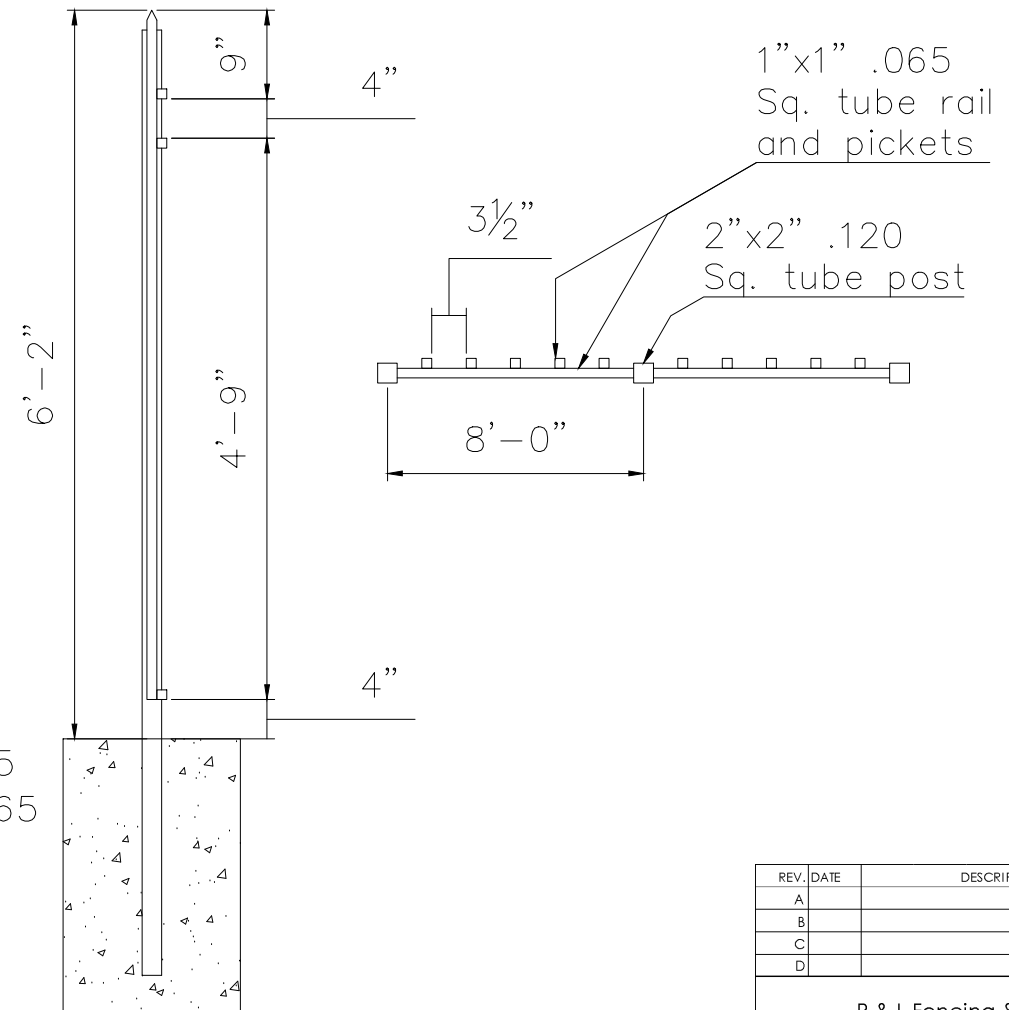
Whiting Turner
Westside School
Powder coat
10-5-15
3765

Cut list
64 x 85" 2x2 .120
96 x 94" 1x1 .095
640 x 70" 1x1 .065

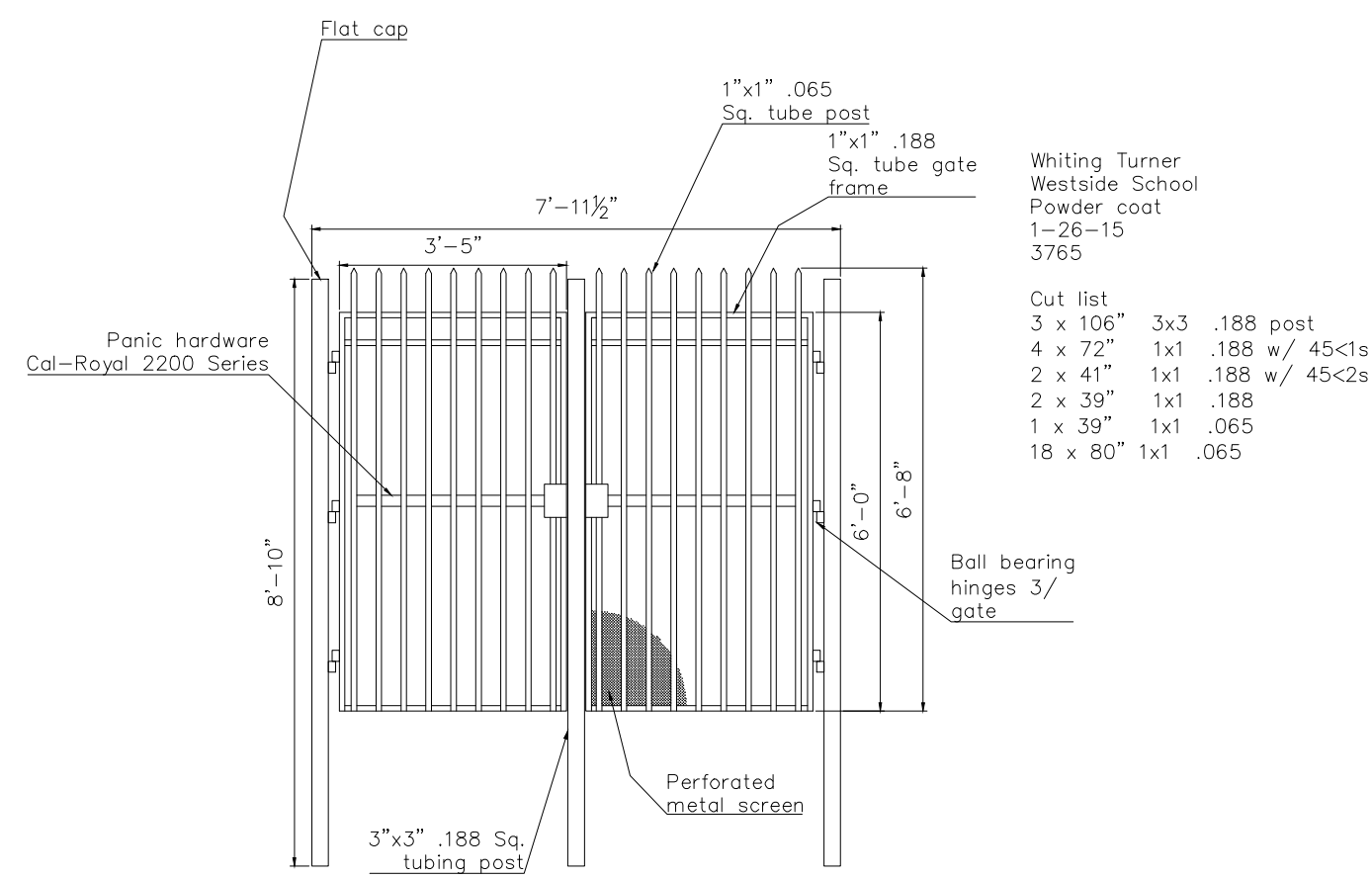


Whiting Turner
Westside School
Powder coat
10-5-15
3765

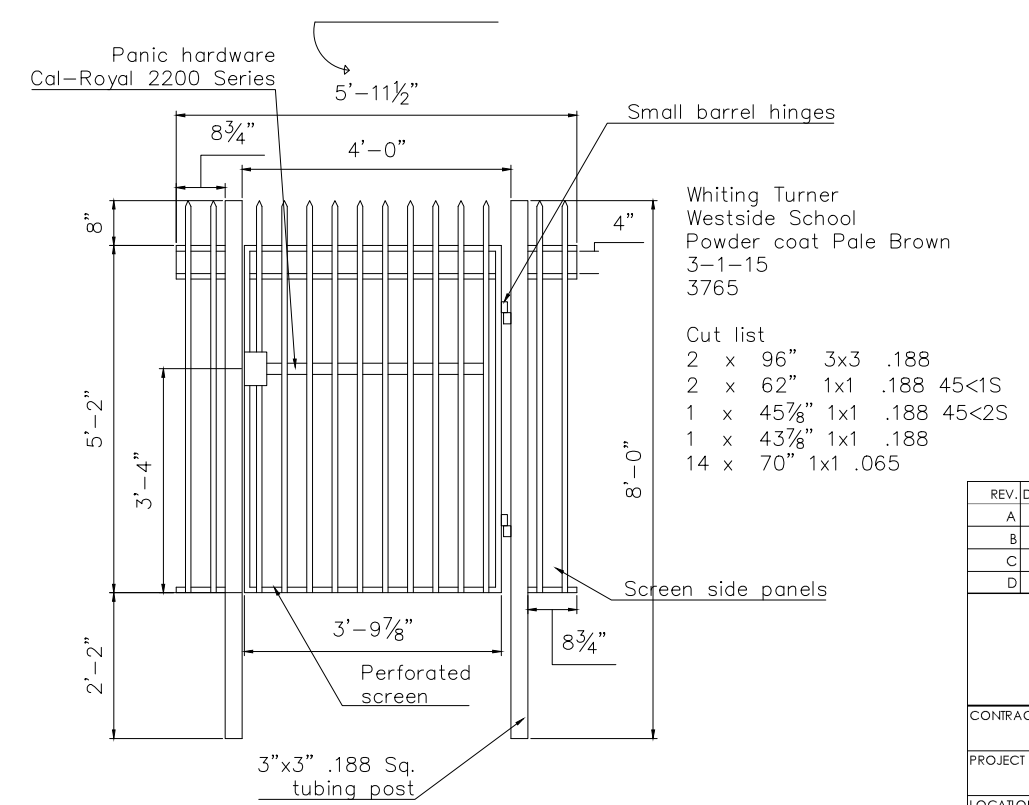
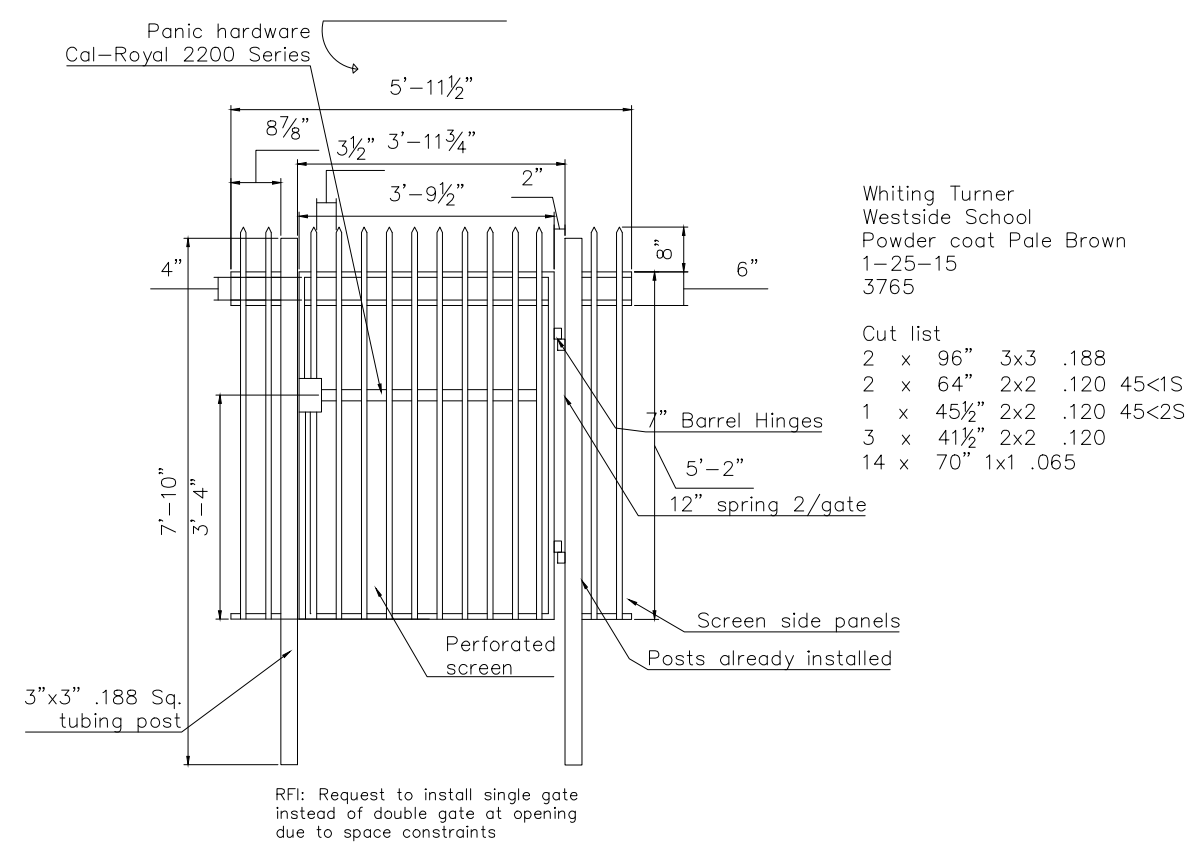
Cut list
192 x 84" 1x1 .095
1088 x 70" 1x1 .065



REV.	DATE	DESCRIPTION	NAME
A			
B			
C			
D			
P & L Fencing & Iron LLC. 2842 Marco St. Las Vegas, NV, 89115			
CONTRACTOR		Whiting Turner	DATE 10/15/15
		DRAWN	JPH
PROJECT		Westside School	CHECKED PWH
		SCALE	NTS
LOCATION		330 W. Washington	DWG #
		JOB #	SHEET #
DESCRIPTION		FENCE AS BUILTS	3765 A7



Interior Access Gates

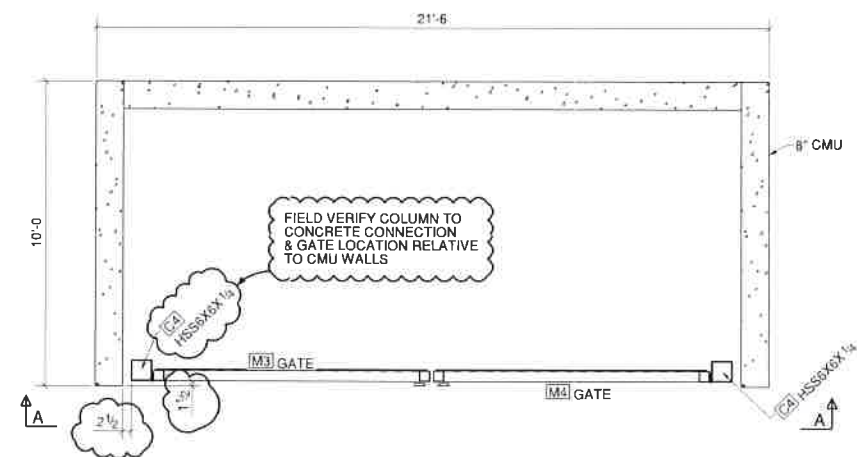


REV.	DATE	DESCRIPTION	NAME
A			
B			
C			
D			

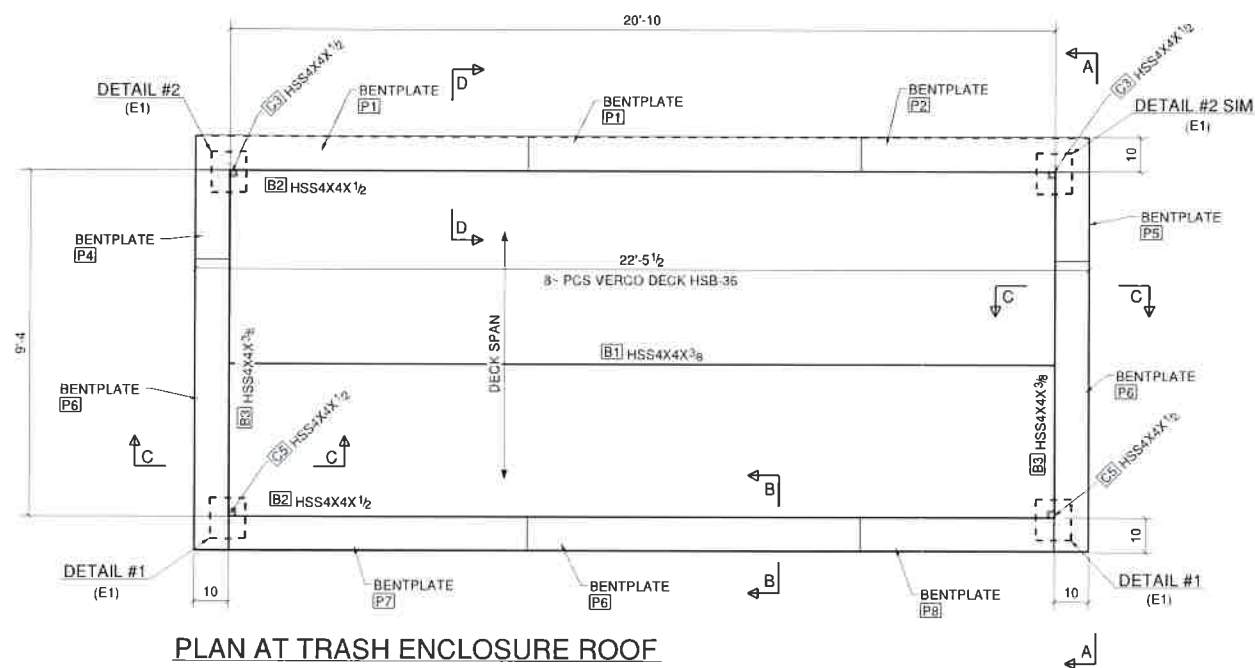
P & L Fencing & Iron LLC. 2842 Marco St. Las Vegas, NV, 89115	
CONTRACTOR	Whiting Turner
PROJECT	Westside School
LOCATION	330 W. Washington
DESCRIPTION	GATE AS BUILTS

DATE	10/15/15
DRAWN	JPH
CHECKED	PWH
SCALE	NTS
DWG #	
JOB #	3765
SHEET #	G3

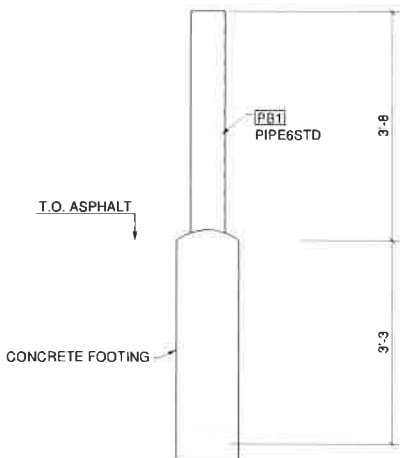
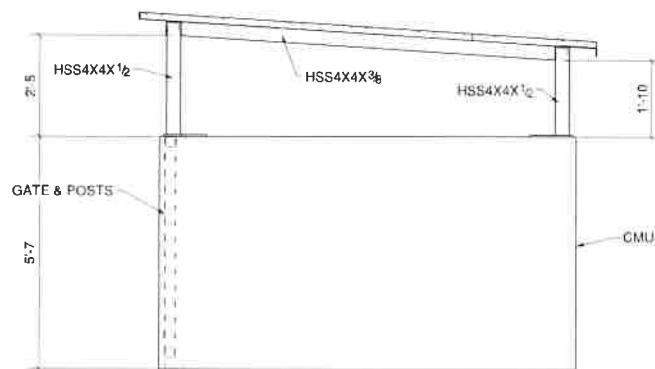
AS-BUILT



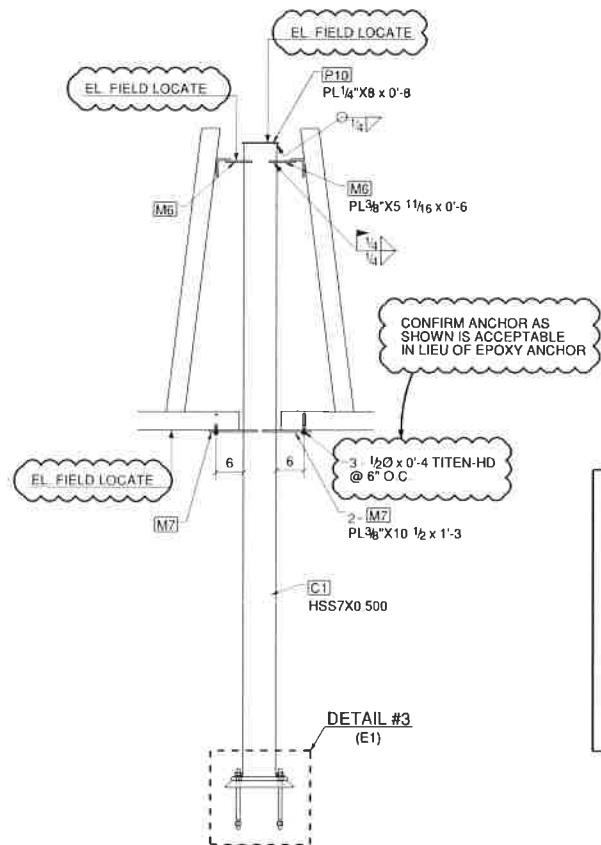
PLAN AT TRASH ENCLOSURE FOUNDATION
REF: S3.1, AS1.10



PLAN AT TRASH ENCLOSURE ROOF
REF: S3.1, AS1.10

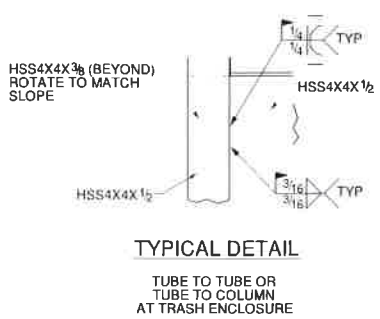


ELEVATION VIEW OF PIPE BOLLARD
REF: 7/AS1.11
12 LOCATIONS



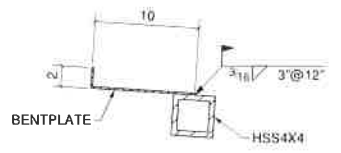
BLDG 1948 CHIMNEY COLUMNS

REF: 110/S3.2
5 LOCATIONS
1 @ S1.1 EAST
1 @ S1.1 SOUTH
1 @ S1.1 WEST
1 @ S1.2 WEST
1 @ S1.2 NORTH



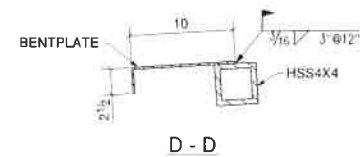
TYPICAL DETAIL

TUBE TO TUBE OR
TUBE TO COLUMN
AT TRASH ENCLOSURE



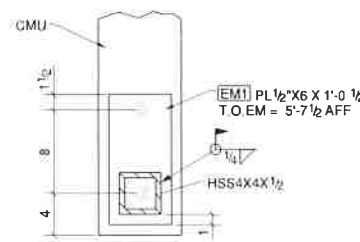
B - B
(ROTATE TO MATCH SLOPE, AS SHOWN)

C - C
(FLAT ROTATION, SLOPING DOWN)

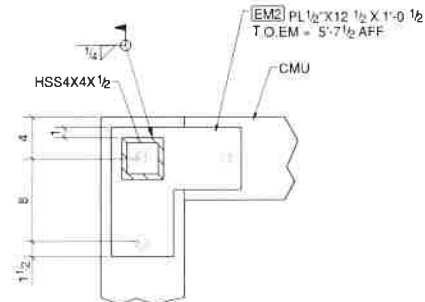


D - D

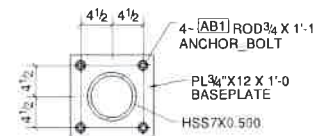
DECK ATTACHMENT DETAILS
PERP. SUPPORTS - (5) PUDDLE WELDS PER SHEET
PARALLEL SUPPORTS - PUDDLE WELDS @ 12 O.C.
SIDE LAPS - BUTTON PUNCHES @ 12" O.C.



