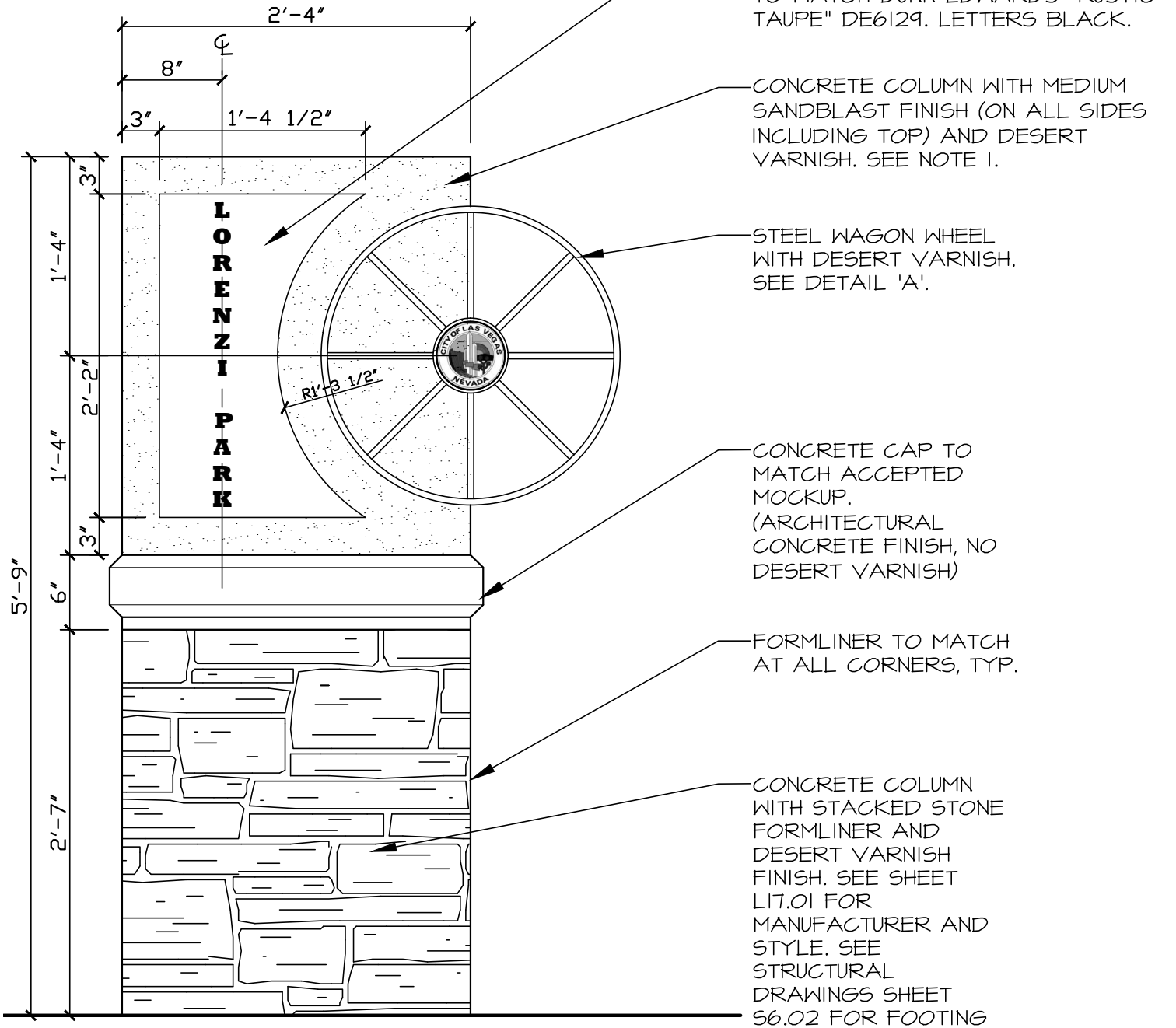


NOTES:

1. EXPOSED AGGREGATE SIZE TO BE 1/4"- 3/8".
2. CONTRACTOR IS NOT TO CONSTRUCT SIGNAGE COLUMN UNTIL INTERPRETIVE PANEL SUBMITTAL HAS BEEN APPROVED.