DETAIL #1



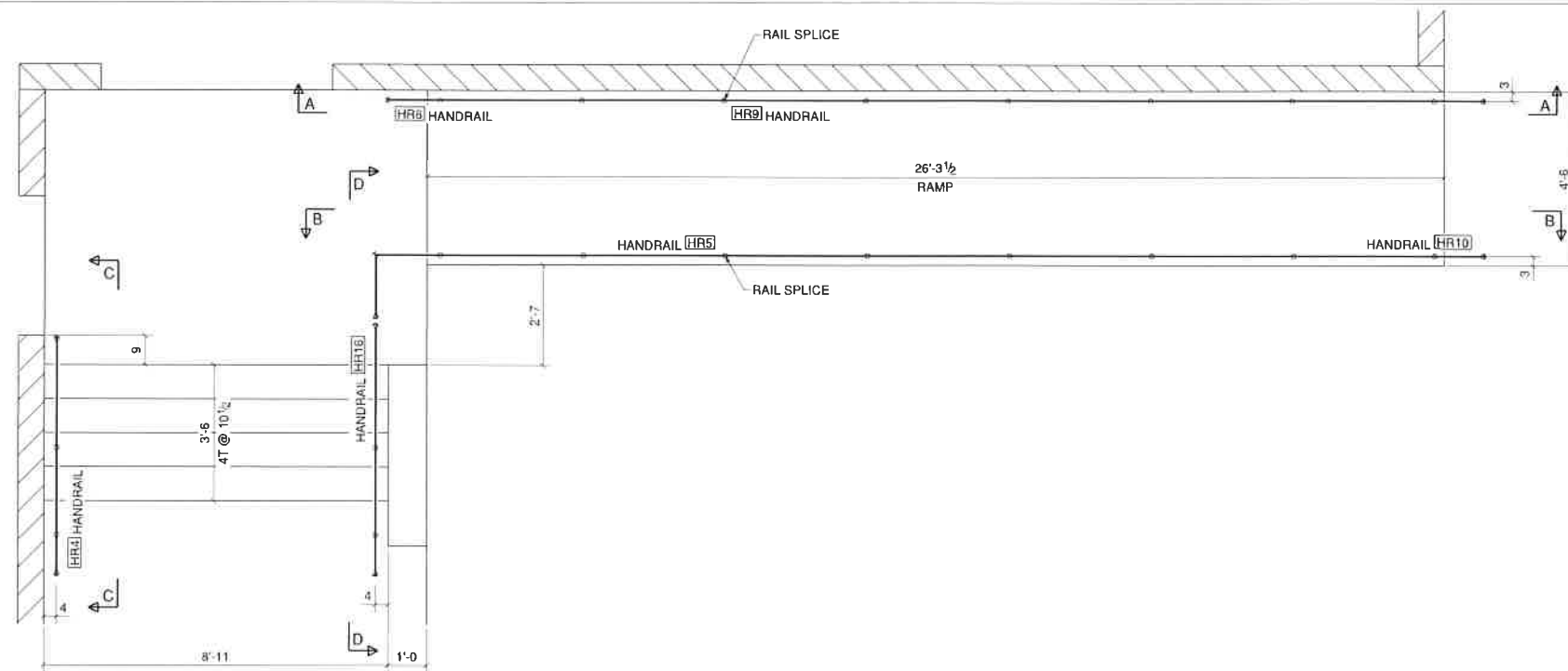
DETAIL #2



DETAIL #3

REV	DATE	DESCRIPTION
0	04/23/15	FOR FIELD USE
1	03/04/15	FOR APPROVAL ONLY
Valley Steel, Inc. 4070 Ponderosa Way Las Vegas, NV 89118		
DESCRIPTION	MISCELLANEOUS METALS	
PROJECT NAME	Westside School - Renovations	
PREPARED BY	JAS	
DRAWN BY	JAS	
DATE DRAWN	03/04/2015	
JOB #	---	DRG #
E1		0

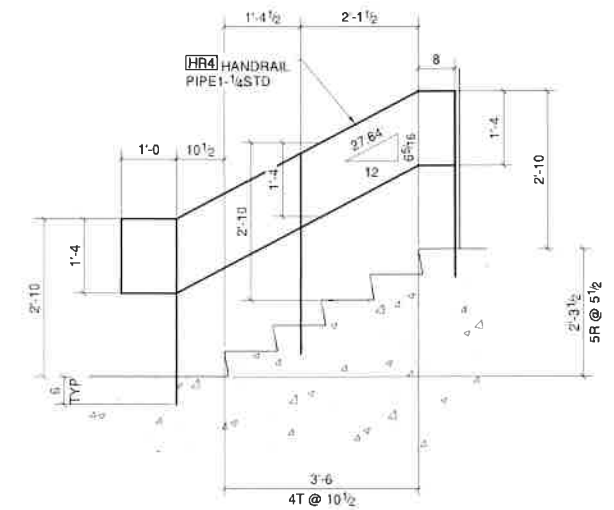
AS-BUILT



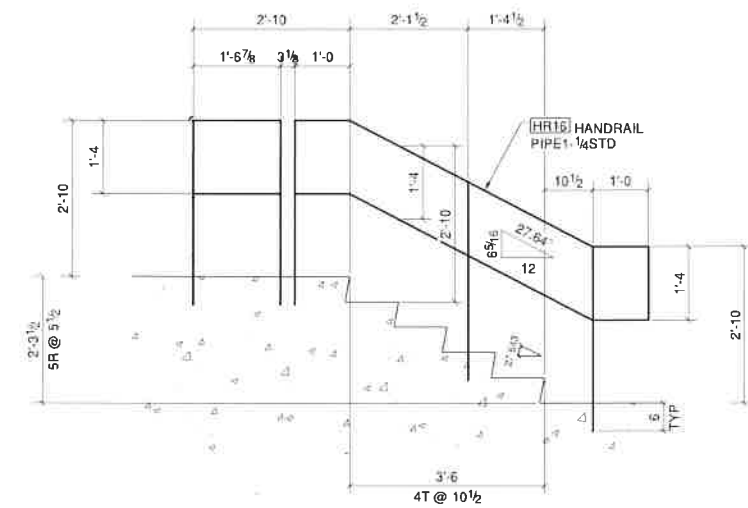
PLAN @ BLDG 1923 NORTH STAIRS

REF: 1/A8.20

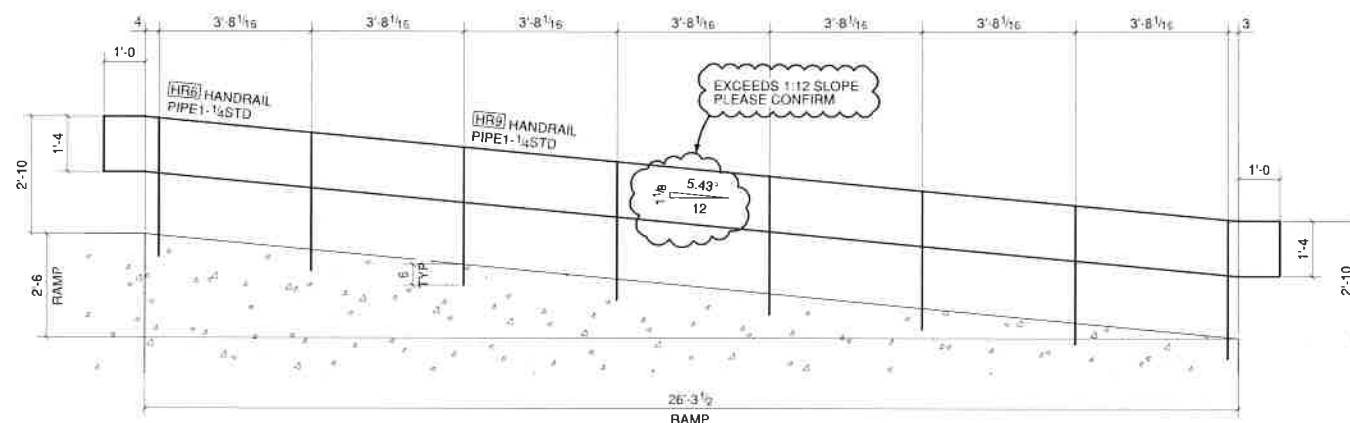
SEE TYPICAL DETAIL FOR RAIL SPLICES WHERE NOTED



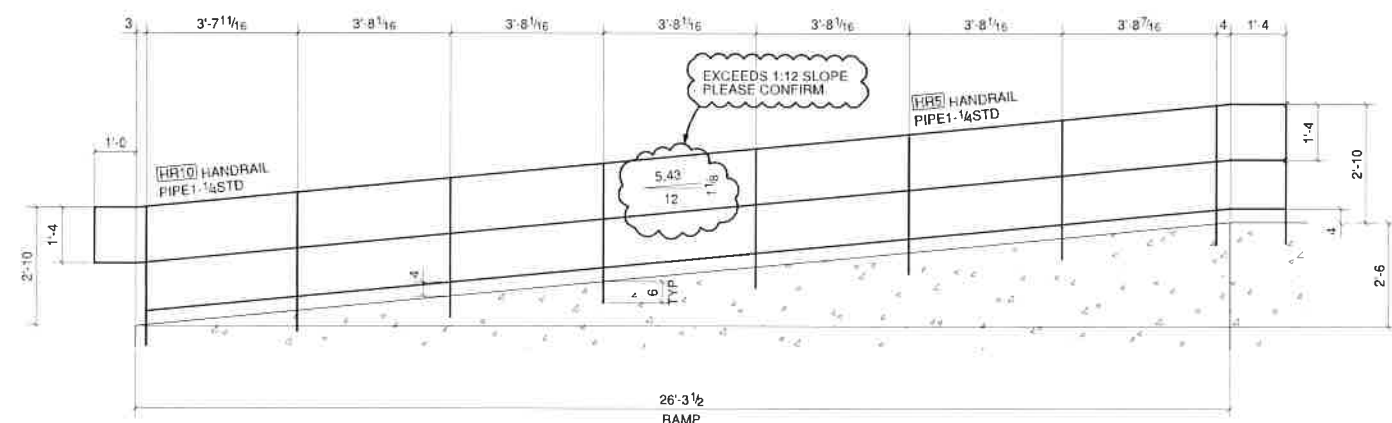
C - C



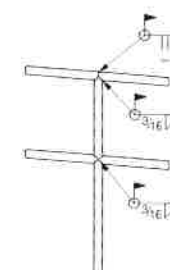
D - D



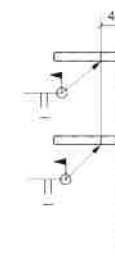
A - A



B - B



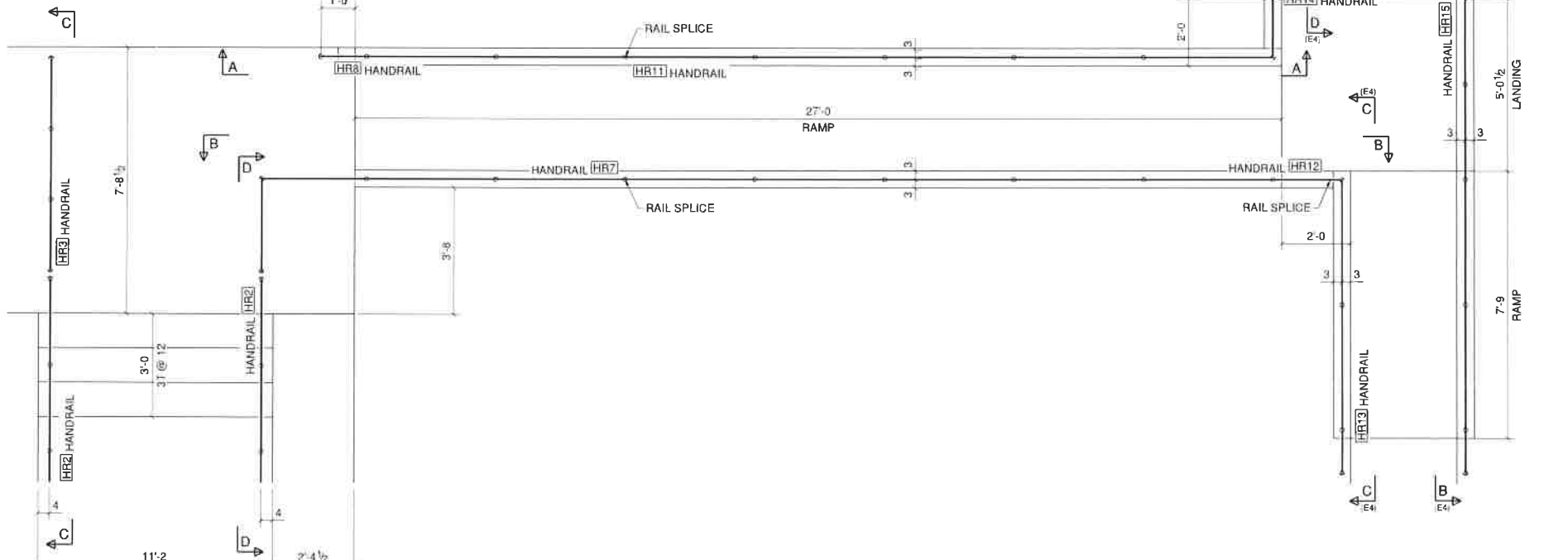
TYPICAL DETAIL
RAIL SPLICE @ POST



TYPICAL DETAIL
RAIL SPLICE @ CORNER

	04/23/15	FOR FIELD USE
	03/04/15	FOR APPROVAL ONLY
REV	DATE	DESCRIPTION
Valley Steel, Inc. 4070 Ponderosa Way Las Vegas, NV 89118		
DESCRIPTION	BLDG 1923 RAILS	
PROJECT NAME	Westside School - Renovations	
PREPARED BY		
DRAWN BY	JAS	JOB No.
DRAWN	03/04/2015	---
		DRG No E2
		

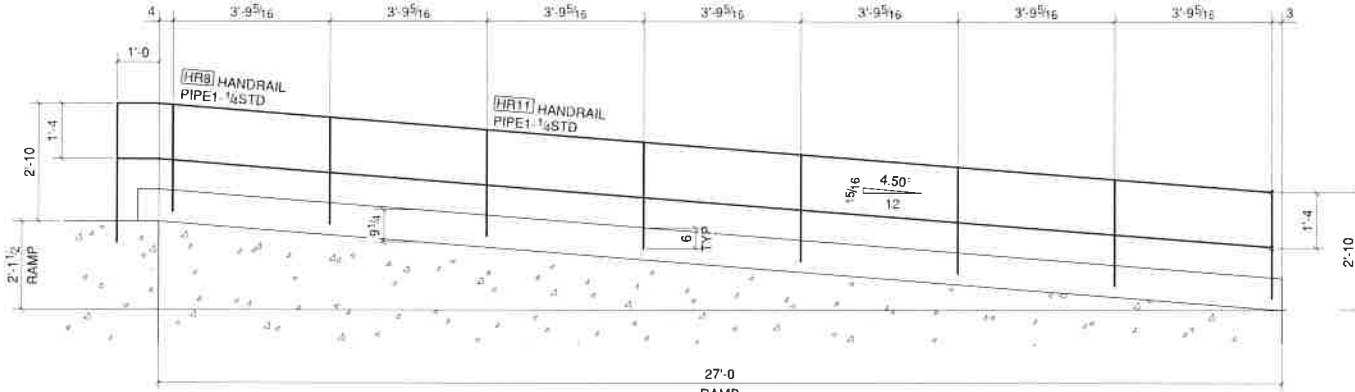
AS-BUILT



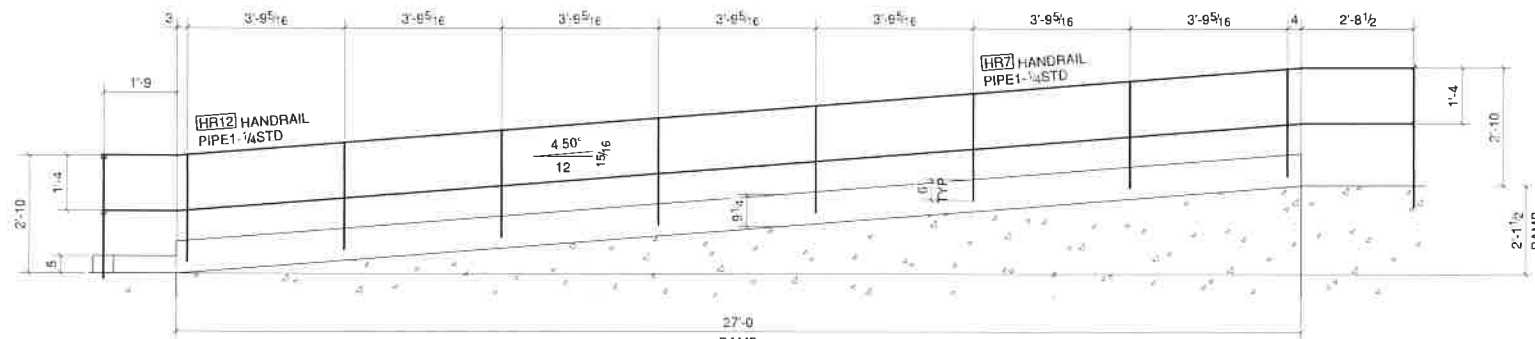
PLAN @ BLDG 1923 SOUTH STAIR & RAMP

REF: 2/A8.20

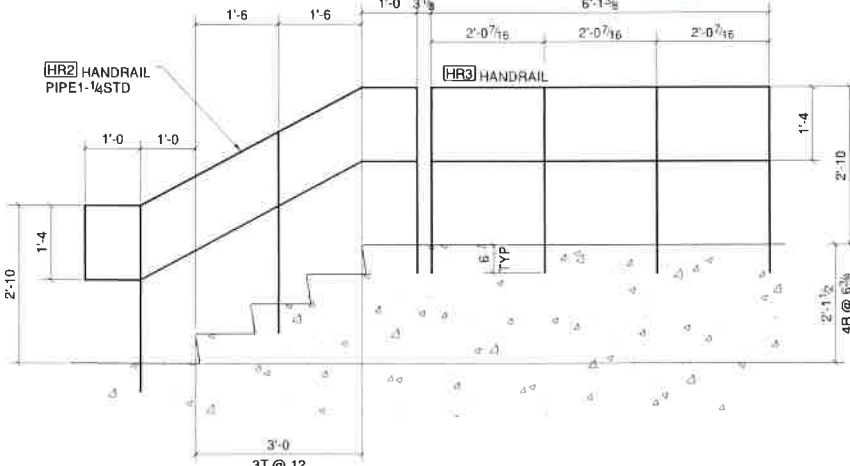
ALL SECTIONS ARE LOCATED ON THIS SHEET (UNO)
SEE TYPICAL DETAIL FOR RAIL SPLICES WHERE NOTED



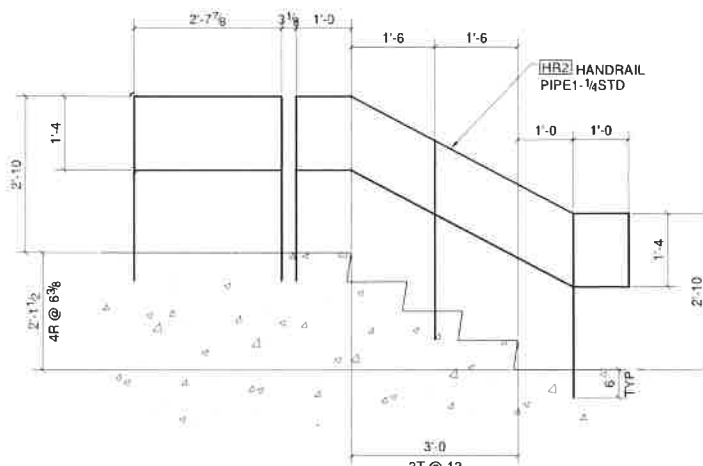
A - A



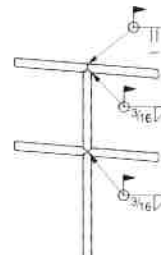
B - E



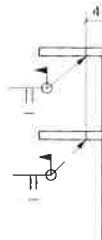
C -



D - D



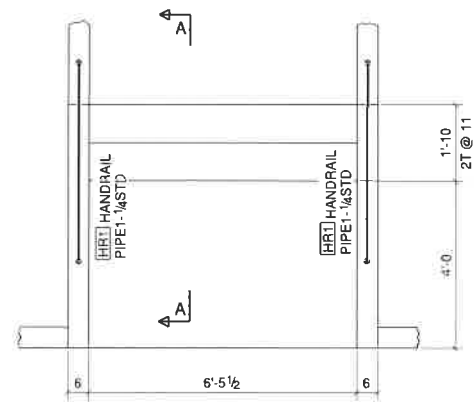
TYPICAL DETAIL
RAIL SPLICE @ POST



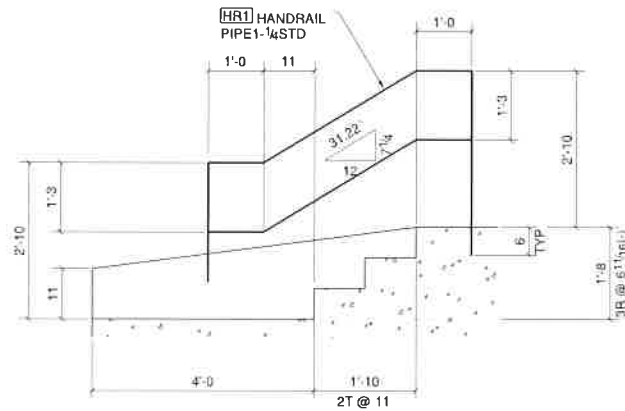
TYPICAL DETAIL
RAIL SPLICE @ CORNER

 	04/23/15	FOR FIELD USE
	03/04/15	FOR APPROVAL ONLY
REV	DATE	DESCRIPTION
Valley Steel, Inc. 4070 Ponderosa Way Las Vegas, NV 89118		
DESCRIPTION	BLDG 1923 RAILS	
PROJECT NAME	Westside School - Renovations	
PREPARED BY		
DRAWN BY	JAS	JCB TJC
DATE DRAWN	03/04/2015	---
		DRG No. E3

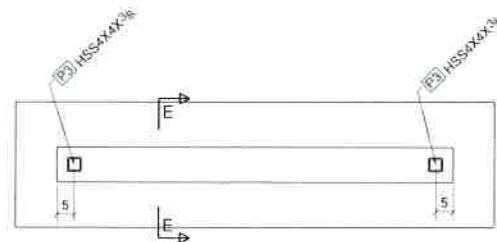
AS-BUILT



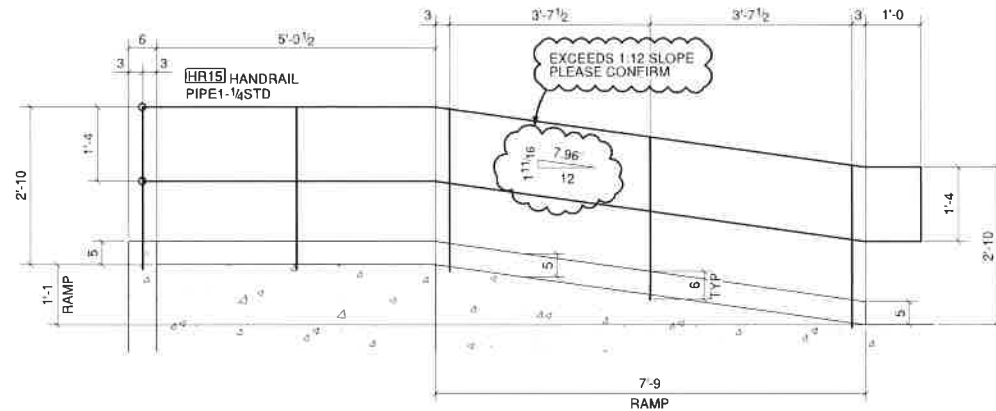
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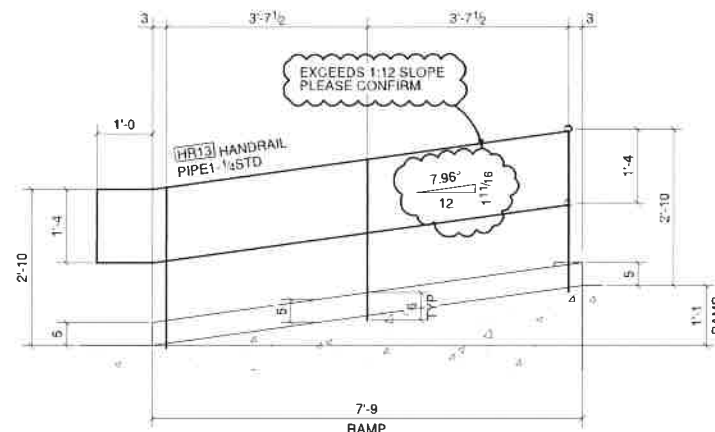
A - A



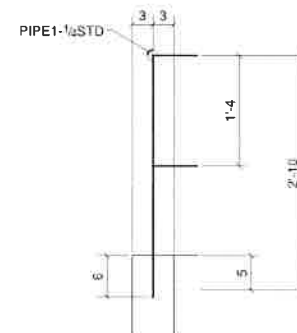
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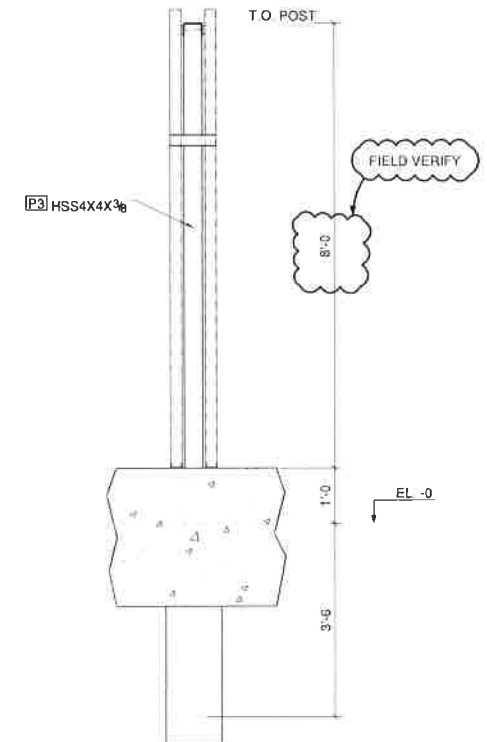
B - B



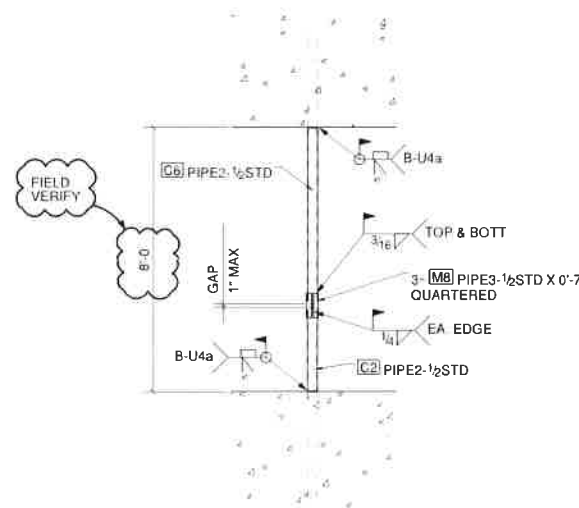
C - C



D - D



E - E



STEEL COLUMN REPAIR
52 LOCATIONS
REF: 214/S4.2

0	04/23/15	FOR FIELD USE
1	03/04/15	FOR APPROVAL ONLY
REV	DATE	DESCRIPTION
Valley Steel, Inc. 4070 Ponderosa Way Las Vegas, NV 89118		
DESCRIPTION	BLDG 1948 RAILS	
PROJECT NAME	Westside School - Renovations	
PREPARED BY		
DRAWN BY	JAS	
DATE DRAWN	03/04/2015	
		0

GENERAL STRUCTURAL NOTES

SEE RFI # 002-001

A. GENERAL

1. THESE DRAWINGS HAVE BEEN PREPARED USING STANDARDS OF PROFESSIONAL CARE AND COMPETENCE NORMALLY EXERCISED UNDER SIMILAR CIRCUMSTANCES BY REPUTABLE STRUCTURAL ENGINEERS IN THIS OR SIMILAR LOCALITIES. THEY ASSUME THAT THE WORK DEPICTED WILL BE PERFORMED BY AN EXPERIENCED CONTRACTOR AND WORKMEN WHO HAVE A WORKING KNOWLEDGE OF THE APPLICABLE CODE STANDARDS AND REQUIREMENTS AND OF INDUSTRY ACCEPTED STANDARD GOOD PRACTICE. AS NOT EVERY CONDITION OR ELEMENT IS (OR CAN BE) EXPLICITLY SHOWN ON THESE DRAWINGS, THE CONTRACTOR SHALL USE INDUSTRY ACCEPTED STANDARD GOOD PRACTICE FOR MISCELLANEOUS WORK NOT EXPLICITLY SHOWN.

2. THESE DRAWINGS REPRESENT THE FINISHED STRUCTURE. THEY DO NOT INDICATE THE METHOD OF CONSTRUCTION. THE CONTRACTOR SHALL BE SOLELY RESPONSIBLE FOR CONSTRUCTION MEANS, METHODS, TECHNIQUES, SEQUENCES, PROCEDURES, LAGGING, SHORING, BRACING, FORT-WORK, ETC. AS REQUIRED FOR THE PROTECTION OF LIFE AND PROPERTY DURING CONSTRUCTION. CONSTRUCTION MATERIALS SHALL BE UNIFORMLY SPREAD OUT SUCH THAT DESIGN LIVE LOAD PER SQUARE FOOT AS NOTED HEREIN IS NOT EXCEEDED.

3. DESIGN OF ITEMS NOT PART OF THE PRIMARY STRUCTURAL SYSTEM (SUCH AS STAIRS, RAILINGS, NON-STRUCTURAL WALLS) AND PREFABRICATED STRUCTURAL ITEMS (SUCH AS FLOOR, ROOF TRUSSES) SHALL BE PROVIDED BY OTHERS UNLESS SPECIFICALLY NOTED ON THESE DRAWINGS. REFER TO SUBMITTALS SECTION FOR ITEMS THAT MUST BE SUBMITTED FOR REVIEW AND FOR SUBMITTAL REQUIREMENTS.

4. THE CONTRACTOR SHALL VERIFY ALL DIMENSIONS, CONDITIONS AND ELEVATIONS WITH ARCH'L DRAWINGS AND RESOLVE ANY DISCREPANCIES WITH THE ARCHITECT PRIOR TO START OF CONSTRUCTION. CONTRACTOR SHALL ESTABLISH AND VERIFY ALL OPENINGS AND INSERTS FOR ARCH'L, MECH, PLUMBING AND ELECTRICAL WITH APPROPRIATE TRADES, DRAWINGS AND SUBCONTRACTORS PRIOR TO CONSTRUCTION.

5. TYPICAL DETAILS AND NOTES SHALL APPLY, THOUGH NOT NECESSARILY INDICATED AT A SPECIFIC LOCATION ON PLANS. WHERE NO DETAILS ARE SHOWN, CONSTRUCTION SHALL CONFORM TO SIMILAR WORK ON THE PROJECT. DETAILS MAY SHOW ONLY ONE SIDE OF CONNECTION OR MAY OMIT INFORMATION FOR CLARITY.

6. NOTES AND DETAILS ON DRAWINGS SHALL TAKE PRECEDENCE OVER GENERAL STRUCTURAL NOTES AND TYPICAL DETAILS.

7. STANDARDS AND CODE REFERENCES NOTED IN THESE CONSTRUCTION DOCUMENTS REFER TO THE EDITIONS ADOPTED BY THE BUILDING CODE SPECIFIED IN THE BASIS FOR DESIGN. LATEST EDITION NOT SPECIFICALLY ADOPTED BY SAID BUILDING CODE REFER TO THE REFINED EDITION.

8. ALL INSPECTIONS REQUIRED BY THE BUILDING CODES, JURISDICTION, OR THESE PLANS SHALL BE PROVIDED BY AN INDEPENDENT INSPECTION COMPANY OR THE BUILDING DEPARTMENT. SITE VISITS BY THE ENGINEER DO NOT CONSTITUTE AN INSPECTION.

B. BASIS FOR DESIGN

1. BUILDING CODE:	INTERNATIONAL BUILDING CODE 2009 WITH SOUTHERN NEVADA ADDENDUMS TO THE 2009 IBC SEISMIC EVALUATION OF EXISTING BUILDINGS ASCE 31-03 INTERNATIONAL EXISTING BUILDING CODE 2009
2. ROOF LOADS:	DEAD LOAD: 15 PSF LIVE LOAD (REDUCIBLE): 20 PSF
3. WIND LOADS:	90 MPH BASIC WIND SPEED EXPOSURE B INTERNAL PRESSURE COEFFICIENT (GCp1) = 0.18
4. SEISMIC LOADS:	SITE CLASS D SEISMIC DESIGN CATEGORY D R = 2 Ss = 0.5671 S1 = 0.16 SDS = 0.509 SD1 = 0.246
5. SNOW LOADS:	NONE

C. FOUNDATION

- FOUNDATIONS DESIGNED PER RECOMMENDATIONS BY CONVERSE CONSULTANTS, REPORT NO. 09-34163-01, DATED JUNE 16, 2010. SITE PREPARATION, GRADING, TESTS, INSPECTIONS, FIELD OBSERVATIONS, OR APPROVAL FROM THE GEOTECHNICAL ENGINEER RECOMMENDED BY THE GEOTECHNICAL ENGINEER AND ANY ADDENDA SHALL BE COMPLETED PRIOR TO CONSTRUCTION OF FOUNDATIONS.
- ALLOWABLE DEAD PLUS LIVE LOAD SOIL PRESSURE = 1200 PSF.
- TRENCHES AND EXCAVATIONS UNDER OR ADJACENT TO FOUNDATIONS SHALL BE PROPERLY BACKFILLED AND COMPACTED.
- WATER PROOFING AS MAY BE REQUIRED AT SOIL FACE OF WALLS BELOW GRADE SHALL BE BY OTHERS.

D. CONCRETE

- ALL CONCRETE CONSTRUCTION SHALL BE PERFORMED IN ACCORDANCE WITH ACI 318 AND ACI 301, EXCEPT AS MODIFIED BY THE CONSTRUCTION DOCUMENTS.
- DUE TO SEVERE/VERY SEVERE SULFATE CONTENT OF THE ON-SITE SOILS, MIN. 28 DAY COMPRESSIVE STRENGTH, F_c, SHALL BE 4500 PSI AND MAX. WATER/CEMENT RATIO SHALL BE 0.45 FOR ALL CONCRETE. (FOUNDATION DESIGN BASED ON 2500 PSI)
- CONCRETE MIXES SHALL BE DESIGNED BY A CERTIFIED LABORATORY, STAMPED BY AN APPROPRIATELY LICENSED SPECIALTY ENGINEER, AND APPROVED BY THE ENGINEER OF RECORD. MIX DESIGNS SHALL INCLUDE THE PROJECT NAME AND INDICATE THEIR USE WITHIN THE STRUCTURE.
- IF USED, EARLY STRENGTH CONCRETE SHALL BE PROPORTIONED TO DEVELOP THE 28 DAY COMPRESSIVE STRENGTH AT THE AGE REQUIRED BY THE CONTRACTOR. CONTRACTOR SHALL SUBMIT TEST DATA FOR REVIEW BY THE STRUCTURAL ENGINEER TO SUBSTANTIATE THE CONCRETE STRENGTH AT THE REQUIRED AGE.
- ALL CONCRETE SHALL BE NORMAL WEIGHT OF 145 POUNDS PER CUBIC FOOT USING HARD ROCK AGGREGATES CONFORMING TO ASTM C33 UNO. WHERE LIGHTWEIGHT CONCRETE IS SPECIFIED, CONCRETE SHALL BE 110 POUNDS PER CUBIC FOOT USING AGGREGATES CONFORMING TO ASTM C330. LARGEST NOMINAL AGGREGATE SIZE SHALL BE 1-1/2" OR GREATER FOR SLABS ON GRADE AND 3/4" OR GREATER FOR ALL OTHER CONCRETE UNO.
- MAX. SLUMP SHALL BE 5 INCHES (EXCEPTION: WHERE ADMIXTURES/PLASTICIZERS HAVE BEEN INCLUDED IN MIX DESIGN TO IMPROVE WORKABILITY, SLUMP LIMIT SHALL BE BASED ON ADMIXTURE MFR'S RECOMMENDATIONS). MIX WATER SHALL BE CLEAN AND POTABLE.
- PORTLAND CEMENT SHALL CONFORM TO ASTM C150. TYPE V CEMENT SHALL BE USED FOR CONCRETE IN CONTACT WITH EARTH. TYPE II CEMENT MAY BE USED ELSEWHERE. CEMENT SHALL BE TYPE V WITH POZZOLAN WHERE CONCRETE IS IN CONTACT WITH SOIL CONTAINING VERY SEVERE SULFATE EXPOSURE.
- FLY ASH MAY BE USED IN CONCRETE, SUBJECT TO APPROVAL BY THE ARCHITECT, PROVIDED THE FOLLOWING CONDITIONS ARE MET:
 - FLY ASH SHALL COMPLY WITH ASTM C618.
 - CEMENT CONTENT SHALL BE REDUCED A MINIMUM OF 15 PERCENT UP TO A MAXIMUM OF 25 PERCENT WHEN COMPARED TO AN EQUIVALENT CONCRETE MIX DESIGN WITHOUT FLY ASH. FLY ASH CONTENT SHALL NOT COMPRISE MORE THAN 35 PERCENT OF THE TOTAL CEMENTITIOUS CONTENT. THE WATER/CEMENT RATIO SHALL BE CALCULATED BASED ON THE TOTAL CEMENTITIOUS MATERIAL IN THE MIX.

D. CONCRETE (CONT'D)

- CLASS F FLY ASH SHALL BE USED IN SULFATE RESISTANT CONCRETE WITH F_c EQUAL TO OR GREATER THAN 4000 PSI. CLASS C FLY ASH MAY BE USED ELSEWHERE.
- WATER SOLUBLE CHLORIDE ION CONCENTRATIONS IN CONCRETE SHALL BE LIMITED PER ACI 318, SECTION 4.4.
- TIME BETWEEN CONCRETE BATCHING AND PLACEMENT SHALL BE IN ACCORDANCE WITH ASTM C94.
- CONCRETE MIXING, PLACEMENT AND QUALITY SHALL BE PER IBC SECTION 1909. MECHANICALLY VIBRATE ALL CONCRETE WHEN PLACED. SLABS ON GRADE NEED BE VIBRATED ONLY AROUND AND UNDER FLOOR DUCTS OR SIMILAR ELEMENTS. REMOVE ALL DEBRIS FROM FORMS BEFORE PLACING CONCRETE. CONCRETE SHALL NOT BE DROPPED THROUGH REINFORCING STEEL SO AS TO CAUSE SEGREGATION OF AGGREGATES. UNCONFINED FALL OF CONCRETE SHALL NOT EXCEED 5 FEET.
- PROTECT CONCRETE FROM DAMAGE OR REDUCED STRENGTH DUE TO COLD OR HOT WEATHER IN ACCORDANCE WITH ACI 305 AND 306. CONTRACTOR SHALL TAKE SPECIAL CURING PRECAUTIONS TO MINIMIZE SHRINKAGE CRACKING OF CONCRETE SLABS.
- ALL ITEMS TO BE CAST IN CONCRETE SUCH AS REINFORCEMENT, DOUELS, BOLTS, ANCHORS, SLEEVES, ETC., SHALL BE SECURELY POSITIONED IN THE FORMS.
- CONSTRUCTION JOINT SURFACES SHALL BE CLEANED AND LAITANCE REMOVED. HORIZONTAL JOINT SURFACES SHALL BE ROUGHENED TO 1/4" AMPLITUDE. THOROUGHLY WET ALL JOINT SURFACES AND REMOVE STANDING WATER IMMEDIATELY PRIOR TO NEW CONCRETE PLACEMENT.
- CONCRETE SHALL BE CURED IN ACCORDANCE WITH ACI 318, SECTIONS 5.11.1 OR 5.11.2, WHICHEVER IS APPLICABLE, UNLESS ALTERNATE METHODS HAVE BEEN APPROVED BY THE ARCHITECT AND ENGINEER. WHERE CURING COMPOUNDS HAVE BEEN APPROVED FOR SLAB CURING, CONTRACTOR SHALL BE RESPONSIBLE FOR VERIFYING COMPATIBILITY OF COMPOUNDS WITH ANTICIPATED FLOOR FINISH (e.g., RESILIENT TILE) PRIOR TO CURING COMPOUND APPLICATION.
- GROUT BENEATH COLUMN BASES OR BEARING PLATES SHALL BE 5000 PSI (MIN) NON-SHRINK FLOWABLE GROUT OR DRYPACK. INSTALL GROUT BENEATH BEARING PLATES BEFORE FRAMING MEMBER IS INSTALLED. AT COLUMNS, INSTALL GROUT BENEATH BASE PLATES AFTER COLUMN HAS BEEN PLUMBED. FRAMING NOT INCLUDING CONCRETE OVER STEEL DECK OR CONCRETE TOPPING AS OCCURS) MAY BE INSTALLED ONE LEVEL ABOVE BASE PLATE PRIOR TO PLACING GROUT BENEATH BASE PLATES. GROUT SHALL BE PLACED BENEATH BASE PLATES PRIOR TO INSTALLATION OF ANY FRAMING TWO OR MORE LEVELS ABOVE BASE PLATE. GROUT DEPTH SHALL BE 1 1/2" TYPICAL, OR SHALL BE SUFFICIENT TO ALLOW GROUT OR DRYPACK TO BE PLACED BENEATH PLATE WITHOUT VOIDS.

E. MASONRY

- ALL MASONRY CONSTRUCTION SHALL BE PERFORMED IN ACCORDANCE WITH IBC 2104 AND ACI 530.1.
- MASONRY COMPRESSIVE STRENGTH, F_m, SHALL BE 1500 PSI UNO. AND SHALL BE VERIFIED BY THE PROBT TEST METHOD OUTLINED IN IBC 2105.2.22. AS A MINIMUM, 28 DAY COMPRESSIVE STRENGTH OF INDIVIDUAL MASONRY COMPONENTS (i.e., BLOCK, GROUT, AND MORTAR) SHALL BE AS NOTED BELOW. GREATER STRENGTHS SHALL BE USED AS REQUIRED FOR COMBINED SYSTEM TO ACHIEVE SPECIFIED VALUE OF F_m. MIN. BLOCK STRENGTH SPECIFIED IS ON NET AREA.

F _m :	BLOCK:	GROUT:	MORTAR:	MORTAR TYPE:
1500 PSI	1900 PSI	2000 PSI	1800 PSI	TYPE 5
2000 PSI	2800 PSI	2800 PSI	1800 PSI	TYPE 5
2500 PSI	3750 PSI	3750 PSI	2500 PSI	TYPE M

3. STRUCTURAL MASONRY SHALL BE HOLLOW, MEDIUM WEIGHT (115 PCF) LOAD-BEARING CONCRETE MASONRY UNITS CONFORMING TO IBC 2103.1. BLOCK TEST DATA BY A CERTIFIED LABORATORY SHALL BE SUBMITTED FOR REVIEW. ALL BLOCKS SHALL BE PLACED IN RUNNING BOND CONSTRUCTION UNO. WITH ALL VERTICAL CELLS IN ALIGNMENT.

4. GROUT SHALL CONFORM TO REQUIREMENTS OF IBC 2103.12. USE SUFFICIENT WATER FOR GROUT TO FLOW INTO ALL SPACES. FLY ASH SHALL BE USED IN GROUT. FLY ASH IS NOT PERMITTED IN GROUT. ONLY SOLID GROUT CELLS WITH REINFORCING UNLESS REQUIREMENT TO SOLID GROUT ENTIRE WALL IS SPECIFICALLY NOTED ON PLANS OR SCHEDULE. BETWEEN GROUT LIFTS, FORM HORIZONTAL CONSTRUCTION JOINTS PER IBC 2104.12.1.

5. MORTAR MIX SHALL CONFORM TO REQUIREMENTS OF IBC 2103.8. SEE TABLE ABOVE FOR MORTAR TYPE.

6. GROUT AND MORTAR MIXES SHALL BE DESIGNED BY A CERTIFIED LABORATORY, STAMPED BY AN APPROPRIATELY LICENSED SPECIALTY ENGINEER, AND APPROVED BY THE ENGINEER OF RECORD. MIX DESIGNS SHALL INCLUDE THE PROJECT NAME AND INDICATE THEIR USE WITHIN THE STRUCTURE.

7. LAP REINFORCING BARS PER TYPICAL REBAR LAP SCHEDULE (MASONRY) UNO.

8. REINFORCING SHALL BE SECURED IN ITS PROPER POSITION WITHIN THE CELL TO PREVENT LATERAL DISPLACEMENT PRIOR TO GROUTING BY WIRE POSITIONERS OR OTHER SUITABLE DEVICES AT INTERVALS NOT EXCEEDING 10'-0" O.C. MAX.

9. MIN. WALL VERT. REINF. UNO. ON PLANS OR DETAILS, SHALL BE 5 BAR VERT. FULL HEIGHT IN CENTER OF GROUTED CELL. AT ALL WALL INTERSECTIONS, CORNERS, WALL ENDS, JAMBS AT WALL OPENINGS, AND AT EACH SIDE OF CONTROL JOINTS. REFER TO PLAN FOR TYPICAL WALL VERT. REINF. SIZE AND SPACING. DOUEL ALL VERT. REINF. TO FOUNDATION WITH DOUELS TO MATCH AND LAP VERT. REINF.

10. MIN. WALL HORIZ. REINF. UNO. ON PLANS OR DETAILS, SHALL BE (2) 5 BARS IN CENTER OF 32" DEEP (MIN) CONTINUOUS GROUTED BOND BEAM AT ELEVATED FLOOR AND ROOF LINES AND SINGLE 5 BAR IN CENTER OF 16" DEEP CONTINUOUS GROUTED BOND BEAM AT TOP OF PARAPET OR FREE STANDING WALL AND AT INTERVALS NOT TO EXCEED 48" O.C. PLACE BARS AT ELEVATED FLOOR AND ROOF LINES CONTINUOUS THROUGH CONTROL JOINTS. PROVIDE BENT BARS PER TYPICAL DETAILS TO MATCH AND LAP HORIZ. BOND BEAM REINF. AT CORNERS AND WALL INTERSECTIONS TO MAINTAIN BOND BEAM CONTINUITY. USE BOND BEAM WITH HORIZ. REINF.

11. MIN. LINTEL, WHERE NOT NOTED ON PLANS, SHALL HAVE A MIN. OF (2) 5 CONTINUOUS HORIZ. BARS IN BOTTOM OF BOND BEAM OR LINTEL BLOCK AND SHALL BE GROUTED SOLID TO A MIN. DEPTH OF 24". SILLS SHALL BE REINFORCED WITH SINGLE 5 BAR IN BOND BEAM BLOCK GROUTED SOLID TO MIN. DEPTH OF 8". ALL LINTEL OR SILL REINF. AND GROUT SHALL EXTEND 2'-0" MIN. PAST JAMBS UNO. ON PLANS OR DETAILS.

12. MASONRY VENEER SHALL BE ATTACHED TO SUPPORTING WALL FRAMING WITH 3/16" DIA. WALL TIES OR DOVETAIL-TYPE METAL TIES OF EQUIVALENT STIFFNESS EMBEDDED INTO HORIZ. MORTAR JOINTS. VERT. SPACINGS OF TIES SHALL BE 16" O.C. MAX. HORIZ. SPACING SHALL BE 24" O.C. MAX. TIES IN ALTERNATE COURSES SHALL BE STAGGERED. PROVIDE 5 WIRE REINF. IN HORIZ. MORTAR JOINTS AT 16" O.C. ENGAGE 5 WIRE WITH WALL ANCHOR TIES. CONSTRUCTION JOINTS IN MASONRY VENEER WALLS SHALL BE LOCATED PER ARCH'L DRAWINGS.

13. MECH, ELECT, AND PLUMBING PENETRATIONS THRU MASONRY SHALL COMPLY WITH THE FOLLOWING:

13.1. DO NOT CUT ANY REINF. THAT MAY INTERFERE WITH PENETRATIONS. INSTALL ANY SLEEVES REQUIRED BY MECH, ELECT, OR PLUMBING PRIOR TO GROUTING.

13.2. PENETRATIONS SHALL NOT BE CORED OR CUT INTO MASONRY WITHOUT PRIOR WRITTEN APPROVAL OF ENGINEER THRU THE ARCHITECT.

13.3. PENETRATIONS THRU LINTELS, FILASTERS, AND JAMBS ARE PERMITTED ONLY WHERE SPECIFICALLY DETAILED.

13.4. CONDUIT AND PIPING PARALLEL TO PLANE OF WALL SHALL NOT BE EMBEDDED IN WALL UNO.

F. REINFORCING STEEL

- REINFORCING STEEL SHALL BE DETAILED AND PLACED IN ACCORDANCE WITH ACI 318 AND CRSI'S MANUAL OF STANDARD PRACTICE.
- REINFORCING STEEL SHALL CONFORM TO ASTM A615 OR ASTM A706 (A706 REQUIRED FOR ALL REINFORCING TO BE WELDED) AND SHALL BE GRADE 60 (F_y = 60 KSI) DEFORMED BARS UNO. REINFORCING IN SLABS ON GRADE MAY BE GRADE 40 (F_y = 40 KSI) DEFORMED BARS FOR ALL BARS 4" AND SMALLER UNO. ON PLANS OR DETAILS.

3. WELDED WIRE FABRIC SHALL CONFORM TO THE REQUIREMENTS OF ASTM A106. LAPS IN WELDED WIRE FABRIC SHALL BE MADE SUCH THAT THE OVERLAP, MEASURED BETWEEN OUTERMOST CROSS WIRE OF EACH FABRIC SHEET, IS NOT LESS THAN THE SPACING OF CROSS WIRES PLUS 2 INCHES.

4. RECTANGULAR FLAT DOUELS AND SMOOTH ROUND DOUELS USED AT CONTROL AND CONSTRUCTION JOINTS IN SLABS ON GRADE SHALL CONFORM TO ASTM A36. REFER TO TYPICAL CONTROL JOINTS IN SLAB ON GRADE DETAIL FOR SIZE, PLACEMENT, SPACING, ETC. RECTANGULAR FLAT DOUELS SHALL BE BY FNA CONSTRUCTION TECHNOLOGIES (800-842-0714) OR OTHER MFR. APPROVED BY ENGINEER. INSTALL ALL PLATE DOUEL BASKET ASSEMBLIES PER MFR'S RECOMMENDATIONS.

5. ALL DIMENSIONS SHOWING THE LOCATION OF REINFORCING STEEL NOT NOTED AS "CLEAR" OR "CLR." ARE TO CENTER OF STEEL. CLEAR COVER FOR NON-PRESTRESSED CONCRETE REINFORCING SHALL BE AS NOTED BELOW UNO. ON PLANS OR DETAILS. CLEAR COVER FOR PRESTRESSED CONCRETE AND FOR PRECAST CONCRETE MANUFACTURED UNDER PLANT CONTROL CONDITIONS SHALL BE PER ACI 318, SECTIONS 1.1.2 AND 1.1.3, RESPECTIVELY.

EXPOSURE CONDITION:	COVER:
CAST AGAINST AND PERMANENTLY EXPOSED TO EARTH	3"
EXPOSED TO EARTH OR WEATHER (INCLUDES SLABS ON GRADE) NO. 5 AND SMALLER NO. 6 AND LARGER	1 1/2" 2"
NOT EXPOSED TO WEATHER OR IN CONTACT WITH GROUND	
STRUCTURAL SLABS, WALLS, JOISTS NO. 11 AND SMALLER NO. 14 AND LARGER	3/4" 1 1/2"
BEAMS, COLUMNS (PRIMARY REINFORCEMENT, TIES, STIRRUPS, SPIRALS)	1 1/2"

6. LAP SPLICES OF REINFORCING STEEL SHALL CONFORM TO TYPICAL REBAR LAP SCHEDULE UNO. NO TACK WELDING OF REINFORCING BARS ALLOWED. LATEST ACI CODE AND DETAILING MANUAL APPLY. AT WALLS AND FOOTINGS, PROVIDE BENT CORNER BARS TO MATCH AND LAP WITH HORIZ. BARS AT ALL CORNERS AND INTERSECTIONS UNO. VERT. WALL BARS SHALL BE SPLICED AT OR NEAR FLOOR LINES. SPLICE TOP BARS AT CENTER LINE OF SPAN AND BOTTOM BARS AT THE SUPPORT IN SPANDRELS, BEAMS, GRADE BEAMS, ETC. UNO. ON PLANS OR DETAILS.

7. MECHANICAL SPLICE COUPLERS SHALL HAVE CURRENT ICC APPROVAL AND SHALL BE CAPABLE OF DEVELOPING 125% OF THE SPLICED BARS' YIELD STRENGTH.

8. ALL REINFORCING SHALL BE BENT COLD. BARS SHALL NOT BE UN-BENT AND RE-BENT. FIELD BENDING OF REBAR SHALL NOT BE ALLOWED UNLESS SPECIFICALLY NOTED.

9. WELDING OF REINFORCING BARS, METAL INSERTS, AND CONNECTIONS SHALL BE MADE ONLY AT LOCATIONS SHOWN ON PLANS OR DETAILS. SEE WELDING SECTION OF G.S.N. FOR ADDITIONAL REQUIREMENTS.

10. REINFORCING BAR SPACINGS SHOWN ON PLANS ARE MAX. ON CENTER DIMENSIONS. DOUEL ALL VERT. REINFORCING TO FOUNDATION. SECURELY TIE ALL BARS IN LOCATION BEFORE PLACING CONCRETE. MIN. CLEAR SPACING BETWEEN PARALLEL REINFORCING BARS SHALL BE THE LARGER OF 1-1/2 TIMES NOMINAL BAR DIA. OR 1-1/3 TIMES MAX. AGGREGATE SIZE OR 1-1/2". CLEAR SPACING LIMITATION APPLIES ALSO TO CLEAR DISTANCE BETWEEN A CONTACT LAP SPLICE AND ADJACENT SPLICES OR BARS.

G. STRUCTURAL STEEL

1. STRUCTURAL STEEL MEMBERS SHALL CONFORM TO THE FOLLOWING STANDARDS AND MATERIAL PROPERTIES UNO.

SHAPE:	STANDARD:	F _y :
ROLLED WIDE FLANGE SECTIONS	ASTM A992	50 KSI
OTHER STANDARD STEEL SHAPES AND ROLLED SECTIONS	ASTM A36	36 KSI
BARS AND PLATES	ASTM A36	36 KSI
PIPES	OR	
HOLLOW STRUCT. SECTIONS (RECT.)	ASTM A572, GRADE 50 (WHERE NOTED)	50 KSI
HOLLOW STRUCT. SECTIONS (ROUND)	ASTM A572, GRADE 50	50 KSI
	ASTM A500, GRADE B	42 KSI

2. ALL STRUCTURAL AND MISC. STEEL SHALL BE FABRICATED AND ERRECTED IN ACCORDANCE WITH AISI 303 AND SHALL BE COMPLETED BY AN "APPROVED STEEL FABRICATOR". THE CITY OF LAS VEGAS RECOGNIZES APPROVED STEEL FABRICATORS THAT ARE LISTED/APPROVED THROUGH THE ICC, CITY OF LOS ANGELES OR OTHER NATIONALLY RECOGNIZED APPROVAL/LISTING AGENCY.