CLV LORENZI PARK PHASE II



L A S V E G A S N V
3191 S. JONES BLVD. | LAS VEGAS, NV 89146
T 702.253.9390 | F 702.253.1016
C A R L S B A D C A
2248 FARADAY AVE. | CARLSBAD, CA 92008

JW ZUNINO
LANDSCAPE ARCHITECTURE

jwzunino.com
NVA #013610 | DATE: 04/26/12 | DRAWN BY: ME | CHECKED BY: CL
NV #150 | UT #28394 | NM #282 | AZ #23194 | CA #2738 | TX #1736

INTERPRETIVE SIGNAGE COLUMN OPTION

SCALE: 1"=1'-0"

SKETCH #

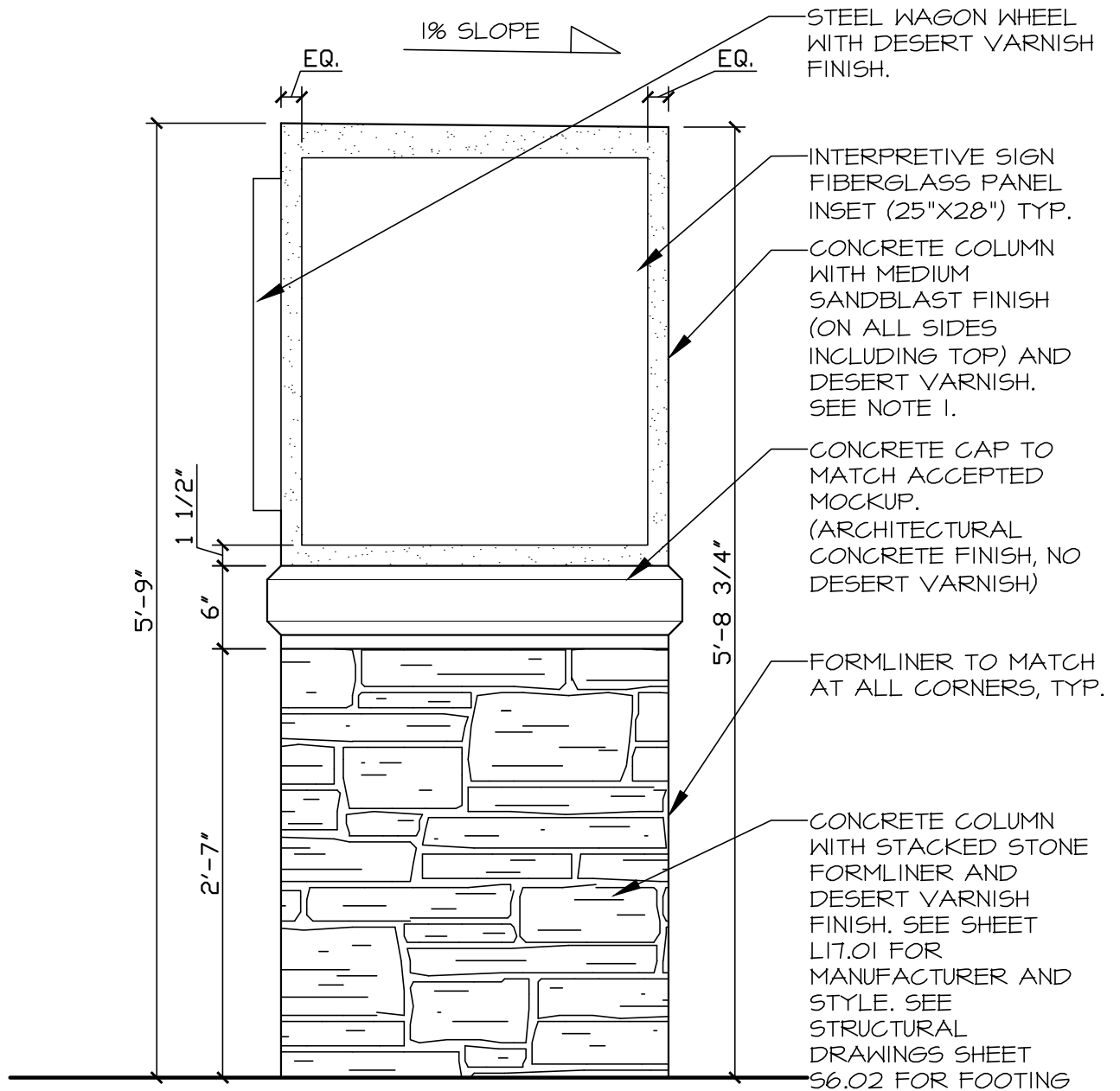
SKL-11..01

REF. DETAIL 'A'
SHEET L5.12

DATE: 10-22-12

NOTES:

1. EXPOSED AGGREGATE SIZE TO BE 1/4"- 3/8".
2. CONTRACTOR IS NOT TO CONSTRUCT SIGNAGE COLUMN UNTIL INTERPRETIVE PANEL SUBMITTAL HAS BEEN APPROVED.



RIGHT VIEW

CLV LORENZI PARK PHASE II

SKETCH #

SKL-11..01



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JWZUNINO
LANDSCAPE ARCHITECTURE

jwzunino.com
NVA #013610 | DATE: 04-26-12 | DRAWN BY: ME | CHECKED BY: CL
NV #150 | UT #28394 | NN #282 | AZ #23194 | CA #2738 | TX #1736