3. SHEAR CONNECTORS (SUCH AS HEADED STEEL STUDS, STEEL BARS, OR STEEL LUGS), REINFORCING BARS, DEFORMED ANCHORS, OR THREADED STUDS SHALL NOT BE ATTACHED TO THE TOP FLANGES OF BEAMS, JOISTS, OR BEAM ATTACHMENTS SO THAT THEY PROJECT VERTICALLY FROM OR HORIZONTALLY ACROSS THE TOP FLANGE OF THE MEMBER UNTIL AFTER THE METAL DECKING, OR OTHER WALKING/WORKING SURFACE, HAS BEEN INSTALLED.

4. SEE CONCRETE SECTION OF G.S.N. FOR GROUTING REQUIREMENTS BENEATH COLUMN BASES AND BEARING PLATES.

H. STRUCTURAL STEEL WELDING

1. ALL WELDING OF STRUCTURAL STEEL SHALL CONFORM TO AISC D11 AND FOLLOW THE PREQUALIFIED JOINT DETAILS INCLUDED THEREIN. WELDING OF JOINTS THAT INCLUDE REINFORCING STEEL SHALL CONFORM TO AISC D14.

2. WELDING SHALL BE PERFORMED BY WELDERS HOLDING VALID CERTIFICATES AND HAVING CURRENT EXPERIENCE IN THE TYPE OF WELD SHOWN ON THE DRAWINGS. CONTRACTOR MAY SHOP WELD OR FIELD WELD AT HIS DISCRETION. ALL COMPLETE JOINT PENETRATION (CJP) WELDS SHALL BE TESTED AND CERTIFIED BY AN INDEPENDENT TESTING LABORATORY.

3. ALL WELDING SHALL USE PREQUALIFIED MATCHING FILLER METALS PER AISC D11, TABLE 3.1, WITH A MIN. TENSILE STRENGTH OF 70 KSI UNO. WELDS BETWEEN REINFORCING BARS SHALL USE PREQUALIFIED MATCHING FILLER METALS PER AISC D14, TABLE 5.1, WITH A MIN. TENSILE STRENGTH OF 90 KSI UNO. (MIN. TENSILE STRENGTHS FOR FILLER METALS USED IN WELDS BETWEEN REINFORCING BARS AND STRUCTURAL STEEL MAY BE 70 KSI).

4. WELDING SHALL BE PERFORMED IN ACCORDANCE WITH A WELDING PROCEDURE SPECIFICATION (WPS) AS REQUIRED IN AISC D11. THE WPS VARIABLES SHALL BE WITHIN THE PARAMETERS ESTABLISHED BY THE FILLER METAL MANUFACTURER. THE WPS SHALL BE SUBMITTED TO THE OWNER'S TESTING AGENCY FOR REVIEW PRIOR TO FABRICATION AND ERECTION. COPIES OF THE WPS SHALL BE ON SITE AND AVAILABLE TO ALL WELDERS AND THE SPECIAL INSPECTOR.

5. WELD LENGTHS CALLED OUT ON PLANS OR DETAILS ARE MINIMUM NET EFFECTIVE LENGTHS UNO.

I. STRUCTURAL STEEL WELDING (CONT'D)

6. ALL MISC. FILLET WELDS NOT NOTED, INCLUDING THOSE FOR STIFFENERS, MISC. PLATES, ETC., SHALL BE PER AISI 360, TABLE J2.4.

7. WELDS SHALL BE SEQUENCED TO MINIMIZE RESIDUAL STRESS DUE TO WELD SHRINKAGE.

I. STRUCTURAL STEEL BOLTS, ANCHORS, HEADED STUDS

1. STRUCTURAL STEEL BOLTS, ANCHORS, ETC., SHALL CONFORM TO THE FOLLOWING STANDARDS AND MATERIAL PROPERTIES UNO:

COMPONENT:	STANDARD:	F _y :
BOLTS	ASTM A325 OR ASTM A490 WHERE NOTED	---
NUTS	ASTM A563	---
WASHERS	ASTM F436	---
ANCHOR RODS	ASTM F1554, GRADE 36 OR GRADE 55 WHERE NOTED (GRADE 55 RODS SHALL COMPLY WITH WELDABILITY SUPPLEMENT S1)	36 KSI 55 KSI
WASHERS (AT ANCHOR RODS)	ASTM A36 OR ASTM F844 (USE STANDARD) (F844 WASHERS PERMITTED ONLY FOR 3/4" DIA. RODS AT 1 1/8" MAX. DIA. HOLES IN BASE PLATE WHERE NO WELD REQ'D. BETWEEN WASHER AND BASE PLATE)	36 KSI

2. ALL BOLTS SHALL BE INSTALLED AS BEARING-TYPE CONNECTIONS WITH THREADS EXCLUDED FROM SHEAR PLANE (TYPE "X" CONNECTION) UNO. HIGH-STRENGTH BOLT ASSEMBLIES SHALL BE IN ACCORDANCE WITH THE RCSC "SPECIFICATION FOR STRUCTURAL JOINTS USING ASTM A325 OR A490 BOLTS" AND SHALL BE SNUG TIGHTENED USING ANY AISC APPROVED METHOD UNO. ALL BOLTS IN SLOTTED OR OVERSIZED HOLES AND ALL HIGH-STRENGTH BOLTS SHALL BE INSTALLED WITH HARDENED WASHERS.

3. ALL ANCHOR RODS AT STEEL COLUMN BASE PLATES SHALL BE RODS WITH THREADS BOTH ENDS WITH HEAVY HEX NUT FULLY THREADED ONTO EMBEDDED END. TACK WELD NUT TO ROD OR SPOIL THREADS TO PREVENT NUT FROM BACKING OFF. ANCHOR RODS SHALL NOT BE REPAIRED, REPLACED OR FIELD-MODIFIED WITHOUT WRITTEN APPROVAL FROM THE ENGINEER.

4. ALL CONNECTIONS INVOLVING WOOD MEMBERS, INCLUDING THOSE WITH THREADED ROD, THREADED STUDS, FOUNDATION ANCHOR BOLTS, THRU-BOLTS, ETC., SHALL USE ASTM A307 MATERIAL UNO. ALL BOLTS, ANCHOR BOLTS, EXPANSION BOLTS, ETC., SHALL BE INSTALLED WITH STEEL WASHERS AT FACE OF WOOD.

5. HEADED STUDS AND AUTOMATIC WELDED DOUELS SHOWN ON PLANS OR DETAILS SHALL BE BY NELSON STUD WELDING, INC. PER ICC ER-2614 AND ICC ER-5217, RESPECTIVELY. STUDS SHALL HAVE FLUXED ENDS AND BE AUTOMATICALLY END-WELDED WITH SUITABLE EQUIPMENT (NO FILLET WELDING OF STUDS PERMITTED UNO.) AT SPACINGS INDICATED ON THE PLANS OR DETAILS. WELDING OF STUDS SHALL CONFORM TO THE REQUIREMENTS OF AISC D11 AND AISC C54. HEADED STUDS AND AUTOMATIC WELDED DOUELS BY OTHER MANUFACTURERS MAY BE SUBSTITUTED PROVIDED THEY ARE OF EQUIVALENT CAPACITY FOR THE INTENDED APPLICATION AND HAVE CURRENT ICC APPROVAL.

6. ALL EXPANSION ANCHORS IN CONCRETE SHALL BE HILTI KWIK BOLT TZ (ICC ESR-1917) OR SIMILAR STRONG-BOLT (ICC ESR-1714) UNO. EXPANSION ANCHORS SHALL BE INSTALLED WITH SPECIAL INSPECTION. SUBSTITUTIONS SHALL ONLY BE PERMITTED WITH PRIOR WRITTEN APPROVAL OF THE ENGINEER THROUGH THE ARCHITECT.

7. ALL THREADED SCREW ANCHORS SHALL BE FOUER'S WEDGE BOLTS PER ICC ESR-1618 FOR MASONRY AND ICC ESR-2516 FOR CONCRETE. SIMPSON TITEN HD PER ICC ESR-1056 FOR MASONRY AND ICC ESR-2116 FOR CONCRETE. THREADED SCREW ANCHORS SHALL BE INSTALLED WITH SPECIAL INSPECTION. SUBSTITUTIONS SHALL ONLY BE PERMITTED WITH PRIOR WRITTEN APPROVAL OF THE ENGINEER THROUGH THE ARCHITECT.

8. ALL EPOXY ANCHORS IN CONCRETE (ALL-THREAD, REBAR, ETC.) SHALL USE HILTI HIT-RE 300-B3 (ICC ESR-2322) OR HILTI HIT-XP (ICC ESR-2508) UNO. ALL EPOXY ANCHORS IN MASONRY (ALL THREADED REBAR, ETC.) SHALL USE SIMPSON SET (ICC ESR-1172) UNO. INSTALL PER MFR'S SPEC'S. FOR EPOXY PRODUCTS, BASE MATERIAL TEMPERATURE SHALL BE NO LESS THAN 40 °F AT THE TIME OF APPLICATION.

I. STEEL DECK

1. STEEL DECK SHALL BE MANUFACTURED BY EITHER VERCO MANUFACTURING CO. PER ICC ER-2018P OR AISC STEEL DECK PER ICC ESR-1414. STEEL DECK BY OTHER MANUFACTURERS MAY BE USED ONLY WITH PRIOR WRITTEN APPROVAL OF THE ENGINEER THROUGH THE ARCHITECT. DECK STEEL SHALL BE GALVANIZED IF FIRE PROOFING IS TO BE APPLIED OR IF DECK WILL BE EXPOSED TO WEATHER. OTHERWISE, DECK STEEL MAY BE EITHER GALVANIZED OR PRIME PAINTED.

2. STEEL DECK TYPE, GAGE, ATTACHMENT TO SUPPORTING MEMBERS, AND SIDE BEAM CONNECTIONS SHALL BE AS NOTED ON PLAN. FLOOR DECKS SHALL BE COMPOSITE DECK UNO. DECK SHALL BE ERRECTED IN ACCORDANCE WITH MFR'S RECOMMENDATIONS.

3. DECK WITH CONCRETE TOPPING SHALL BE CONTINUOUS OVER 3 OR MORE SPANS UNO. (EXCEPTION: CONDITIONS ALLOWING FOR ONLY 2 CONTINUOUS SPANS OR LESS ARE ACCEPTABLE ONLY IF DECK IS SHORED UNTIL CONCRETE HAS REACHED DESIGN STRENGTH). DECK WITHOUT CONCRETE TOPPING SHALL BE CONTINUOUS OVER 2 OR MORE SPANS UNO. BEARING LENGTH AT SUPPORTS SHALL BE 3" MIN. FOR DECK 3" DEEP OR MORE AND 2" MIN. FOR ALL OTHER DECK. END LAPS AT SUPPORTS SHALL BE 2" MIN. (EXCEPTION: WHERE HEADED SHEAR STUDS ARE TO BE WELDED TO SUPPORTING BEAM, BUT DECK ENDS TOGETHER OVER SUPPORT IN LIEU OF END LAPPING TO ALLOW FOR SHEAR STUD WELDING THROUGH DECK).

4. ALL DECK WELDING SHALL COMPLY WITH AISC D13. USE E60 OR E70 SERIES ELECTRODES (MIN. ROD DIA. = 1/8"). ALL ARC SPOT (PUDDLE) WELDS SHALL HAVE AN EFFECTIVE FUSION AREA NOT LESS THAN 1/2" IN DIA. OR 3/8" x 1". ANY CRACK BEAM WELDS SHALL HAVE A FUSION AREA NOT LESS THAN 3/8" x 1 1/2". WELDS IN AREAS EXPOSED TO WEATHER SHALL BE DE-SLAGGED, AND PAINTED WITH A ZINC RICH PRIMER.

5. STEEL DECK SUPPLIER SHALL PROVIDE ALL NECESSARY ACCESSORIES SUCH AS ENDORS AND VALLEY PLATES, SADDLES, COVER PLATES, MID STOPS NOT SPECIFICALLY NOTED ON PLANS, ETC., TO MAKE THE JOB COMPLETE.

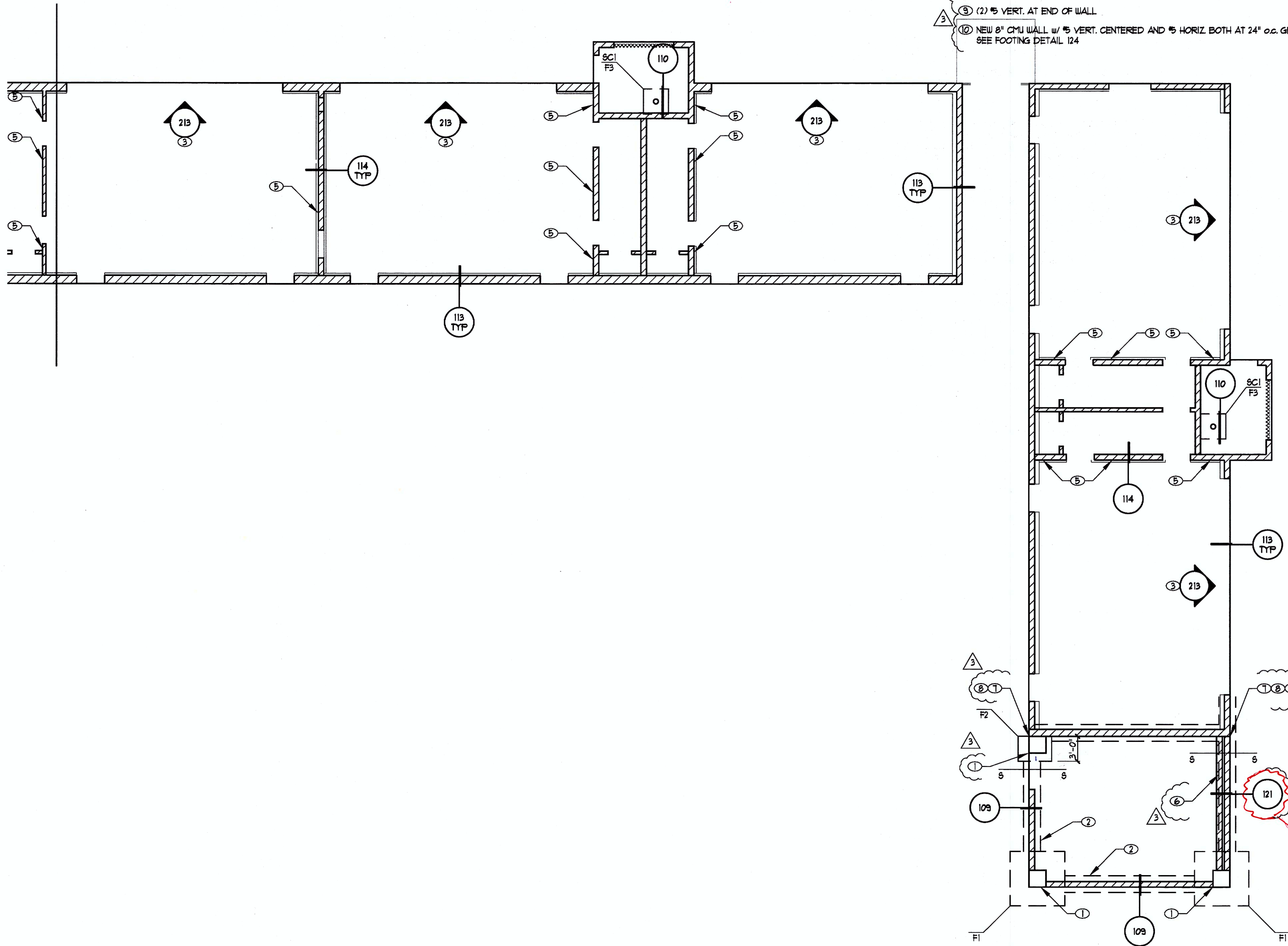
6. CONCENTRATED LOADS (i.e., CEILING AND BOFIT FRAMING, MECH. EQUIPMENT, PLUMBING, ETC.) SHALL NOT BE SUPPORTED FROM STEEL DECK. CONTRACTOR SHALL PROVIDE ENGINEERED FRAMING SYSTEM TO DISTRIBUTE CONCENTRATED LOADS TO STEEL JOISTS OR BEAMS.

7. PIPING AND ELECTRICAL CONDUITS SHALL NOT BE EMBEDDED IN CONCRETE OVER STEEL DECK.

STANDARD ABBREVIATIONS

AB.	ANCHOR BOLT
ACI	AMERICAN CONCRETE INSTITUTE
ACCS	ALL COMMON SURFACES
AISC	AMERICAN INSTITUTE OF STEEL CONSTRUCTION
AISI	AMERICAN IRON AND STEEL INSTITUTE
ALT.	ALTERNATE
ANSI	AMERICAN NATIONAL STANDARDS INSTITUTE
AR	ANCHOR ROD
ARCH'L	ARCHITECTURAL
ASTM	AMERICAN SOCIETY FOR TESTING AND MATERIALS
AUS	AMERICAN WELDING SOCIETY
B.F.	BOUNDARY FASTENERS
BFF	BELOW FINISH FLOOR
BJT	BOTTOM
BRS.	BEARING
CJT	CONTROL JOINT
CJP.	COMPLETE JOINT PENETRATION
CL.	CLEAR LINE
CLR.	CLEAR DIMENSION TO FACE OF REBAR
COLL.	COLUMN
CONT.	CONTINUOUS
CRSI	CONCRETE REINFORCING STEEL INSTITUTE
DIA.	DIAMETER
DIAG.	DIAGONAL
DWG.	DRAWING
EF.	EACH FACE
EL.	ELEVATION
ELECT.	ELECTRICAL
EOR.	ENGINEER OF RECORD
EQ.	EQUAL
EW.	EACH WAY
FF.	FINISH FLOOR
FLR.	FLOOR
FT.	FOOT
FTG.	FOOTING
GA.	GAUGE
GLB.	GLULAM BEAM
G.S.N.	GENERAL STRUCTURAL NOTES
GR.	GRIDER TRUSS
HORIZ.	HORIZONTAL
HSS	HOLLOW STRUCTURAL SECTION
IBC	INTERNATIONAL BUILDING CODE
ICC.	INTERNATIONAL CODE COUNCIL
ID.	INSIDE DIAMETER
INFO.	INFORMATION
JT.	JOINT
KIP.	KIP (1000 LBS)
K.O.	KNOCKOUT
KSI	KIPS PER SQUARE INCH
LLH	LONG LEG HORIZONTAL
LLV	LONG LEG VERTICAL
LVL.	LAMINATED STRAND LUMBER
LVL.	LAMINATED VENEER LUMBER
MFR.	MANUFACTURER
MAX.	MAXIMUM
MECH.	MECHANICAL
MIN.	MINIMUM
MISC.	MISCELLANEOUS
N/A.	NOT TO SCALE
O.C.	ON CENTER
OD.	OUTSIDE DIAMETER
OPP.	OPPOSITE
PL.	PL

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FOUNDATION PLAN
SCALE: 1/8" = 1'-0"

MARK	WIDTH	LENGTH	THICKNESS	FOOTING REBAR	NOTES
F1	6'-6"	6'-6"	1'-3"	(1) #5 E. W. BOTTOM	.
F2	3'-0"	4'-0"	1'-0"	(3) #4 E. W. BOTTOM	.
F3	3'-0"	3'-0"	1'-0"	(4) #4 E. W. BOTTOM	.
F4	2'-0"	CONT.	1'-0"	(3) #4 CONT. BOTTOM	.

FOUNDATION PLAN KEYNOTES

- MASONRY FILASTER PER DETAIL 101
- REMOVE (E) CONCRETE AND RECONSTRUCT W/ 5" CONCRETE. CONCRETE SLAB W/ #4 AT 16" O.C. EACH WAY
- CONTRACTOR TO PROVIDE UNIT PRICE IN BID FOR REPAIRING MISSING COLUMNS AT EXISTING WINDOWS.
- SAVE EXISTING COLUMNS AND COLUMN FOOTINGS. ENCASE EXISTING COLUMNS AND COLUMN BASE PLATE W/ 3" MIN. CONCRETE COVER
- GROUT EXISTING WALL SOLID
- NEW 12" CMU WALL PER DETAIL 121
- DOUCEL NEW FOOTING TO EXISTING FOOTING W/ (2) #5 EPOXY DOUELS PER G&N. 12" APART HORIZ. EMBED 6" MIN.
- PLACE BOTTOM OF NEW FOOTING AT SAME ELEVATION AS BOTTOM OF EXISTING FOOTING
- (2) #5 VERT. AT END OF WALL
- NEW 8" CMU WALL W/ #5 VERT. CENTERED AND #5 HORIZ. BOTH AT 24" O.C. GROUT SOLID. SEE FOOTING DETAIL 124

FOUNDATION PLAN NOTES

- VERIFY ALL DIMENSIONS, ELEVATIONS, SLOPES, ETC., W/ ARCH'L. AND/OR CIVIL PRIOR TO CONSTRUCTION. RESOLVE DISCREPANCIES WITH ARCHITECT.
- VERIFY LOCATION AND SIZE OF ALL INSERTS AND OPENINGS IN SLAB WITH ARCH'L., MECH., PLUMBING, AND ELECT. PRIOR TO CONSTRUCTION.
- ALL FOOTINGS SHALL EXTEND 12" INCHES MIN. BELOW PAD GRADE. PAD GRADE IS DEFINED AS TOP OF SLAB FOR INTERIOR FOOTINGS, LOWEST ADJACENT COMPACTED SUBGRADE (PAD GRADE BEFORE LANDSCAPING) OR NATURAL GRADE WITHIN 5 FEET OF BUILDING FOR PERIMETER FOOTINGS. OR TOP OF EXISTING PAVING OR CONCRETE WHERE EXISTING PAVING OR CONCRETE IS PERMANENTLY LOCATED DIRECTLY ADJACENT TO BUILDING AND EXTENDS AT LEAST 5 FEET FROM BUILDING. FOOTING EXCAVATIONS SHALL BE CLEAN AND FREE FROM LOOSE DEBRIS, STANDING WATER, OR UNCOMPACTED MATERIAL AT TIME OF CONCRETE PLACEMENT. CONTRACTOR SHALL COORDINATE FOOTING EMBEDMENT WITH CIVIL GRADING PLANS TO ENSURE THAT MINIMUM EMBEDMENT SPECIFIED ABOVE IS MAINTAINED.
- FINISH FLOOR ELEVATION VARIES. SEE CIVIL DUGS.
- F1, F2, F3, ... DENOTES FOOTING PER SCHEDULE THIS SHEET. TYPICAL FOOTING SHALL BE MARK "F1" UNO. ON PLAN.
- S _____ S DENOTES FOOTING STEP PER TYPICAL DETAIL 05.
- PLACE SLAB(S) ON GRADE OVER SUBGRADE AND VAPOR BARRIER (AS REQ'D.) PER THE GEOTECHNICAL REPORT REFERENCED ON G&N.
- REMOVE EXISTING INTERIOR SLAB ON GRADE. POUR NEW INTERIOR SLAB ON GRADE 5" THICK W/ #4 AT 16" O.C. EACH WAY. PLACE REIN. 2" CLR FROM TOP OF SLAB. AS AN ALTERNATE, REINFORCE SLAB W/ POLYPROPYLENE FIBERS PER G&N.
- INSTALL SLAB CONTROL JOINTS PER DETAIL 01. MATCH CLOSURE POUR (WHERE PRESENT) CONTROL JOINTS WITH SLAB CONTROL JOINTS.
- SEE DETAIL 02 FOR PIPES/CONDUIT AT FTGS. PIPES/CONDUIT SHALL NOT PASS THRU NOR BE LOCATED BENEATH ISOLATED PAD FOOTINGS.
- SEE DETAIL 101 FOR TRASH ENCLOSURE.
- SEE DETAILS 105 AND 108 FOR PERIMETER FENCE FILASTERS AND STEEL FENCE POST BETWEEN FILASTERS
- SEE DETAIL 112A FOR THE PLANTER WALLS. REFER TO ARCH'L DRAWINGS FOR LOCATIONS AND CONFIGURATIONS
- SEE DETAIL 115 FOR MONUMENT SIGN
- DETERIORATION OF THE CONCRETE STEM WALL AND MASONRY HAS OCCURRED JUST ABOVE THE SOIL LINE. SEE DETAIL 118 FOR REPAIRS. AT LOCATION WHERE REPAIRS HAVE ALREADY BEEN MADE, REMOVE THE REPAIR CLEAN AND REPAIR AREA PER DETAIL 118. CONTRACTOR TO OBSERVE ENTIRE BUILDING TO LOCATE AREAS WHICH REQUIRE REPAIR.

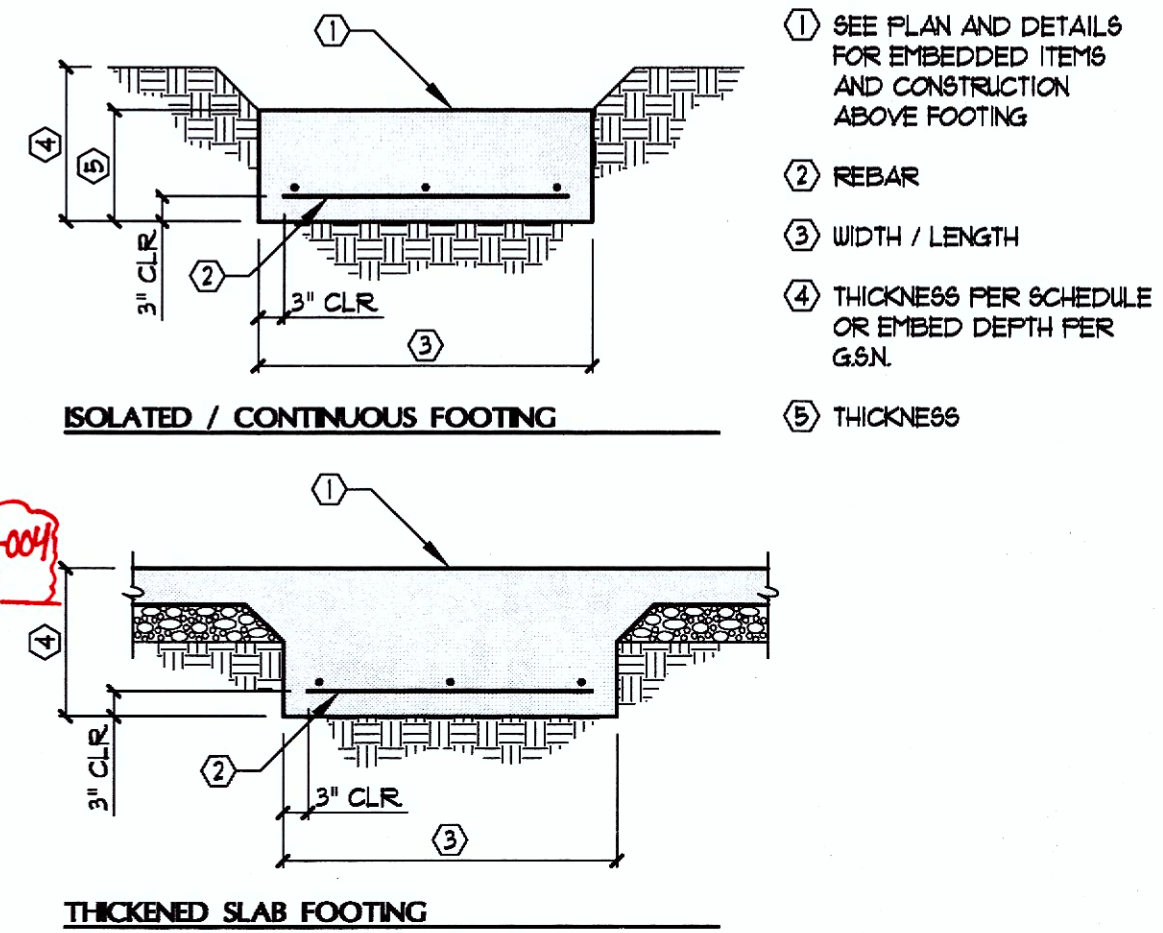
- F. PROVIDE FOOTINGS AT PLAZA TABLE AND CHAIRS AS FOLLOWS:
- FOOTING AT PLAZA TABLE - 3'-0" SQ. PAD x 10" THICK W/ (3) #4 EACH WAY BOTTOM, 3" CLR FROM BOTTOM OF FOOTING. MAX. DIA. OF TABLE LEG AT BASE OF LEG IS 24". TOP OF FOOTING 2" ABOVE FINISH SURFACE
 - PROVIDE A 14" WIDE x 10" THICK CURVED FOOTING FOR PLAZA TABLE CHAIRS W/ (2) #4 CONT. BOTTOM 3" CLR FROM BOTTOM OF FOOTING. CONTINUE FOOTING 8" MIN. PAST FACE OF CHAIR LEG. EACH END OF FOOTING. TOP OF FOOTING 2" ABOVE FINISH SURFACE
- G. SEE DETAIL 112A FOR FOOTINGS AT CONCRETE BENCHES SHOWN ON LANDSCAPE DRAWINGS DETAILS L16 AND L17 ON SHEET L3.02
- R. WHERE THE MASONRY WALL HAS BEEN INFILLED AND THERE IS A GAP BETWEEN THE INFILL AND THE MASONRY WALL, FILL THE GAP AS FOLLOWS:
- WHERE THE GAP IS A MIN. OF 1/8" OR GREATER, FILL THE GAP W/ SIKADUR 31 HI-MOD GEL. PLUS REPAIR MORTAR. SEE DETAIL 118 FOR ADDITIONAL INFORMATION.
 - WHERE THE GAP IS LESS THAN 1/8" FILL THE GAP W/ SIKADUR 31 HI-MOD GEL. SEE DETAIL 118 NOTES A, B AND D FOR INSTALLATION INSTRUCTION.

STEEL COLUMN (SC) SCHEDULE

- A. FOR BASE PLATE CONFIGURATION, AIR PATTERN, AND COLUMN-TO-BASE PLATE CONNECTION, SEE TYPICAL COLUMN BASE PLATE SCHEDULE OR DETAIL.
- B. STANDARD EFFECTIVE ANCHOR ROD EMBEDMENT = 1 1/2" UNO. EFFECTIVE EMBEDMENT SPECIFIED DENOTES MIN. DISTANCE REQ'D. BETWEEN T.O. CONCRETE AND T.O. EMBEDDED NUT UNO.

MARK	SIZE	BASE PLATE	NOTES
SC1	H85TxD500	3/4" x 12" x 1'-0" W/ (4) 3/4" DIA. AIR	.

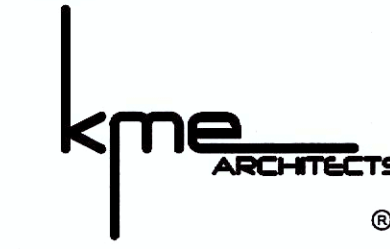
FOOTING (F) SCHEDULE



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A. VERIFY ALL DIMENSIONS, ELEVATIONS, SLOPES, ETC., w/ ARCH'L PRIOR TO CONSTRUCTION. RESOLVE DISCREPANCIES WITH ARCHITECT.

B. VERIFY LOCATION AND SIZE OF ALL INSERTS AND OPENINGS IN SLAB WITH ARCH'L, MECH, PLUMBING, AND ELECT. PRIOR TO CONSTRUCTION.

C. MISC. ITEMS SUCH AS STAIRS, LANDINGS, RAILINGS, MECH. EQUIPMENT ATTACHMENT TO PRIMARY STRUCTURAL FRAMING, ETC. SHALL BE DESIGNED BY OTHERS AND SUBMITTED FOR REVIEW AS DEFERRED SUBMITTALS PRIOR TO CONSTRUCTION. SEE 6.5N FOR DEFERRED SUBMITTAL REQUIREMENTS.

D. INDICATES APPROXIMATE LOCATION AND MAX. WEIGHT OF MECH'L UNITS. SEE MECH'L DRAWING FOR EXACT LOCATION

E. SISTER EXISTING 2x4 w/ NEW 2x4 #1 AT ALL LOCATIONS TO SUPPORT ANY NEW ITEMS SUCH AS SPRINKLES, MECH'L UNITS, MECH'L DUCTS OR ELEC'L. NAIL TOGETHER w/ 16d AT 12" o.c. STAGGERED TOP AND BOTTOM. BEAR ON EXISTING DECKER AT TRUSS

- ① NEW GLBS TO SUPPORT EXISTING FRAMING
- ② EXISTING ELEVATED CONCRETE SLAB
- ③ ADD BLOCKING BETWEEN JOIST PER DETAIL 212
- ④ 1/2" PLYWOOD AT CEILING FRAMING PER DETAIL 217
- ⑤ SEE DETAIL 220 FOR CONNECTIONS AT EXISTING ROOF TRUSSES
- ⑥ SEE DETAIL 220 FOR ADDED MEMBERS AT EXISTING ROOF FRAMING
- ⑦ (1) TRIMMER STUD PLUS (1) KING STUD
- ⑧ HUC HANGER
- ⑨ SEE DETAIL 221 FOR STRENGTHENING OF EXISTING TRUSSES
- ⑩ SISTER EXISTING 2x6's w/ NEW 2x6's 4' w/ 16d AT 12" o.c. STAGGERED TOP AND BOTTOM
- ⑪ SISTER EXISTING 2x6 RIDGE w/ NEW 2x6's 4' w/ 16d AT 12" o.c. TOP AND BOTTOM AND w/ A36 EACH END TO EXISTING TRUSS
- ⑫ EXISTING SUB-TRUSS AT 6'-6" o.c. FIELD VERIFY LOCATION
- ⑬ REPAIR BROKEN TRUSS PER DETAIL 222
- ⑭ ATTACH NEW MASONRY TO EXISTING ELEVATED CONCRETE SLAB PER DETAIL 223
- ⑮ NEW 6x6 POST w/ PC CAP AND ABUS66 POST BASE w/ 5/8" DIA. THREADED ROD EPOXY ANCHOR PER G.S.N. w/ 8" MIN. EMBEDMENT
- ⑯ REMOVE EXISTING POST AFTER INSTALLATION OF NEW POST

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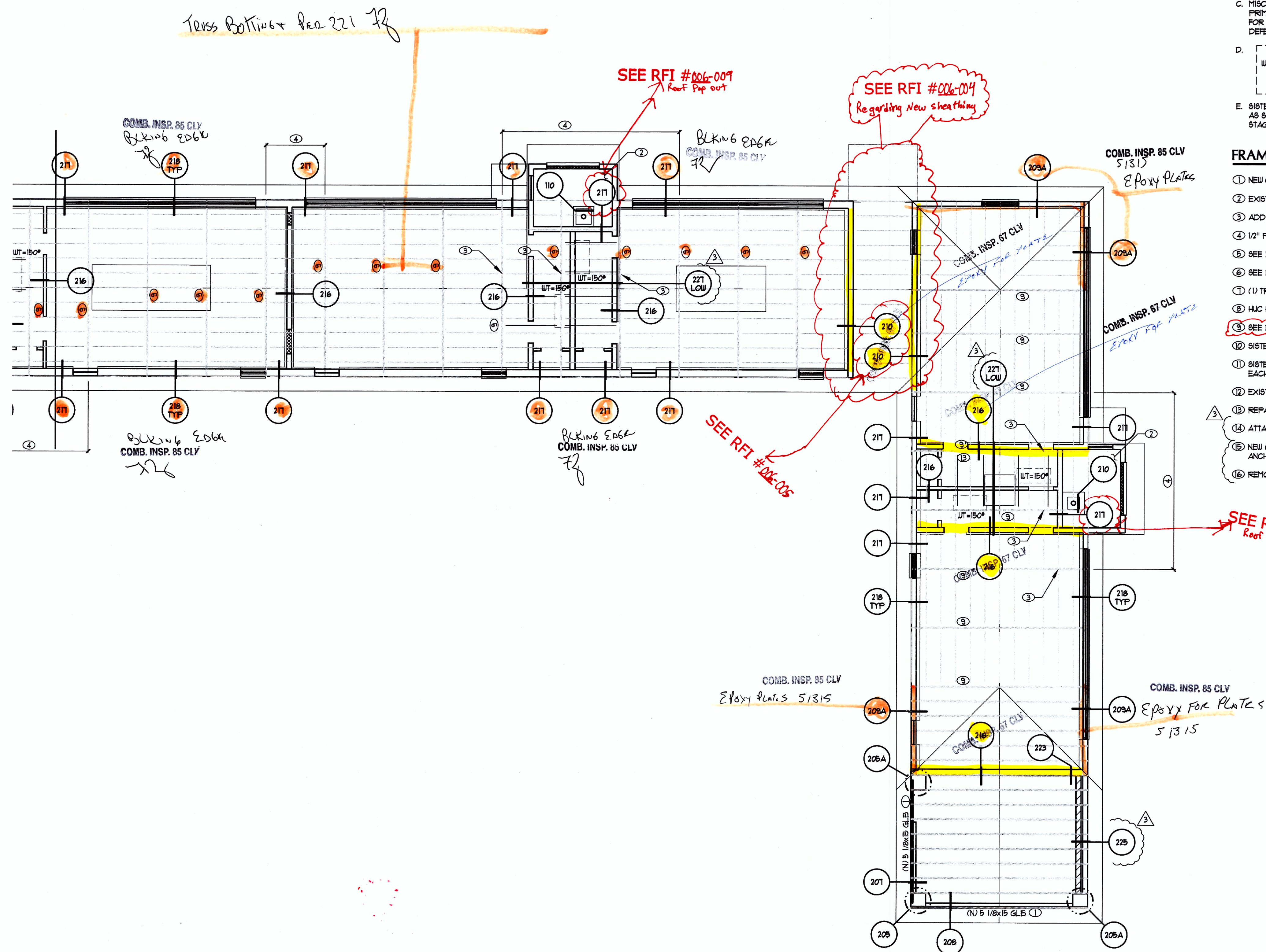
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ROOF FRAMING PLAN

SCALE: 1/8" = 1'-0"

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ROOF FRAMING PLAN
SCALE: 1/8" = 1'-0"

FRAMING PLAN NOTES

- VERIFY ALL DIMENSIONS, ELEVATIONS, SLOPES, ETC., w/ ARCH'L PRIOR TO CONSTRUCTION. RESOLVE DISCREPANCIES WITH ARCHITECT.
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- INDICATES APPROXIMATE LOCATION AND MAX. WEIGHT OF MECH'L UNITS. SEE MECH'L DRAWING FOR EXACT LOCATION.
- SISTER EXISTING 2x4 w/ NEW 2x4 #1 AT ALL LOCATIONS TO SUPPORT ANY NEW ITEMS SUCH AS SPRINKLES, MECH'L UNITS, MECH'L DUCTS OR ELECT'L. NAIL TOGETHER w/ 16d AT 12" o.c. STAGGERED TOP AND BOTTOM. BEAR ON EXISTING LEDGER AT TRUSSES.

FRAMING PLAN KEYNOTES

- NEW GLBS TO SUPPORT EXISTING FRAMING
- EXISTING ELEVATED CONCRETE SLAB
- ADD BLOCKING BETWEEN JOIST PER DETAIL 212
- 1/2" PLYWOOD AT CEILING FRAMING PER DETAIL 211
- SEE DETAIL 220 FOR CONNECTIONS AT EXISTING ROOF TRUSSES
- SEE DETAIL 220 FOR ADDED MEMBERS AT EXISTING ROOF FRAMING
- (1) TRIMMER STUD PLUS (1) KING STUD
- HIC HANGER
- SEE DETAIL 221 FOR STRENGTHENING OF EXISTING TRUSSES
- SISTER EXISTING 2x6's w/ NEW 2x6's #1 w/ 16d AT 12" o.c. STAGGERED TOP AND BOTTOM
- SISTER EXISTING 2x6 RIDGE w/ NEW 2x6 #1 w/ 16d AT 12" o.c. TOP AND BOTTOM AND w/ A35 EACH END TO EXISTING TRUSSES
- EXISTING SUB-TRUSSES AT 6'-6" o.c. FIELD VERIFY LOCATION
- REPAIR BROKEN TRUSSES PER DETAIL 222
- ATTACH NEW MASONRY TO EXISTING ELEVATED CONCRETE SLAB PER DETAIL 223
- NEW 6x6 POST w/ PC CAP AND ABUS66 POST BASE w/ 5/8" DIA. THREADED ROD EPOXY ANCHOR PER G.S.N. w/ 8" MIN. EMBEDMENT
- REMOVE EXISTING POST AFTER INSTALLATION OF NEW POST

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C. MISC. ITEMS SUCH AS STAIRS, LANDINGS, RAILINGS, MECH. EQUIPMENT ATTACHMENT TO PRIMARY STRUCTURAL FRAMING, ETC., SHALL BE DESIGNED BY OTHERS AND SUBMITTED FOR REVIEW AS DEFERRED SUBMITTALS PRIOR TO CONSTRUCTION. SEE 6.5N FOR DEFERRED SUBMITTAL REQUIREMENTS.

D. INDICATES APPROXIMATE LOCATION AND MAX. WEIGHT OF MECH'L UNITS. SEE MECH'L DRAWING FOR EXACT LOCATION

E. SISTER EXISTING 2x4 w/ NEW 2x4 #1 AT ALL LOCATIONS TO SUPPORT ANY NEW ITEMS SUCH AS SPRINKLES, MECH'L UNITS, MECH'L DUCTS OR ELEC'L. NAIL TOGETHER w/ 16d AT 12" o.c. STAGGERED TOP AND BOTTOM. BEAR ON EXISTING LEDGER AT TRUSS

- ① NEW GLBS TO SUPPORT EXISTING FRAMING
- ② EXISTING ELEVATED CONCRETE SLAB
- ③ ADD BLOCKING BETWEEN JOIST PER DETAIL 212
- ④ 1/2" PLYWOOD AT CEILING FRAMING PER DETAIL 211
- ⑤ SEE DETAIL 220 FOR CONNECTIONS AT EXISTING ROOF TRUSSES
- ⑥ SEE DETAIL 220 FOR ADDED MEMBERS AT EXISTING ROOF FRAMING
- ⑦ (1) TRIMMER STUD FLUB (1) KING STUD
- ⑧ HUC HANGER
- ⑨ SEE DETAIL 221 FOR STRENGTHENING OF EXISTING TRUSSES
- ⑩ SISTER EXISTING 2x6's w/ NEW 2x6's @ 1 w/ 16d AT 12" o.c. STAGGERED TOP AND BOTTOM
- ⑪ SISTER EXISTING 2x6 RIDGE w/ NEW 2x6 @ 1 w/ 16d AT 12" o.c. TOP AND BOTTOM AND w/ A36 EACH END TO EXISTING TRUSS
- ⑫ EXISTING SUB-TRUSS AT 6'-6" o.c. FIELD VERIFY LOCATION
- ⑬ REPAIR BROKEN TRUSS PER DETAIL 222
- ⑭ ATTACH NEW MASONRY TO EXISTING ELEVATED CONCRETE SLAB PER DETAIL 223
- ⑮ NEW 6x6 POST w/ PC CAF AND A8U66 POST BASE w/ 5/8" DIA. THREADED ROD EPOXY ANCHOR PER G.S.N. w/ 8" MIN. EMBEDMENT
- ⑯ REMOVE EXISTING POST AFTER INSTALLATION OF NEW POST

[illegible]

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ROOF FRAMING PLAN - PLAN 1923

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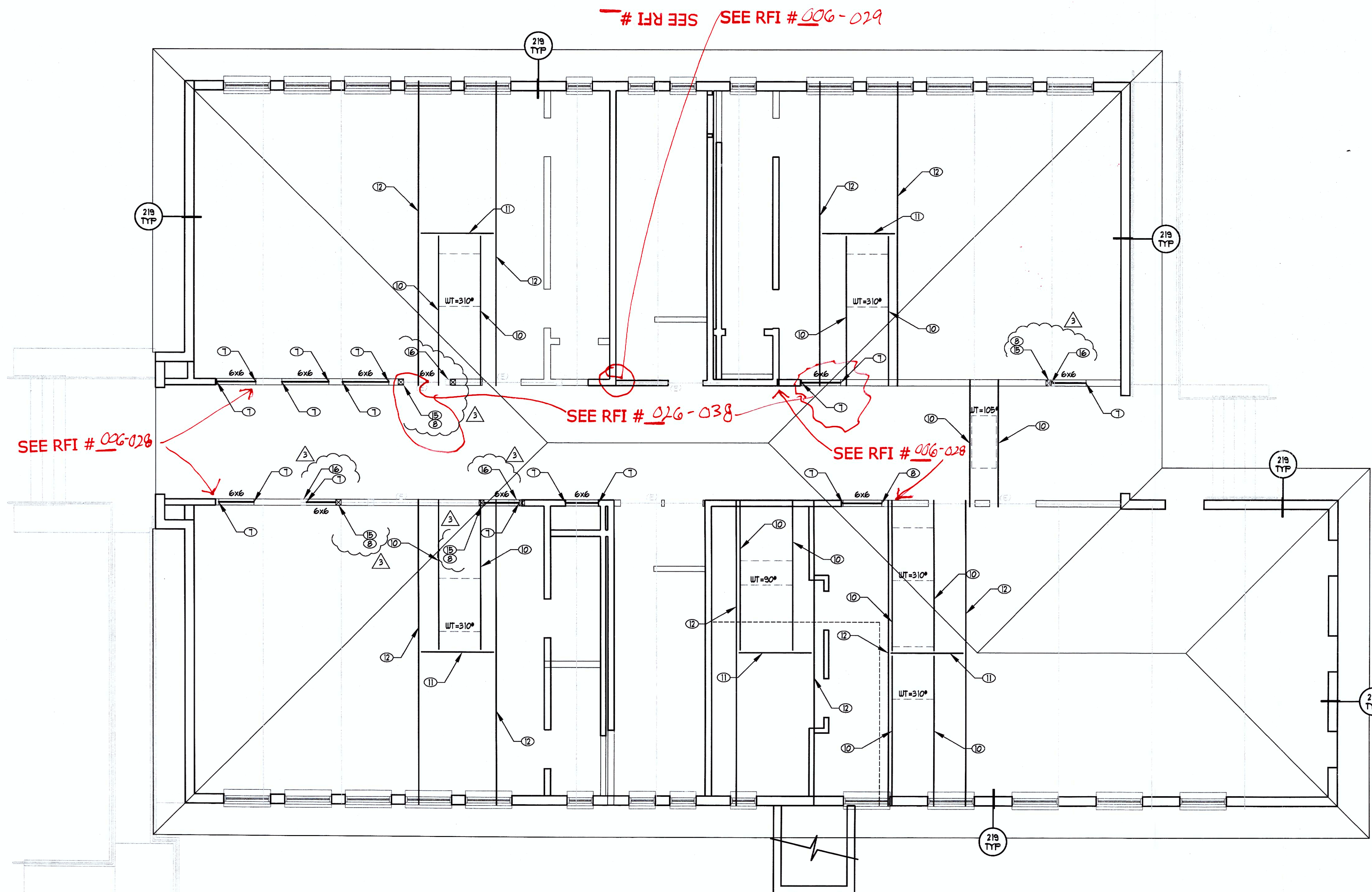
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DATE	08-11-2014
SCALE	NTS
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SHT NO	

S2.3

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SEE RFI # 026-025

SEE RFI # 026-024 - Ceiling Access Panels

SEE RFI # 006-026 - Sister framing for Fire Sprinkler

— ROOF FRAMING PLAN.

SCALE: 3/16" = 1'-0"

\\wse\dfs\projects\2011\112338\DWG\Structural\23-112338-PR1.dwg 11/18/2014 5:22:23 PM VAC



SEE RFI #032-007

- 101 TRASH ENCLOSURE
101-112338



- ① SLOPE PER ARCH.
- ② TYPICAL ROOF DECK SHALL BE 1 1/2" x 20 G.A. B-36 DECK PER G.S.N. UNO. DECK ATTACHMENT TO ROOF FRAMING, INCLUDING DETAILS OF OPENINGS AND DRAG/COLLECTOR ELEMENTS NOTED ON PLAN, SHALL BE AS FOLLOWS UNO. :
 - SUPPORTS PERPEND. TO DECK SPAN:
 - SUPPORTS PARALLEL TO DECK SPAN:
 - SIDE BEAM CONNECTIONS:
- ③ H864x4x1/2 COLUMN

① GATE POST AND POST FOOTING B
OTHERS

REVISIONS

ARCHITECT

kme
ARCHITECTS

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CONSULTA



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engineers

1635 VILLAGE CENTER CIRCLE, SUITE 200
LAS VEGAS, NEVADA 89134
p 702.933.7090 f 702.933.7091 800.933.7611
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FILE 2012-007