INTERPRETIVE SIGNAGE COLUMN OPTION

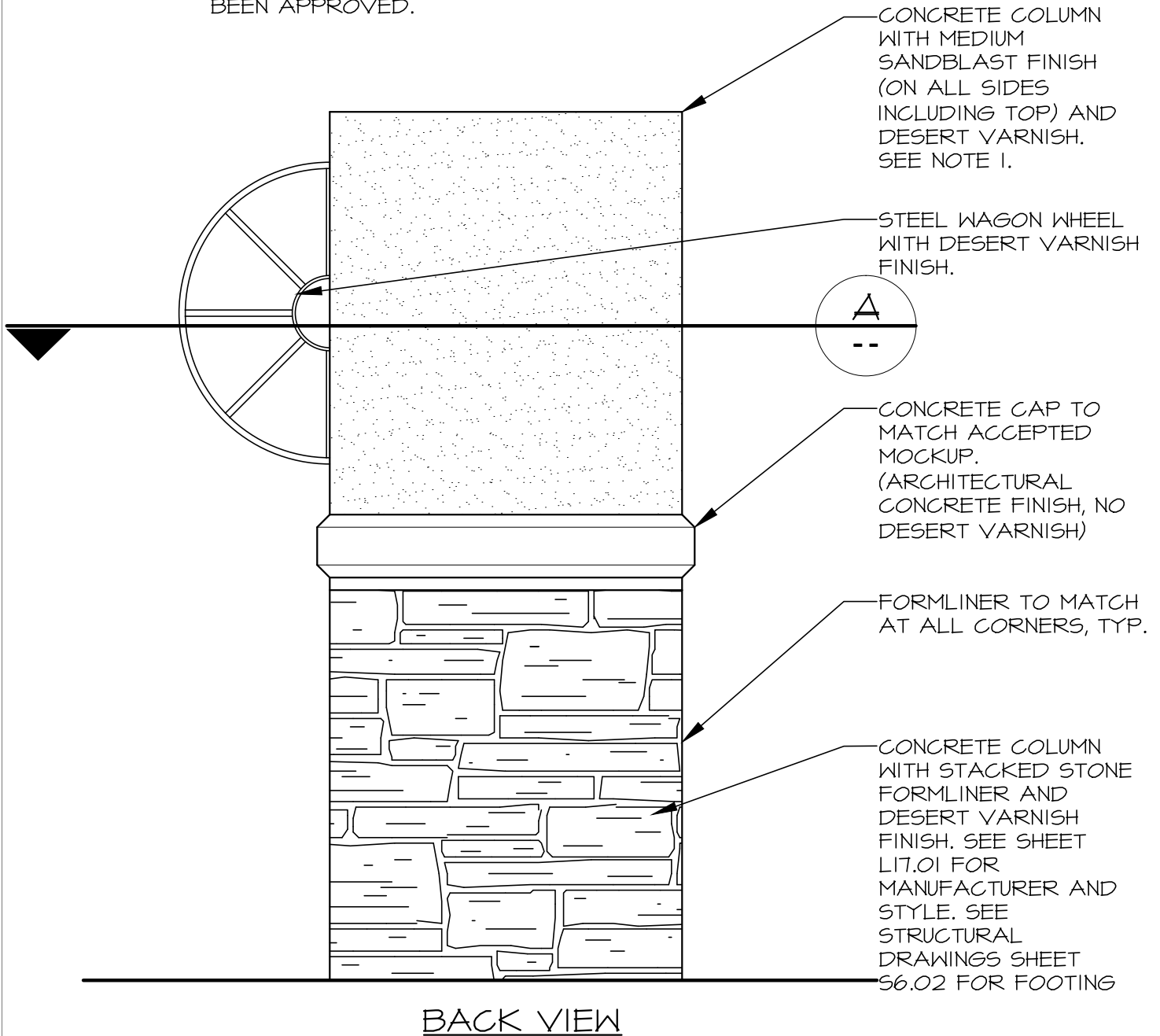
REF. DETAIL 'A'
SHEET L5.12

SCALE: 1"=1'-0"

DATE: 10-22-12

NOTES:

1. EXPOSED AGGREGATE SIZE TO BE 1/4"- 3/8".
2. CONTRACTOR IS NOT TO CONSTRUCT SIGNAGE COLUMN UNTIL INTERPRETIVE PANEL SUBMITTAL HAS BEEN APPROVED.



CLV LORENZI PARK PHASE II



JWZUNINO
LANDSCAPE ARCHITECTURE

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jwzunino.com
JWZUNINO 01/2012 DATE: 04/20/12 DRAWN BY: ME CHECKED BY: CL
NV #150 | UT #28394 | NM #262 | AZ #23194 | CA #2738 | TX #1736

INTERPRETIVE SIGNAGE COLUMN OPTION

SCALE: 1"=1'-0"

SKETCH #

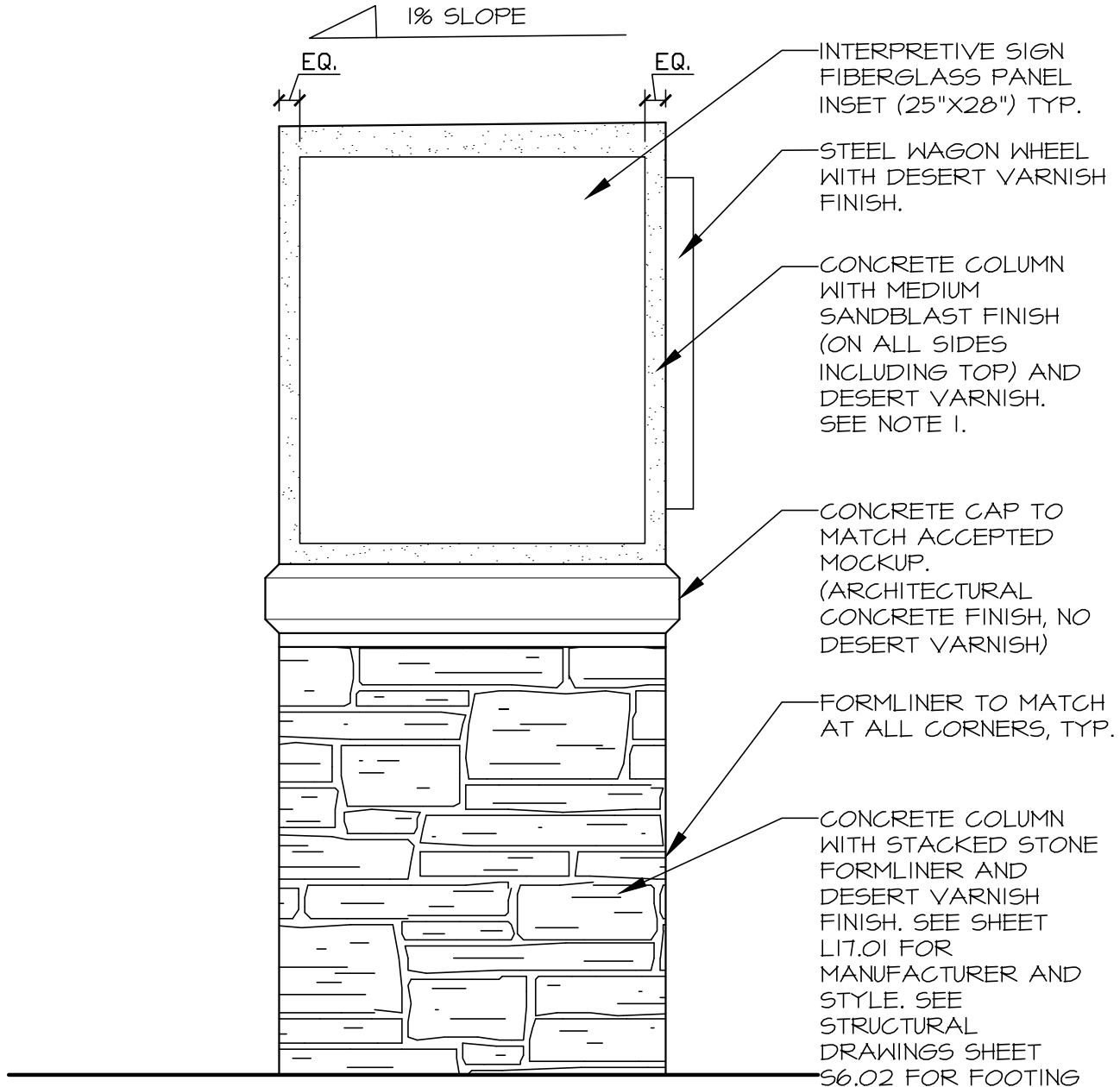
SKL-11..01

REF. DETAIL 'A'
SHEET L5.12

DATE: 10-22-12

NOTES:

1. EXPOSED AGGREGATE SIZE TO BE 1/4"- 3/8".
2. CONTRACTOR IS NOT TO CONSTRUCT SIGNAGE COLUMN UNTIL INTERPRETIVE PANEL SUBMITTAL HAS BEEN APPROVED.



LEFT VIEW

CLV LORENZI PARK PHASE II



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2248 FARADAY AVE. | CARLSBAD, CA 92008

JWZUNINO
LANDSCAPE ARCHITECTURE

jwzunino.com
JWZ 01/30/12 DATE 04/05/12 DRAWN BY ME CHECKED BY CL
NV #150 | UT #26394 | NH #262 | AZ #23194 | CA #2738 | TX #1736

INTERPRETIVE SIGNAGE COLUMN OPTION

SCALE: 1"=1'-0"