DATE 08-11-2014

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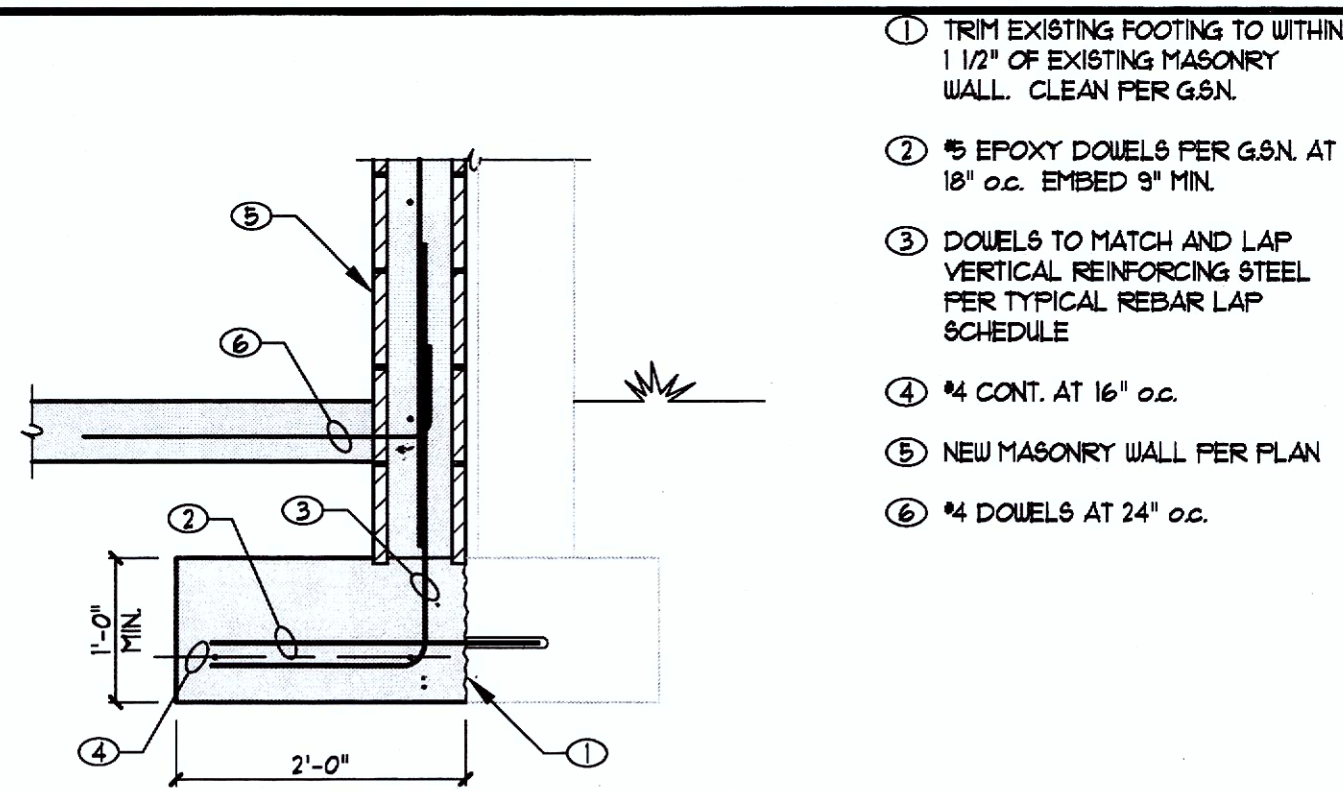
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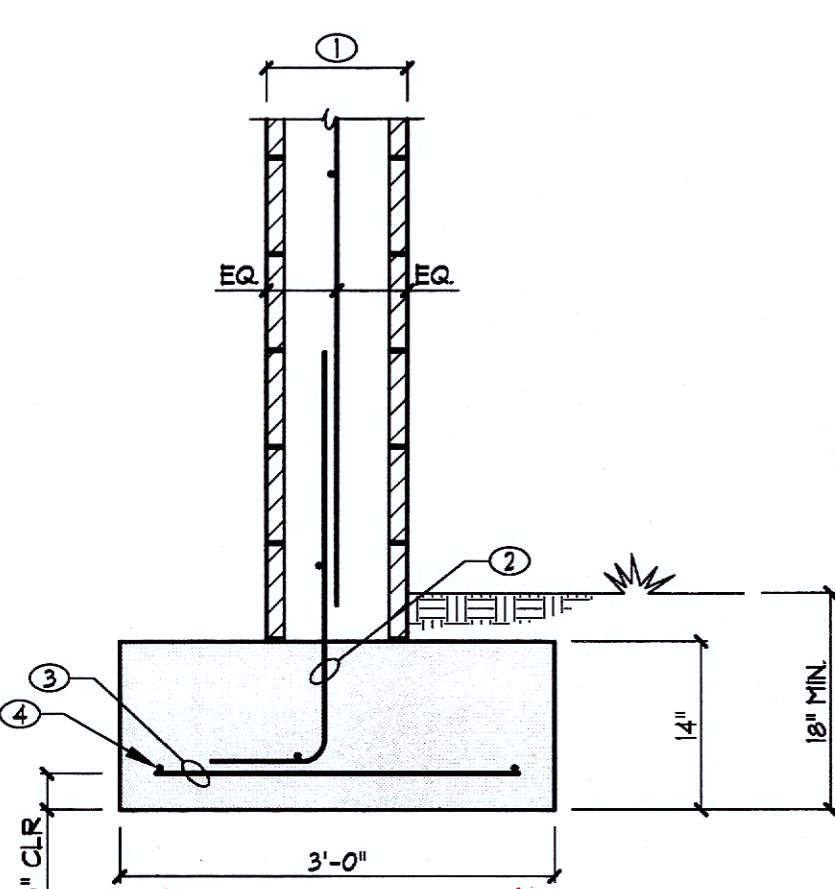
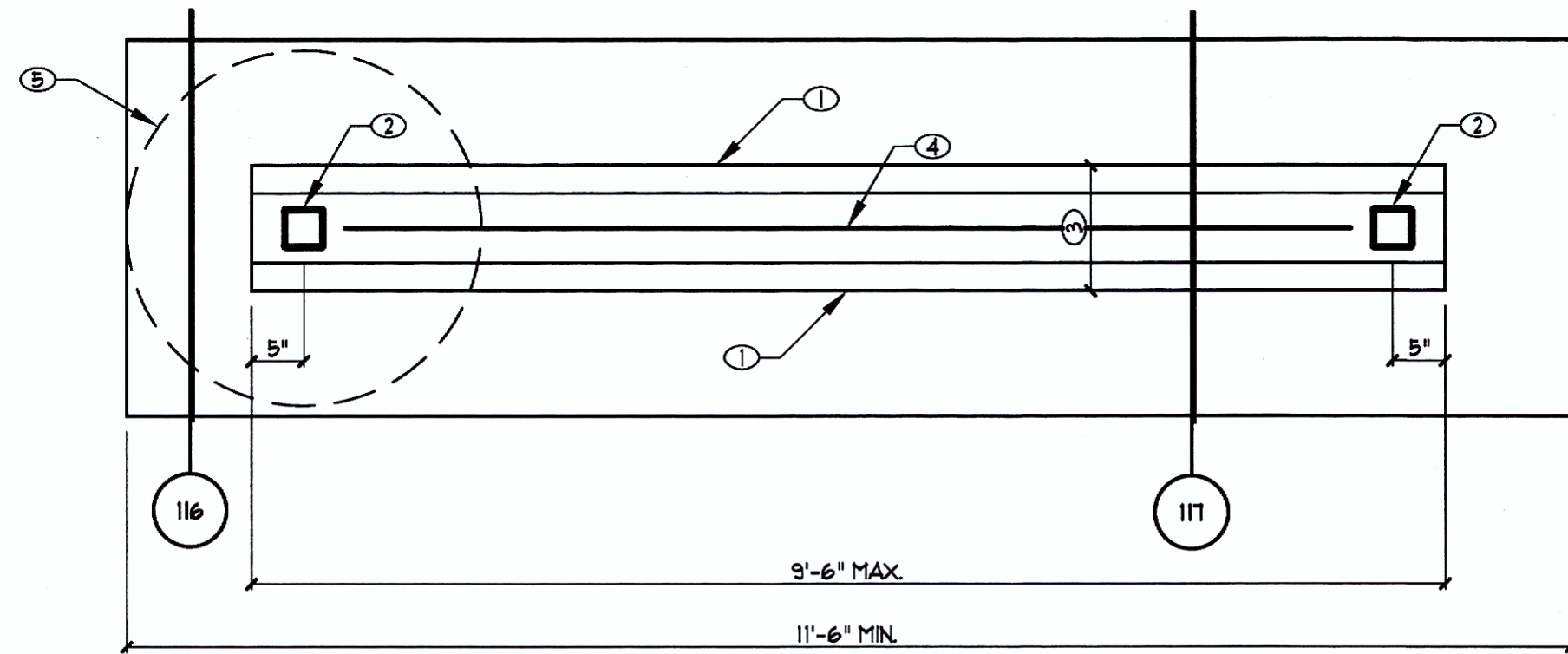
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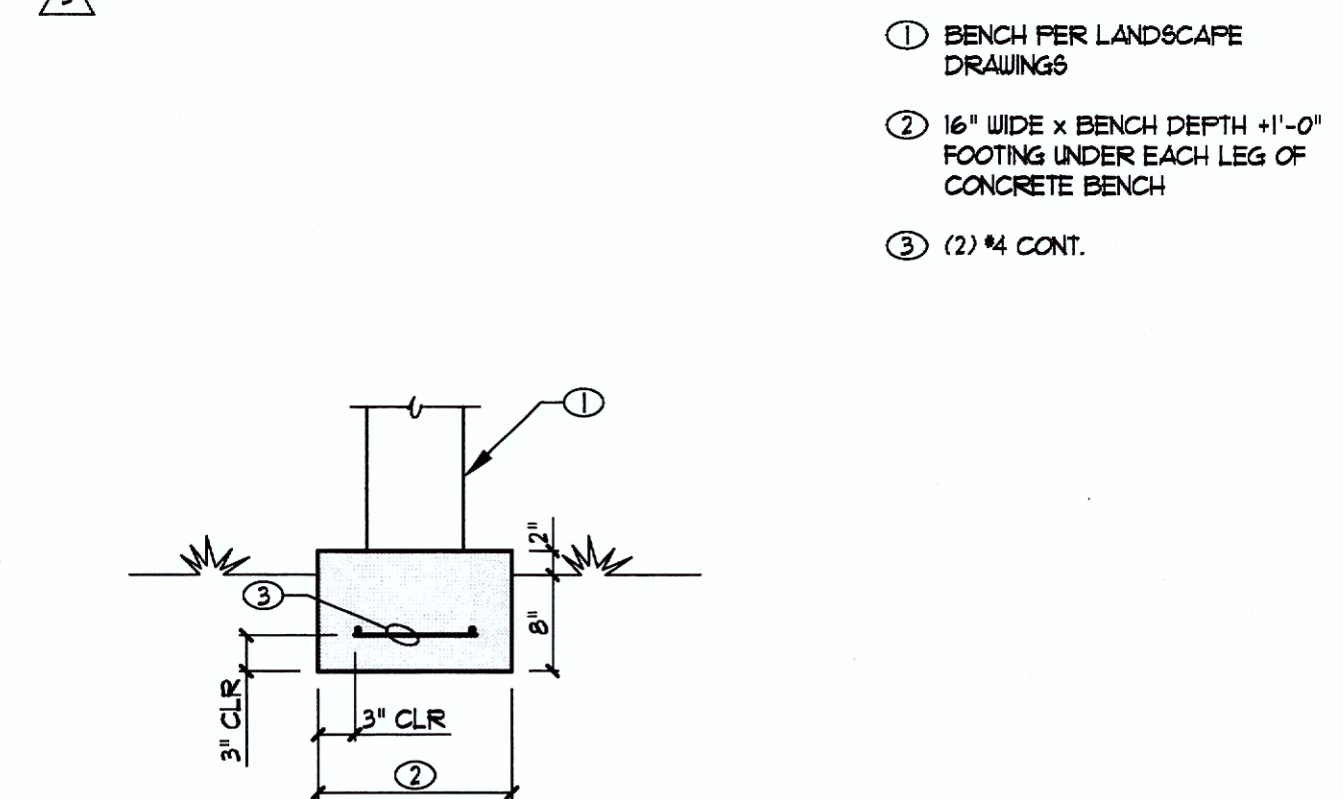
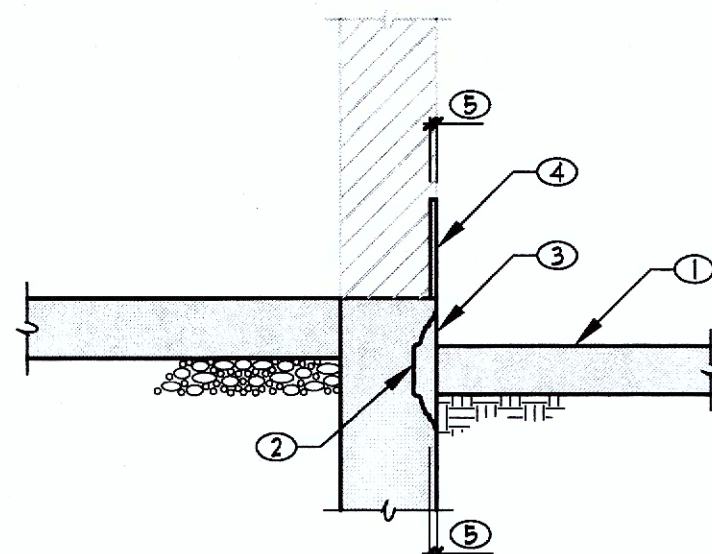
120 MASONRY WALL AT FOOTING
120-112338 NO SCALE

115 MONUMENT SIGN PLAN
115-112338



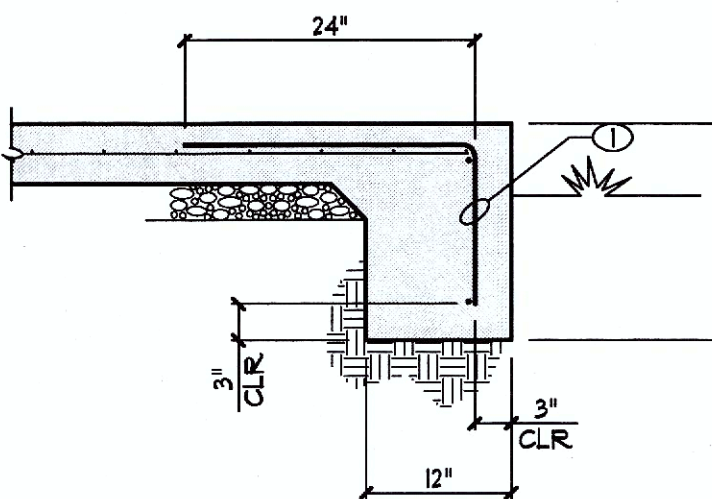
121 MASONRY WALL AT FOOTING
121-112338 NO SCALE

116 METAL STUDS AT FOOTING
116-112338



122 FOOTING AT CONCRETE BENCH
122-112338 NO SCALE

117 METAL STUDS AT FOOTING
117-112338



118 REPAIR EXPOSED FOUNDATION REINFORCING
118-112338

119 CONCRETE/SLAB EDGE
119-112338 NO SCALE

123 CONCRETE SLAB AT CONCRETE STEM
123-112338



124 CONCRETE SLAB AT CONCRETE STEM
124-112338



125 RETAINING WALL
125-112338



126 PLANTER WALL
126-112338



127 SITE RETAINING WALL
127-112338



128 RETAINING WALL
128-112338



129 RETAINING WALL
129-112338



130 RETAINING WALL
130-112338



131 RETAINING WALL
131-112338



132 RETAINING WALL
132-112338



133 CONCRETE SLAB AT CONCRETE STEM
133-112338



134 CONCRETE SLAB AT CONCRETE STEM
134-112338



135 RETAINING WALL
135-112338



136 RETAINING WALL
136-112338



137 RETAINING WALL
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138 RETAINING WALL
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139 RETAINING WALL
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140 RETAINING WALL
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141 RETAINING WALL
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142 RETAINING WALL
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143 CONCRETE SLAB AT CONCRETE STEM
143-112338



144 CONCRETE SLAB AT CONCRETE STEM
144-112338



145 RETAINING WALL
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146 RETAINING WALL
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147 RETAINING WALL
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148 RETAINING WALL
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149 RETAINING WALL
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150 RETAINING WALL
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151 RETAINING WALL
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152 RETAINING WALL
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153 CONCRETE SLAB AT CONCRETE STEM
153-112338



154 CONCRETE SLAB AT CONCRETE STEM
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155 RETAINING WALL
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156 RETAINING WALL
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161 RETAINING WALL
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162 RETAINING WALL
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163 CONCRETE SLAB AT CONCRETE STEM
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164 CONCRETE SLAB AT CONCRETE STEM
164-112338



165 RETAINING WALL
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167 RETAINING WALL
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168 RETAINING WALL
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170 RETAINING WALL
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171 CONCRETE SLAB AT CONCRETE STEM
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172 CONCRETE SLAB AT CONCRETE STEM
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173 RETAINING WALL
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175 RETAINING WALL
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176 RETAINING WALL
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177 CONCRETE SLAB AT CONCRETE STEM
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178 CONCRETE SLAB AT CONCRETE STEM
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179 RETAINING WALL
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182 RETAINING WALL
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183 CONCRETE SLAB AT CONCRETE STEM
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184 CONCRETE SLAB AT CONCRETE STEM
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191 CONCRETE SLAB AT CONCRETE STEM
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192 CONCRETE SLAB AT CONCRETE STEM
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193 RETAINING WALL
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197 CONCRETE SLAB AT CONCRETE STEM
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198 CONCRETE SLAB AT CONCRETE STEM
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199 RETAINING WALL
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201 CONCRETE SLAB AT CONCRETE STEM
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202 CONCRETE SLAB AT CONCRETE STEM
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203 RETAINING WALL
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204 RETAINING WALL
204-112338



205 RETAINING WALL
205-112338



206 CONCRETE SLAB AT CONCRETE STEM
206-112338

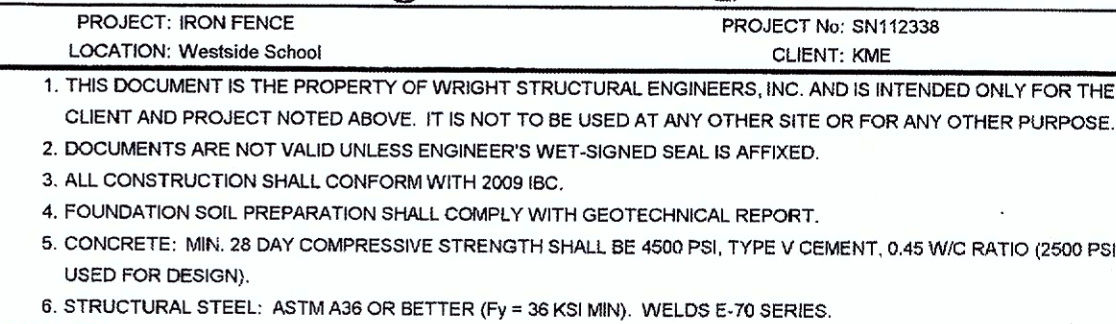


207 CONCRETE SLAB AT CONCRETE STEM
207-112338



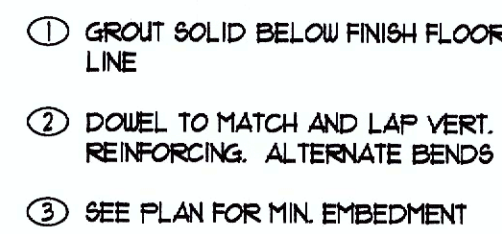
208 RETAINING WALL
208-112338





NO SCALE

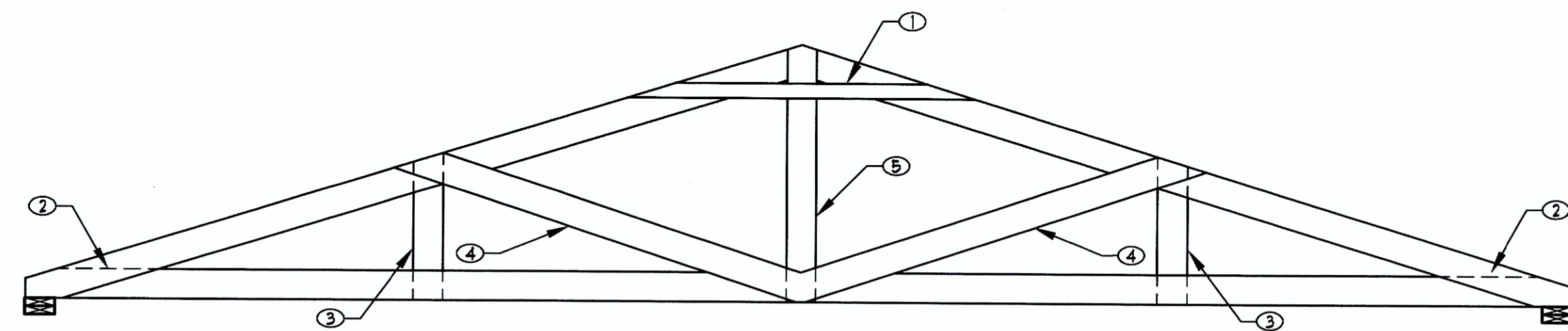
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NO SCALE

S3.4

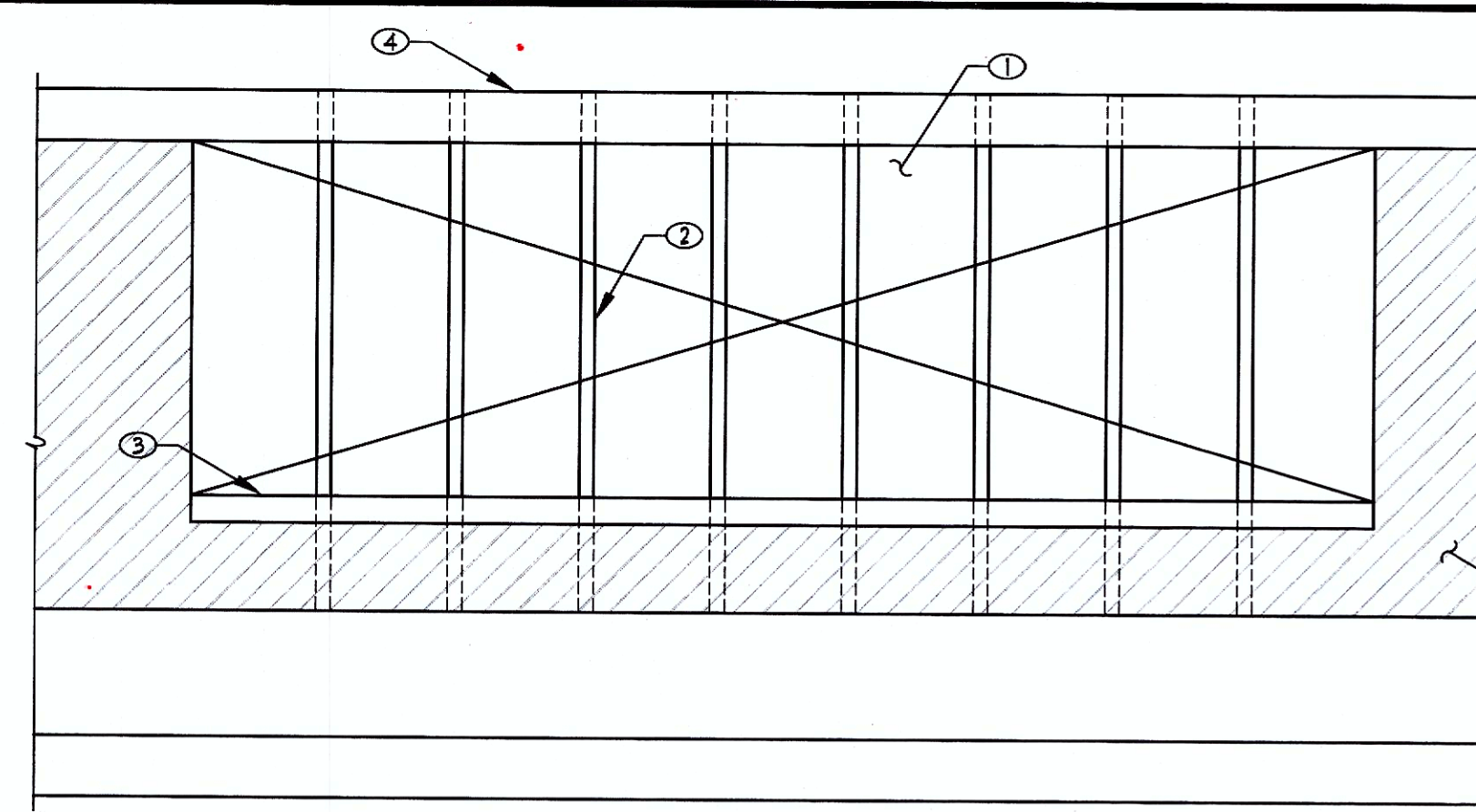
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220 EXISTING ROOF TRUSS
220-112338

- SEE RFI #006-013
- 1 NEW COLLAR TIES AT EACH TRUSS w/ (4) 16d EACH TRUSS
 - 2 ADD (8) 1/4" x 3" SD6 SCREWS AT BOTTOM CHORD TO TOP CHORD CONNECTION
 - 3 VERIFY OR ADD AT ADDED MEMBER (2) 16d EACH END
 - 4 VERIFY OR ADD AT ADDED MEMBER (6) 16d EACH END
 - 5 VERIFY OR ADD AT ADDED MEMBER (5) 16d EACH END
- SEE RFI #006-010
+ gussets

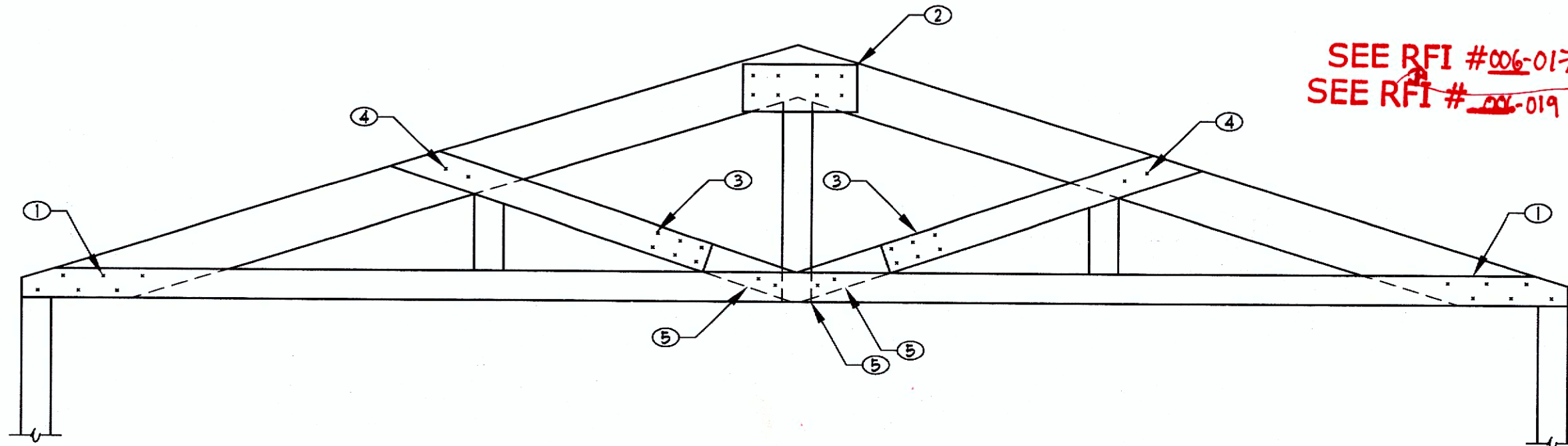
- A. TRUSSES SPACED AT 2'-6" o.c. MAX. FIELD VERIFY
- B. PROVIDE VERT. AND HORIZ. MEMBERS AT ALL EXISTING TRUSSES WHERE THEY DO NOT OCCUR
- C. ALL NEW MEMBERS TO BE 2x6 #1 MIN.



213 WOOD BLOCKING AT WOOD TRUSS
213-112338

- 1 REMOVE EXISTING MASONRY FROM ENCLOSED WINDOW. SAVE REMOVED MASONRY FOR FUTURE USE
- 2 DO NOT CUT EXISTING LINTEL SUPPORT COLUMN. COLUMN SPACED 4'-44" o.c. FIELD VERIFY LOCATIONS PRIOR TO REMOVING MASONRY
- 3 CONCRETE SILL
- 4 CONCRETE BOND BEAM. FIELD VERIFY SIZE
- 5 EXISTING MASONRY

- A. WHERE EXISTING COLUMNS HAVE BEEN REMOVED INSTALL NEW COLUMNS PER DETAIL 214
- B. IF EXISTING COLUMN IS NOTCH DURING REMOVAL OF MASONRY, REPAIR BY FILLING NOTCH WITH WELD. IF NOTCH HAS CUT COMPLETELY THROUGH COLUMN WALL, CONTACT ENGINEER FOR REPAIR

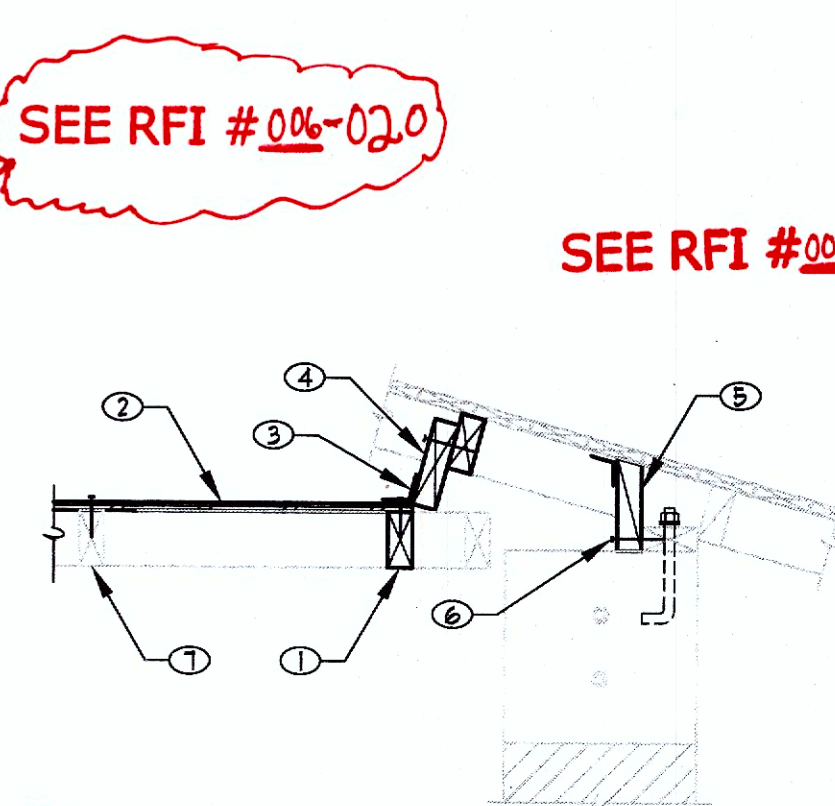


221 EXISTING ROOF TRUSS
221-112338

- SEE RFI #006-017
SEE RFI #006-019

- 1 ADD (3) 5/8" DIA. BOLTS THRU BOTTOM AND TOP CHORDS
- 2 ADD (2) 5/8" DIA. BOLTS THRU TOP CHORD
- 3 ADD (6) 1/4" x 3" SD6 SCREWS EACH SIDE OF EXISTING 2x. PLACE BOLTS WITHIN 2" OF END OF 1x. STAGGER BOLTS 1" ON EACH SIDE OF 2x
- 4 ADD 5/8" DIA. BOLT THRU DIAGONAL AND TOP CHORD
- 5 ADD (4) 1/4" x 2" SD6 SCREWS EACH SIDE OF BOTTOM CHORD AT BOTH DIAGONAL AND AT VERTICAL. ALIGN SCREWS HORIZONTALLY IN (2) ROWS MAX ALONG BOTTOM CHORD

- A. VERIFY EXISTING THRU BOLTS ARE 5/8" MIN.