SKETCH #

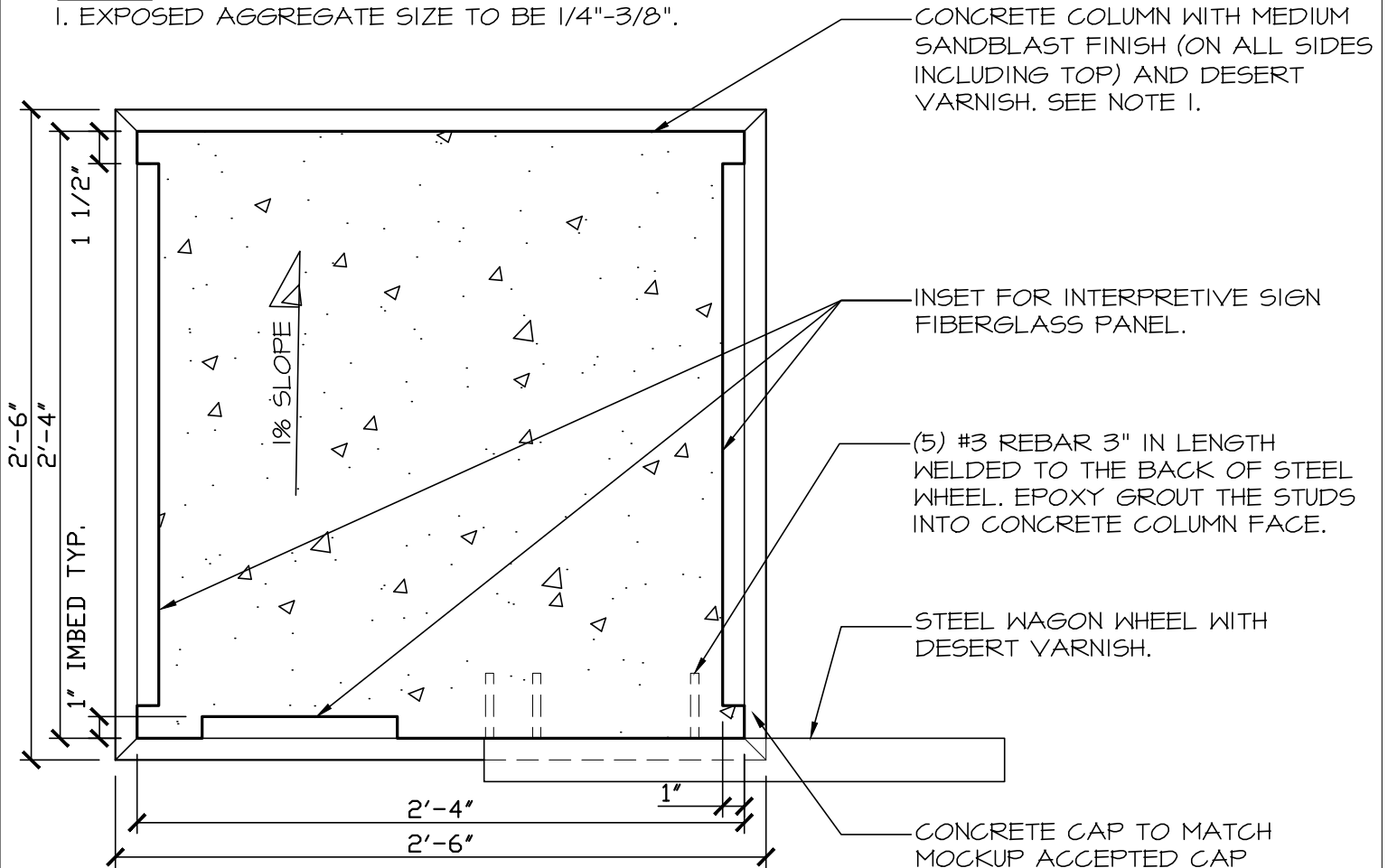
SKL-11..01

REF. DETAIL 'A'
SHEET L5.12

DATE: 10-22-12

NOTES:

1. EXPOSED AGGREGATE SIZE TO BE 1/4"-3/8".



SECTION 'A'

CLV LORENZI PARK PHASE II



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JWZUNINO
LANDSCAPE ARCHITECTURE

jwzunino.com 303.913.3510 DATE: 04/26/12 DRAWN BY: AE CHECKED BY: CL
NV #150 | UT #28394 | NM #262 | AZ #23194 | CA #2738 | TX #1736

INTERPRETIVE SIGNAGE
COLUMN OPTION

SCALE: 1-1/2"=1'-0"