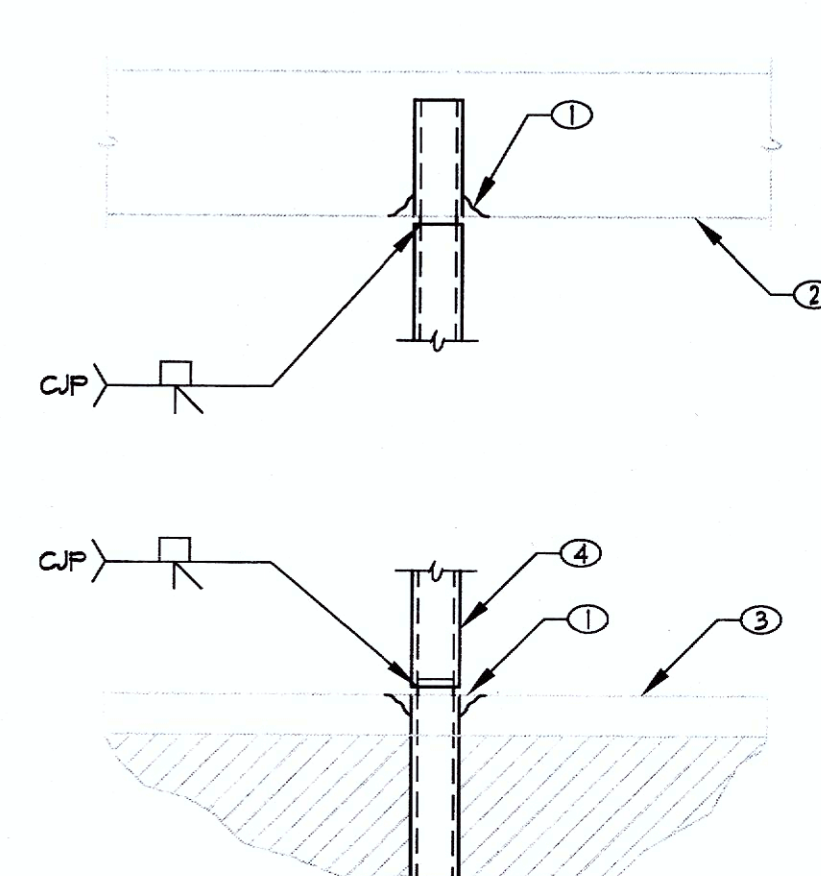


217 WOOD JOIST AT MASONRY WALL
217-112338

- SEE RFI #006-014

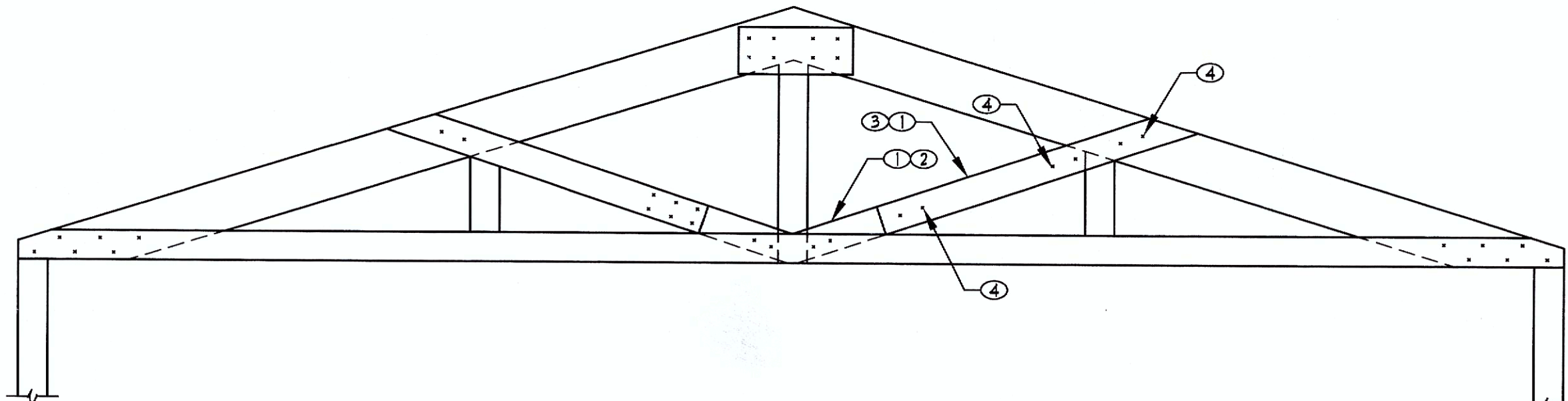
- 1 2x4 w/ Bd at 4" o.c. TO SHEATHING
- 2 1/2" SHEATHING PER DETAIL 216
- 3 LB90 AT 24" o.c.
- 4 2x CONT. w/ 16d AT 4" o.c. TO EXISTING RAFTER
- 5 2x SHAPED SOLID BLOCKING w/ LB90 AT 12" o.c. w/ NAILS AND SCREWS PER DETAIL 201
- 6 16d AT 4" o.c.
- 7 ADD CEILING JOIST WHERE NO JOIST EXIST 2x4 AT 16" o.c.

- SEE RFI #006-009



214 STEEL COLUMN REPAIR
214-112338

- 1 CHIP CONCRETE BACK TO ALLOW FOR WELDING NEW COLUMN SECTION TO EXISTING COLUMN SECTION. PATCH w/ GROUT AFTER INSTALLATION OF COLUMN
- 2 EXISTING CONCRETE BOND BEAM
- 3 EXISTING CONCRETE SILL OVER MASONRY WALL
- 4 NEW COLUMN SECTION TO MATCH EXISTING COLUMN SECTION. FIELD VERIFY COLUMN SIZE



222 EXISTING ROOF TRUSS REPAIR
222-112338

- 1 REMOVE EXISTING 1x DIAGONAL AT TOP CHORD AND 2x6 DIAGONAL AT BOTTOM CHORD
- 2 NEW 2x6 #1 DIAGONAL BOLTED TO BOTTOM CHORD PER DETAIL 221
- 3 NEW 2x6 #1 DIAGONAL EACH SIDE OF TOP CHORD AND NEW DIAGONAL
- 4 (2) 5/8" DIA. BOLTS AT 3" APART w/ 1 1/4" MIN. EDGE DISTANCE

- A. SHORE ROOF FRAMING PRIOR TO STARTING ANY TRUSS REPAIR
- B. SEE DETAIL 221 FOR STRENGTHENING OF EXISTING CONNECTION

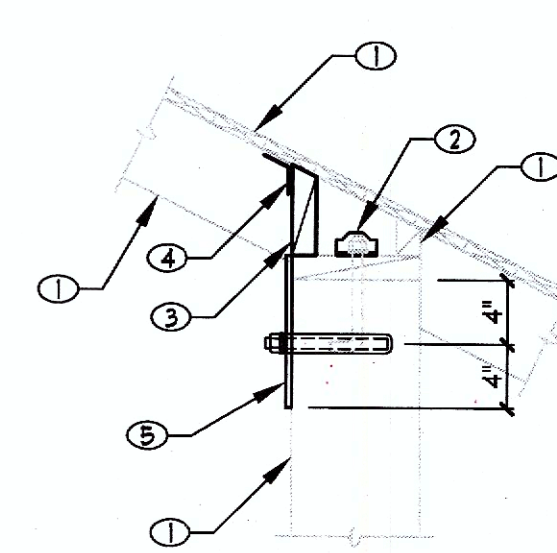
218 WOOD BLOCKING AT MASONRY WALL
218-112338

- SEE RFI #006-016

- 1 2x SHAPED SOLID BLOCKING w/ LB90 AT 24" o.c. w/ NAILS AND SCREWS PER DETAIL 201
- 2 16d AT 4" o.c.

- SEE RFI #006-023

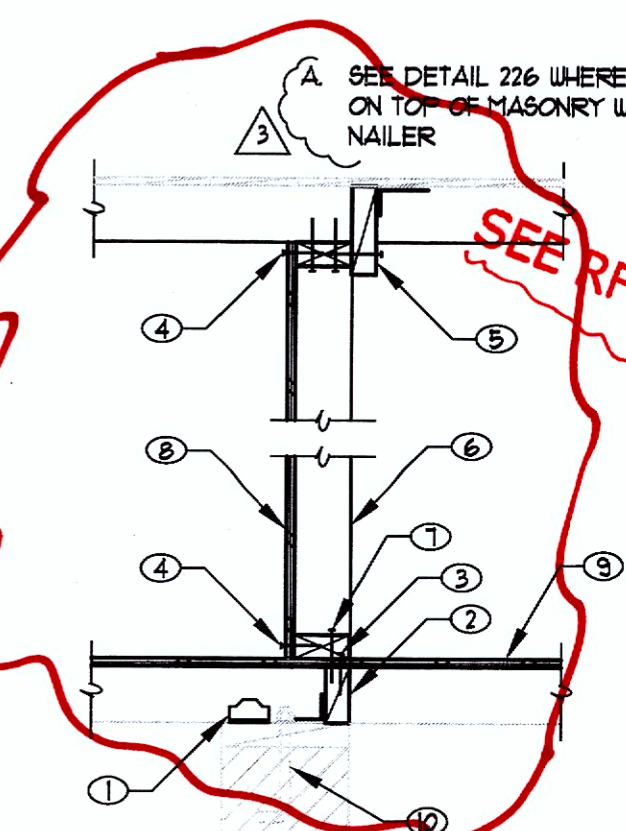
219 WOOD JOIST AT CONCRETE WALL
219-112338



215 SOLID BLOCKING AT MASONRY WALL
215-112338

- 1 EXISTING FRAMING
- 2 A35 EACH RAFTER w/ (6) #3 x 1 1/2" NAILER
- 3 2x SHAPED SOLID BLOCKING w/ LTP4 TO EXISTING NAILER
- 4 LB90 AT 16" o.c. w/ SCREWS TO ROOF SHEATHING PER DETAIL 201
- 5 3/8" x 11" PLATE AT 16" o.c. w/ (2) 5/8" DIA. SIMPSON EPOXY ANCHORS PER G&N. SPACED AT 8" APART. EMBED 5 1/2". PRE-DRILL PLATE FOR (3) #3 x 2 1/2" SIMPSON SCREWS SPACED 4" o.c.

- SEE RFI #006-022

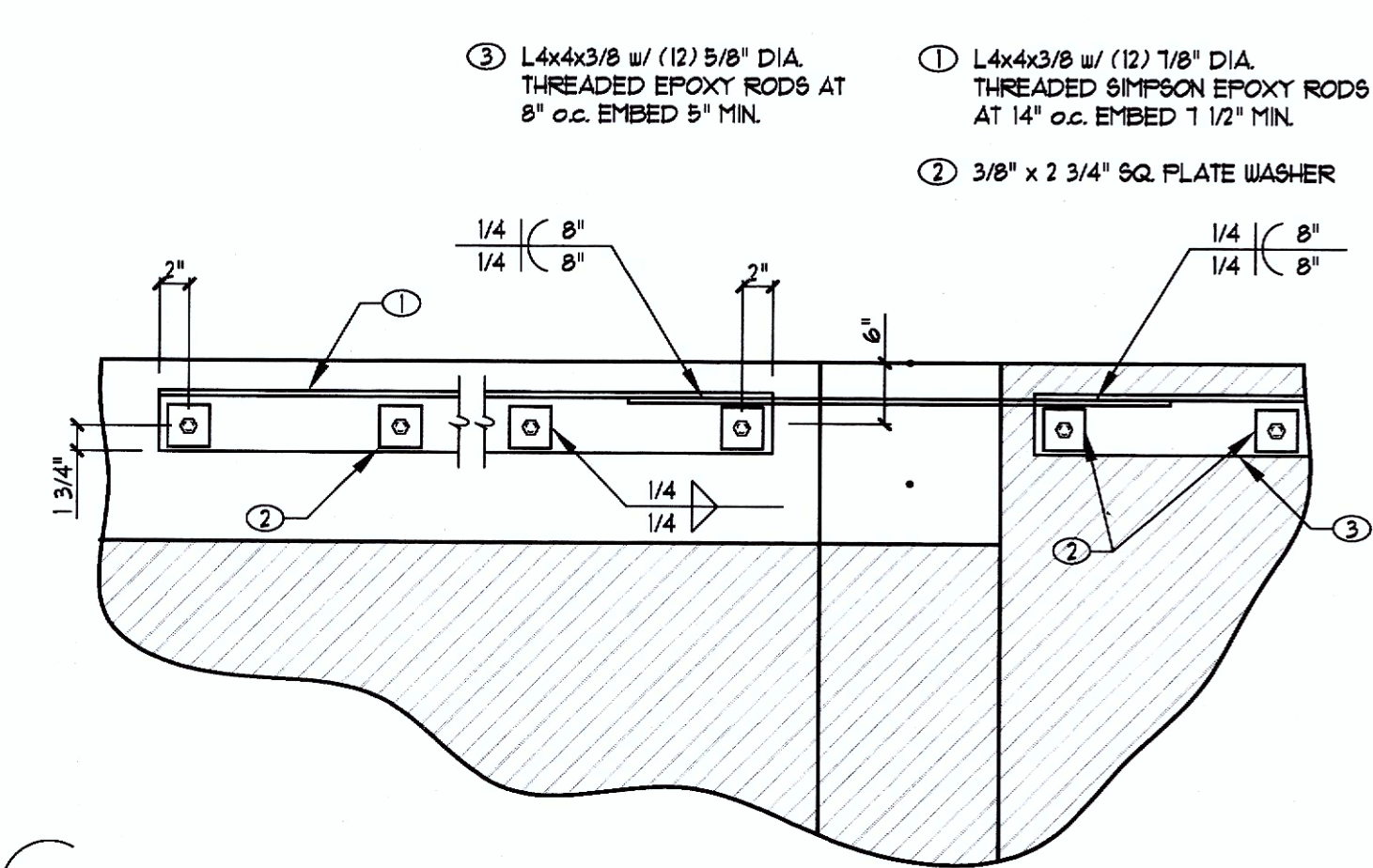


- SEE RFI #006-005/008

216 WOOD STUD WALL AT MASONRY WALL
216-112338

- 1 A35 AT EACH 2x4 JOIST w/ (6) #3 x 1 1/2" SCREWS EACH LEG
- 2 2x6 SOLID BLOCKING w/ LTP4 AT 24" o.c. TO NAILER AND A35 TO 1x SHEATHING PER DETAIL 208
- 3 OPTION FOR KEYNOTE (2). A35 AT 24" o.c. w/ (6) #3 x 1 1/2" SCREWS EACH LEG
- 4 3/8" x 11" PLATE AT 16" o.c. w/ (2) 5/8" DIA. SIMPSON EPOXY ANCHORS PER G&N. SPACED AT 8" APART. EMBED 5 1/2" IN GROUTED CELL. PRE-DRILL PLATE FOR (3) #3 x 2 1/2" SIMPSON SCREWS SPACED 4" APART. GROUT CELL IF NOT GROUTED

- Top plate recessed
SEE RFI #006-018



223 DRAG STRUT AT MASONRY WALL
223-112338

- 1 L4x4x3/8 w/ (12) 5/8" DIA. THREADED EPOXY RODS AT 8" o.c. EMBED 5" MIN.
- 2 3/8" x 2 3/4" SQ. PLATE WASHER
- 3 L4x4x3/8 w/ (12) 5/8" DIA. THREADED EPOXY RODS AT 14" o.c. EMBED 1 1/2" MIN.

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FILE 2012-007

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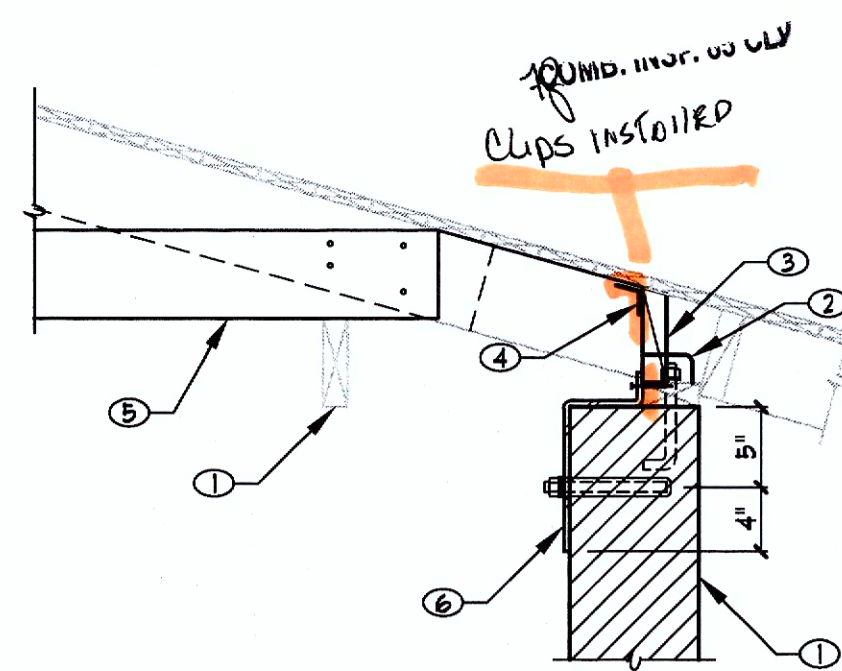
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SHT NO

S4.2

REVISED 8/27/03



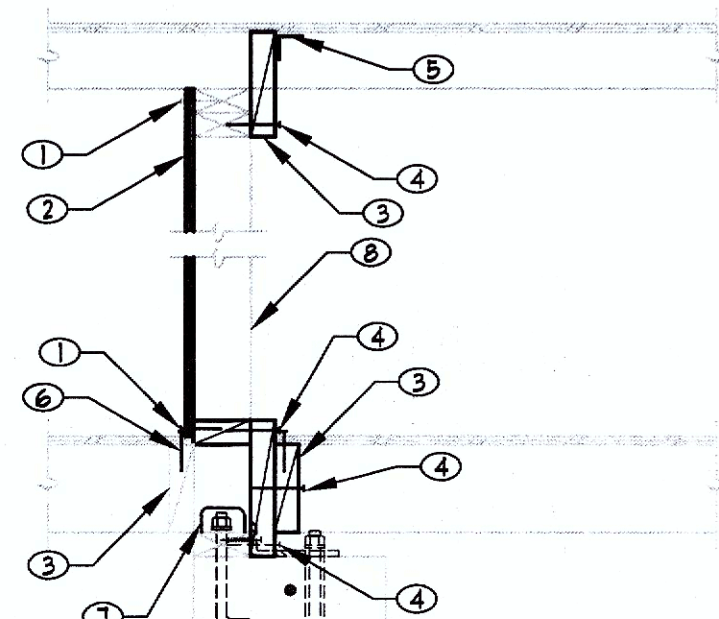
- EXISTING FRAMING
- A35 EACH RAFTER w/ (4) 1/2" x 1 1/2" LONG SCREWS EACH LEG
- 2x SHAPED SOLID BLOCKING w/ LTP4 TO EXISTING NAILER
- L990 AT 24" o.c. w/ SCREWS TO ROOF SHEATHING PER DETAIL 201
- 2x6 TIE CONT. OVER TOP OF EXISTING CEILING JOIST w/ (4) 16d NAILS EACH END. NO TIE AT 209A
- AT 209A 3/8" BENT PLATE x 6" LONG w/ (2) 5/8" DIA. THREADED SIMPSON EPOXY ANCHOR PER G&N SPACED AT 8" EMBED 5 1/2". PRE-DRILL PLATE FOR (3) 1/2" x 2 1/2" SIMPSON SCREWS SPACED AT 4" o.c. GROUT CELL SOLID IF NOT SOLID GROUTED

209 WOOD JOIST AT MASONRY WALL

209-112338
209A AS NOTED → SEE RFI #006-010

NO SCALE

- EDGE FASTENERS

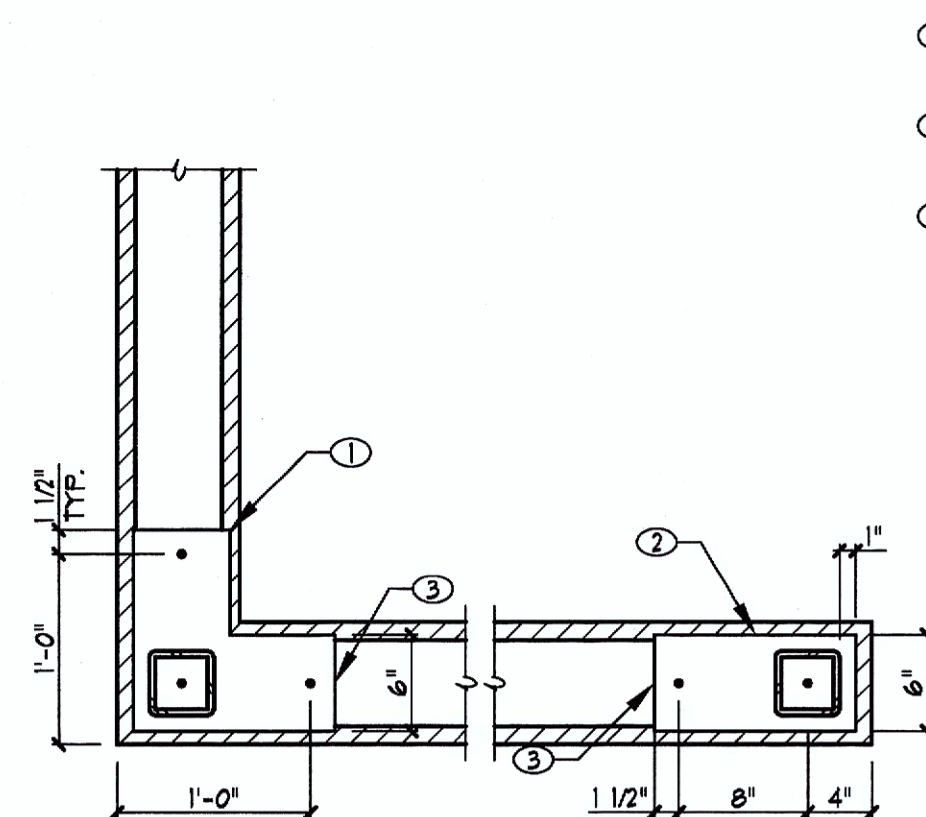


- 3/8" MIN. SHEATHING w/ 8d AT 4" o.c. ALL EDGES AND 12" o.c. FILLED. BLOCK ALL EDGES
- 2x SOLID BLOCKING
- 16d AT 4" o.c.
- A35 AT 16" o.c. ATTACHMENT TO BLOCK AND SHEATHING PER DETAIL 208
- BOUNDARY FASTENER
- A34 EACH JOIST
- ADD 2x4 AT 16" o.c. WHERE NO EXISTING 2x4s
- 3/8" BENT PLATE x 6" AT 2'-0" 5/8" DIA. THREADED SIMPSON EPOXY ANCHOR PER G&N EMBED 5 1/2". PRE-DRILL PLATE FOR (3) 1/2" x 2 1/2" SIMPSON SCREWS SPACED AT 2" o.c.

210 STUD WALL AT MASONRY WALL

210-112338

NO SCALE

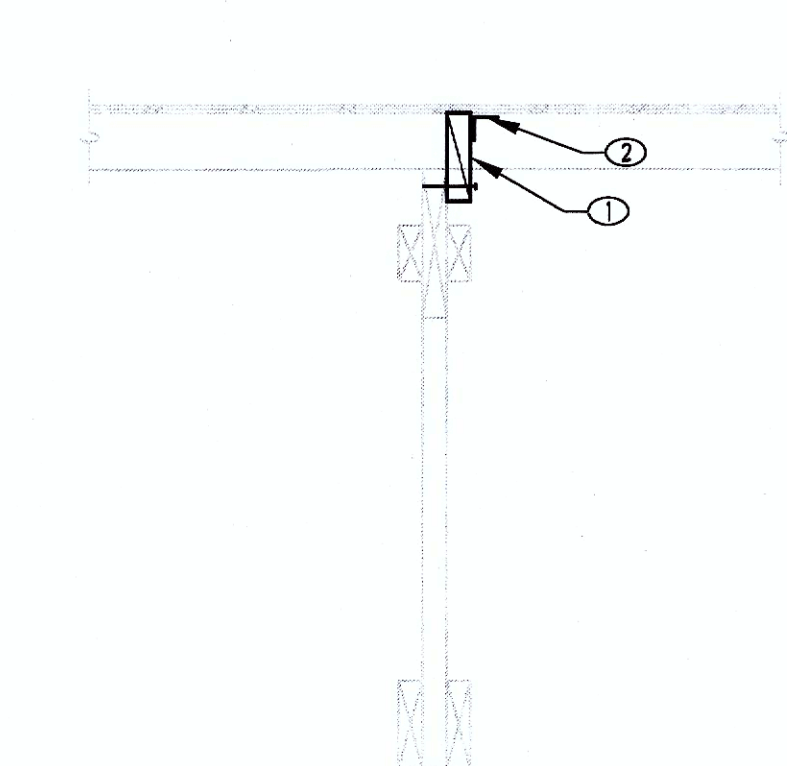


- 1/2" x 6" 1" PLATE w/ 3/4" DIA. x 8" WELDED STUD EACH END
- 1/2" PLATE w/ (2) 3/4" DIA. x 8" WELDED STUDS
- 1/2" GROUT BELOW PLATE PER G&N

211 STEEL COLUMN AT MASONRY WALL

211-112338

NO SCALE

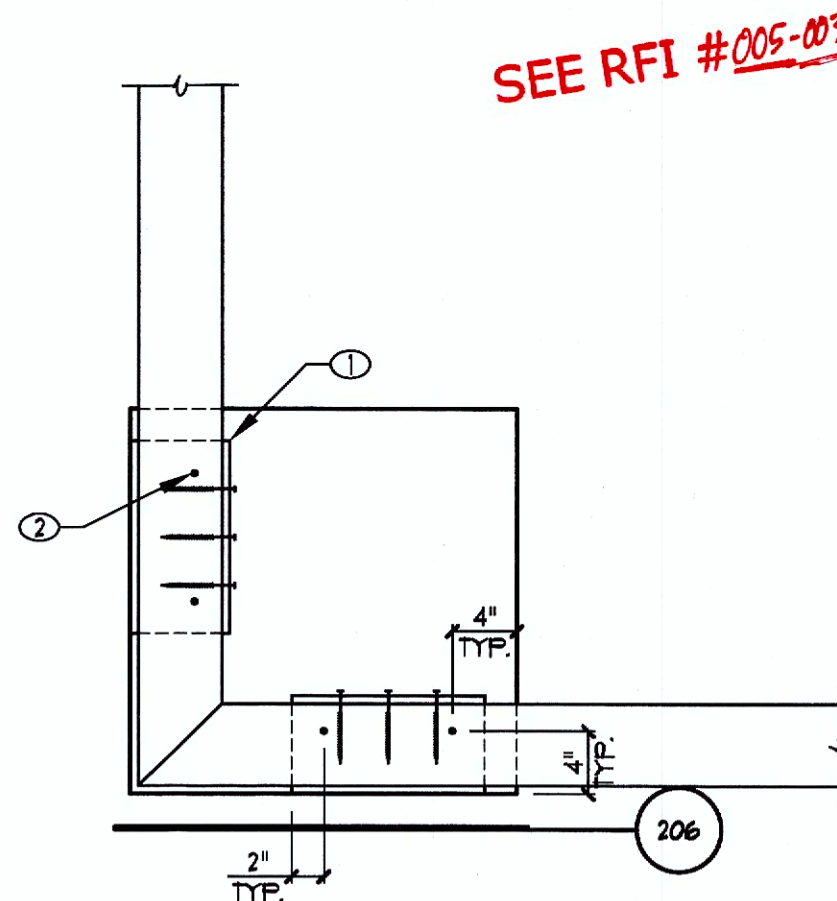


- 2x6 SOLID BLOCKING w/ 16d AT 6" o.c.
- A35 AT 24" o.c. PER DETAIL 208

212 WOOD BLOCKING AT WOOD TRUSS

212-112338

NO SCALE



SEE RFI #005-003

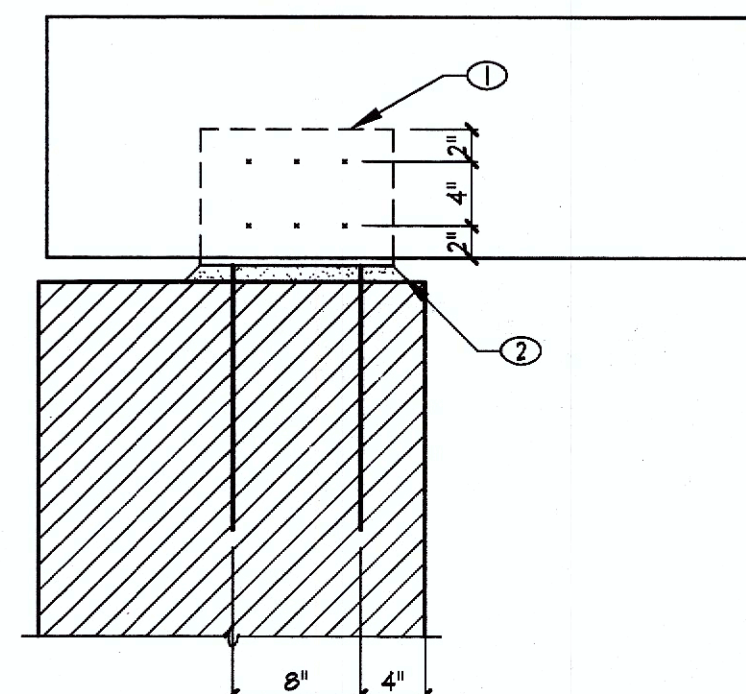
- 18x6x1/16 x 1'-0" LONG w/ (2) ROWS OF (3) SD614x4 1/2 SCREWS AT 4" o.c.
- (2) 1/2" x 1'-0" WELDABLE REBAR
- SINGLE BEAM AT 205A. SEE PLAN FOR BEAM ORIENTATION

205 WOOD BEAM AT MASONRY COLUMN

205-112338
205A AS NOTED

NO SCALE

- 18x4 PER DETAIL 205
- 1/2" 1" GROUT



206 WOOD BEAM AT MASONRY COLUMN

206-112338

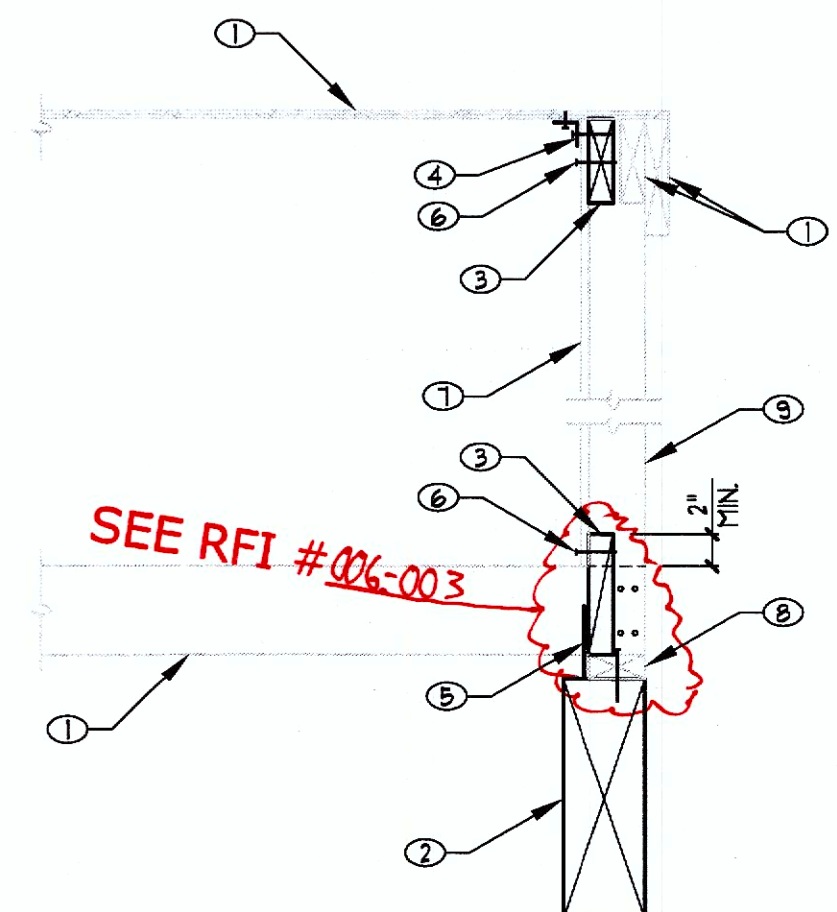
NO SCALE

- SAVE EXISTING ROOF FRAMING. SHORE AS REQ'D DURING CONSTRUCTION OF PORCH
- NEW BEAM PER PLAN
- 2x SHAPED SOLID BLOCKING
- LTP4 AT 24" o.c. TO BLOCK AND 2x SILL w/ A35 BETWEEN TO SILL AND BEAM
- L990 AT 24" o.c. w/ (4) 10d TO BLOCK AND (6) 0.138 DIAMETER 3/4" LONG SIMPSON STRONG TIE 8PA x 6" x 3/4"-R200 SCREWS
- 2x6 TIE CONT. OVER TOP OF EXISTING CEILING JOIST w/ (4) 16d NAILS EACH END
- REMOVE EXISTING ANCHOR BOLT FROM EXISTING 2x SILL

207 WOOD JOIST AT WOOD BEAM

207-112338

NO SCALE



SEE RFI #006-003

- SAVE EXISTING ROOF FRAMING. SHORE AS REQ'D DURING CONSTRUCTION OF PORCH
- NEW BEAM PER PLAN
- 2x SOLID BLOCKING
- A35 AT 24" o.c. w/ (6) 8d TO BLOCK AND (6) 0.138 DIAMETER 3/4" LONG SIMPSON STRONG TIES 8PA x 6" x 3/4"-R-200 SCREWS
- LTP4 AT 24" o.c.
- EDGE FASTENER
- 3/8" MIN. SHEATHING w/ 8d AT 4" o.c. ALL EDGES. BLOCK ALL SHEATHING EDGES
- REMOVE EXISTING ANCHOR BOLTS FROM 2x SILL. NAIL TO BEAM w/ 16d AT 6" o.c.
- DO NOT REMOVE EXISTING SIDING

208 WOOD JOIST AT WOOD BEAM

208-112338

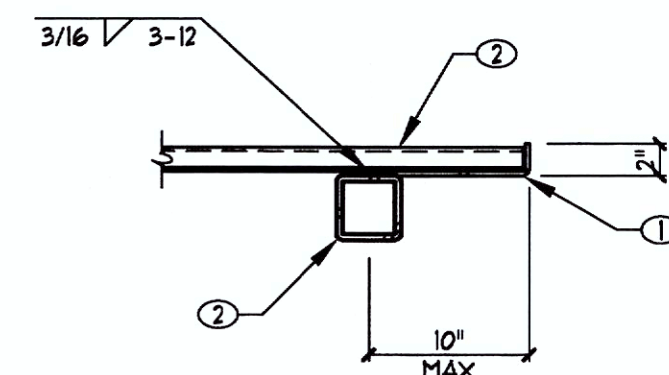
NO SCALE

201 STEEL DECK AT STEEL BEAM

201-112338

NO SCALE

- 3/16" CONT. BENT PLATE
- BEAM AND STEEL DECK PER PLAN



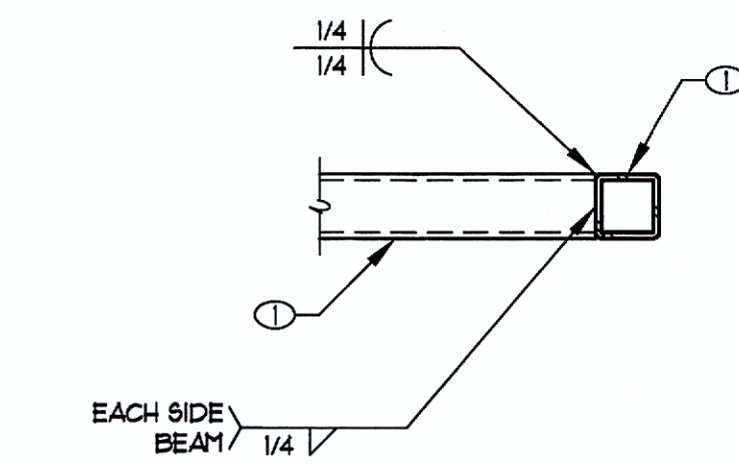
A. DECK ORIENTATION MAY BE AT 90° TO THAT SHOWN

202 STEEL DECK AT STEEL BEAM

202-112338

NO SCALE

- BEAM PER PLAN

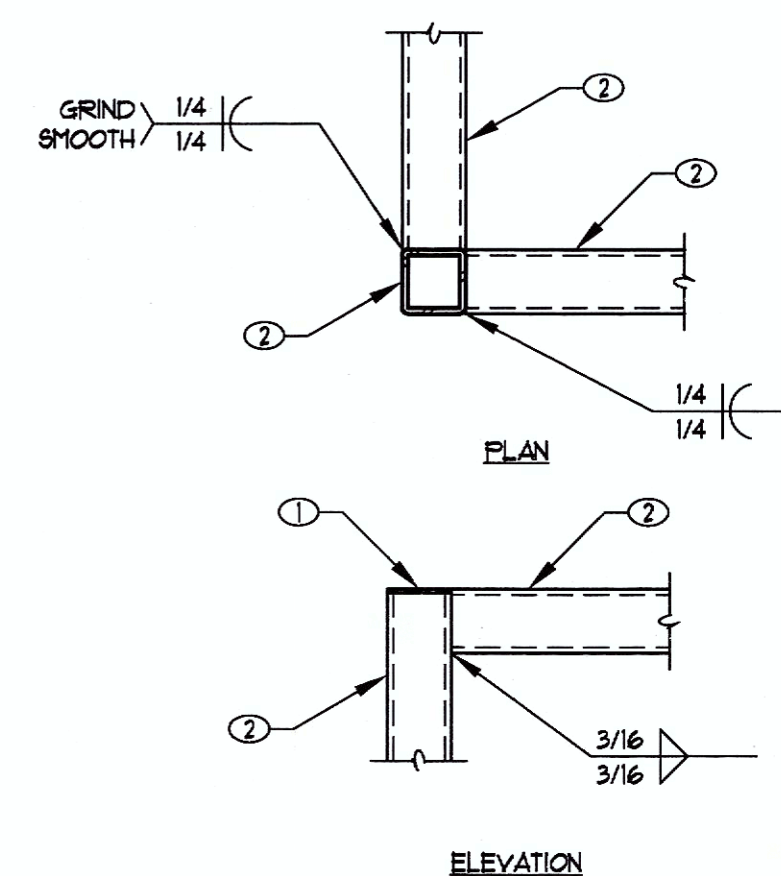


203 STEEL BEAM AT STEEL BEAM

203-112338

NO SCALE

- 1/4" CAP PLATE
- BEAM AND COLUMN PER PLAN



ELEVATION

204 STEEL BEAM AT STEEL COLUMN

204-112338

NO SCALE

REVISIONS

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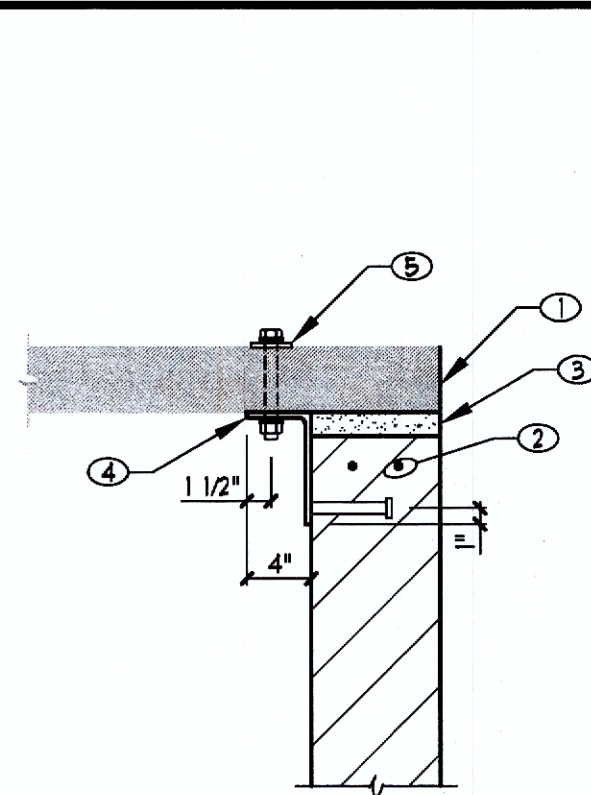
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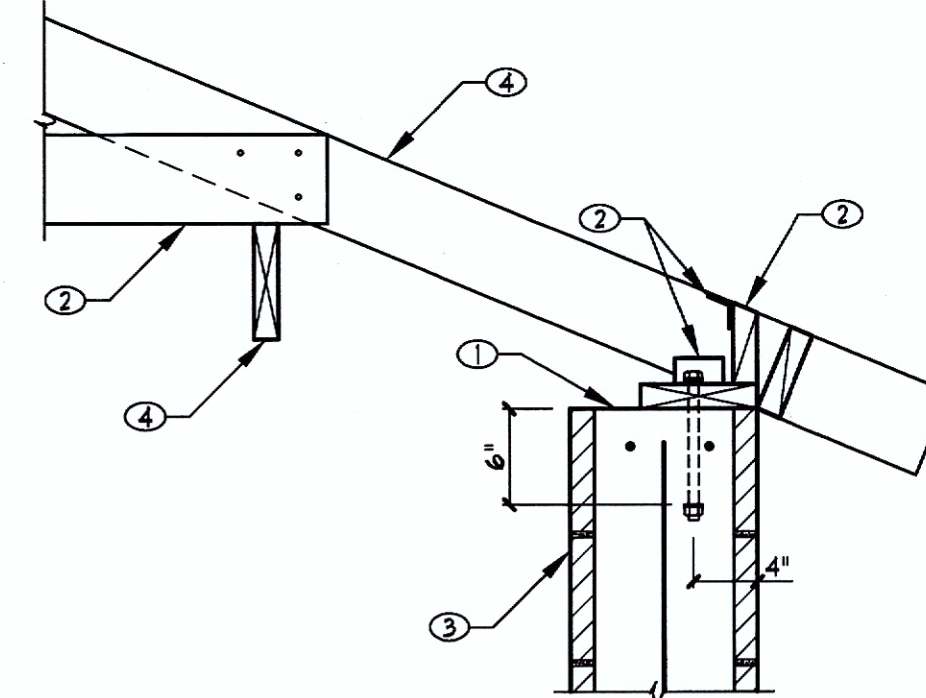
REVISED 8/27/03

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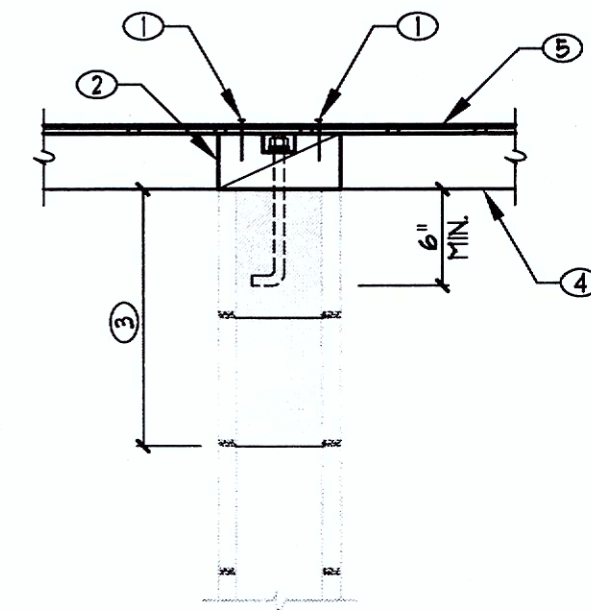
229 MASONRY WALL AT EXISTING CONCRETE SLAB
229-112338 NO SCALE

- 1 CUT EDGE OF EXISTING SLAB TO MATCH FACE OF NEW MASONRY WALL
- 2 (2) #5 CONT.
- 3 +/- 1 1/2" GROUT
- 4 L7x4x3/8 x 1'-0" AT 16" o.c. w/ (2) 3/4" DIA. BOLTS THRU EXISTING SLAB AND w/ (2) 5/8" DIA. x 5' LONG WELDED STUDS TO MASONRY. SPACE BOLTS AND STUDS 8" APART
- 5 2 1/2" SQ. x 1/4" PLATE WASHER



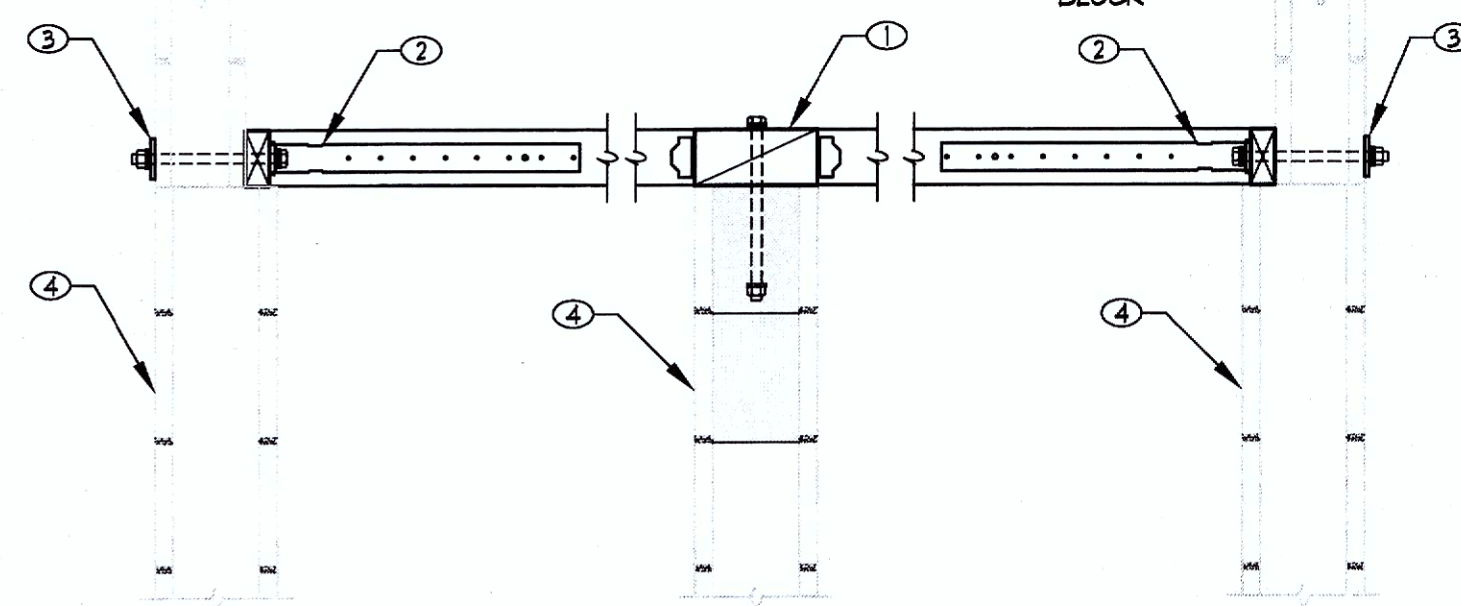
225 MASONRY WALL AT ROOF FRAMING
225-112338 NO SCALE

- 1 NEW 3x8 NAILER w/ 5/8" DIA. AB. AT 24" o.c.
- 2 A36, L90, SHAPED SOLID BLOCKING AND 2x6 TIE PER DETAIL 209
- 3 12" CMU WALL
- 4 EXISTING RAFTERS AND CEILING JOIST



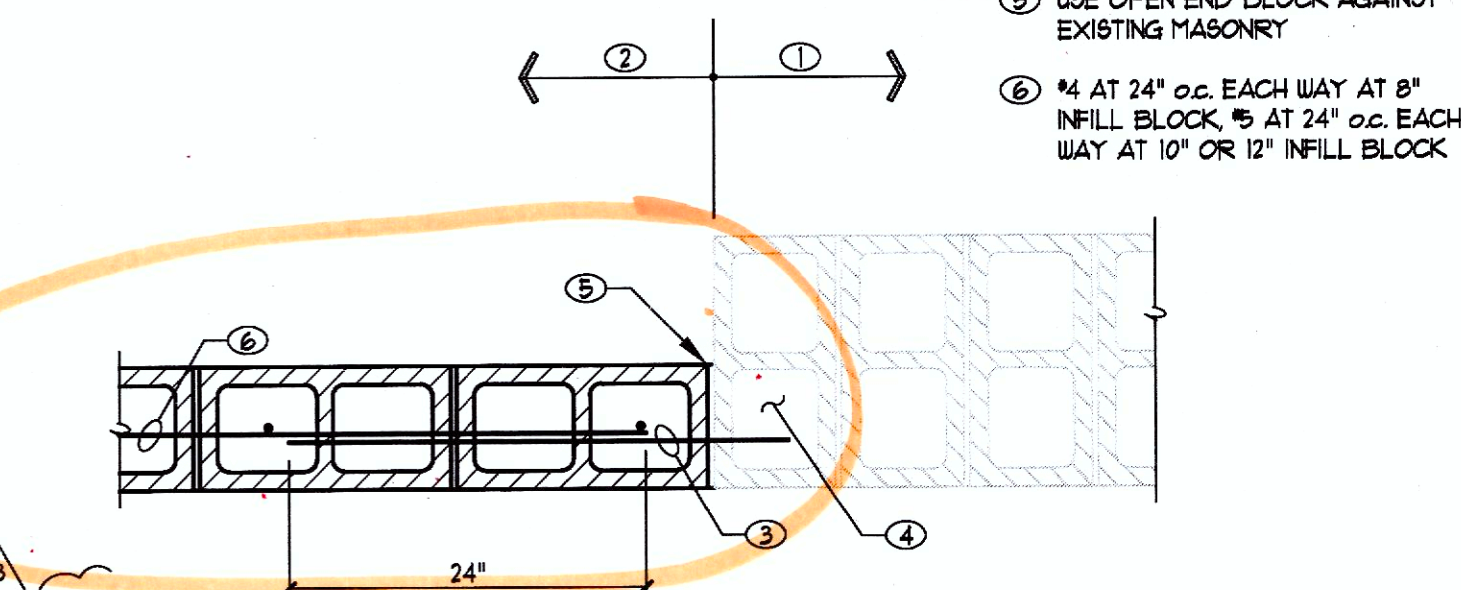
226 CEILING JOIST AT EXISTING MASONRY WALL (NO EXISTING NAILER)
226-112338 NO SCALE

- 1 BOUNDARY FASTENERS
- 2 4x8 SOLID BLOCKING w/ 5/8" DIA. AB. AT 16" o.c. RECESS AB.
- 3 GROUT SOLID TOP (2) COARSE OF MASONRY
- 4 EXISTING 2x4 CEILING JOIST AT 16" o.c. ADD JOIST AND ATTACH TO EXISTING TRUSS BOTTOM CHORD w/ LUS24 HANGERS
- 5 SHEATHING PER PLAN



227 WALL ANCHOR EXISTING MASONRY WALL
227-112338 NO SCALE

- 1 EXISTING MASONRY
- 2 NEW MASONRY, THICKNESS PER ARCH'L
- 3 #4 DOUCEL AT 24" o.c.
- 4 GROUT CELL OF EXISTING MASONRY SOLID
- 5 USE OPEN END BLOCK AGAINST EXISTING MASONRY
- 6 #4 AT 24" o.c. EACH WAY AT 8" INFILL BLOCK #5 AT 24" o.c. EACH WAY AT 10" OR 12" INFILL BLOCK



228 MASONRY INFILL AT EXISTING MASONRY
228-112338 NO SCALE

3-Door INFILLS Downels
4-14-15 72 COMB. INSP. 85 CLV

REVISIONS

NO.	DESCRIPTION	DATE
3	PLAN CHECK	

ARCHITECT

KME ARCHITECTS

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