SKETCH #

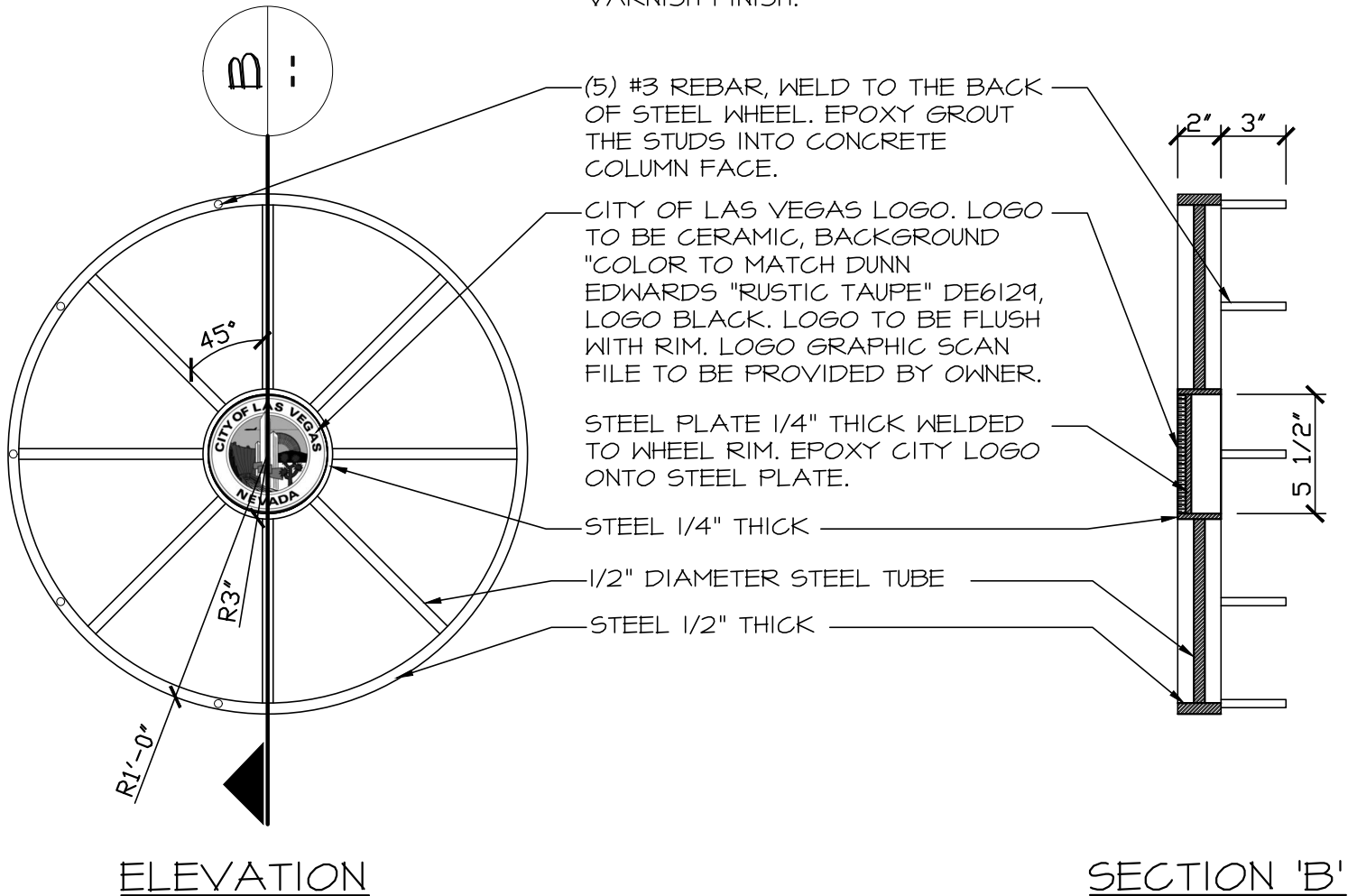
SKL-11..01

REF. DETAIL 'A'
SHEET L5.12

DATE: 10-22-12

NOTES:

1. STEEL WHEEL TO BE HOT DIP GALVANIZED AFTER FABRICATION THEN APPLY DESERT VARNISH FINISH.



DETAIL 'A'

CLV LORENZI PARK PHASE II



L A S V E G A S N V
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2248 FARADAY AVE. | CARLSBAD, CA 92008

JW ZUNINO
LANDSCAPE ARCHITECTURE

jwzunino.com
JWZ 0913020 DATE: 04/26/12 DRAWN BY: AE CHECKED BY: CL
NV #150 | UT #28394 | NN #282 | AZ #23194 | CA #2738 | TX #1736

**INTERPRETIVE SIGNAGE
COLUMN OPTION**

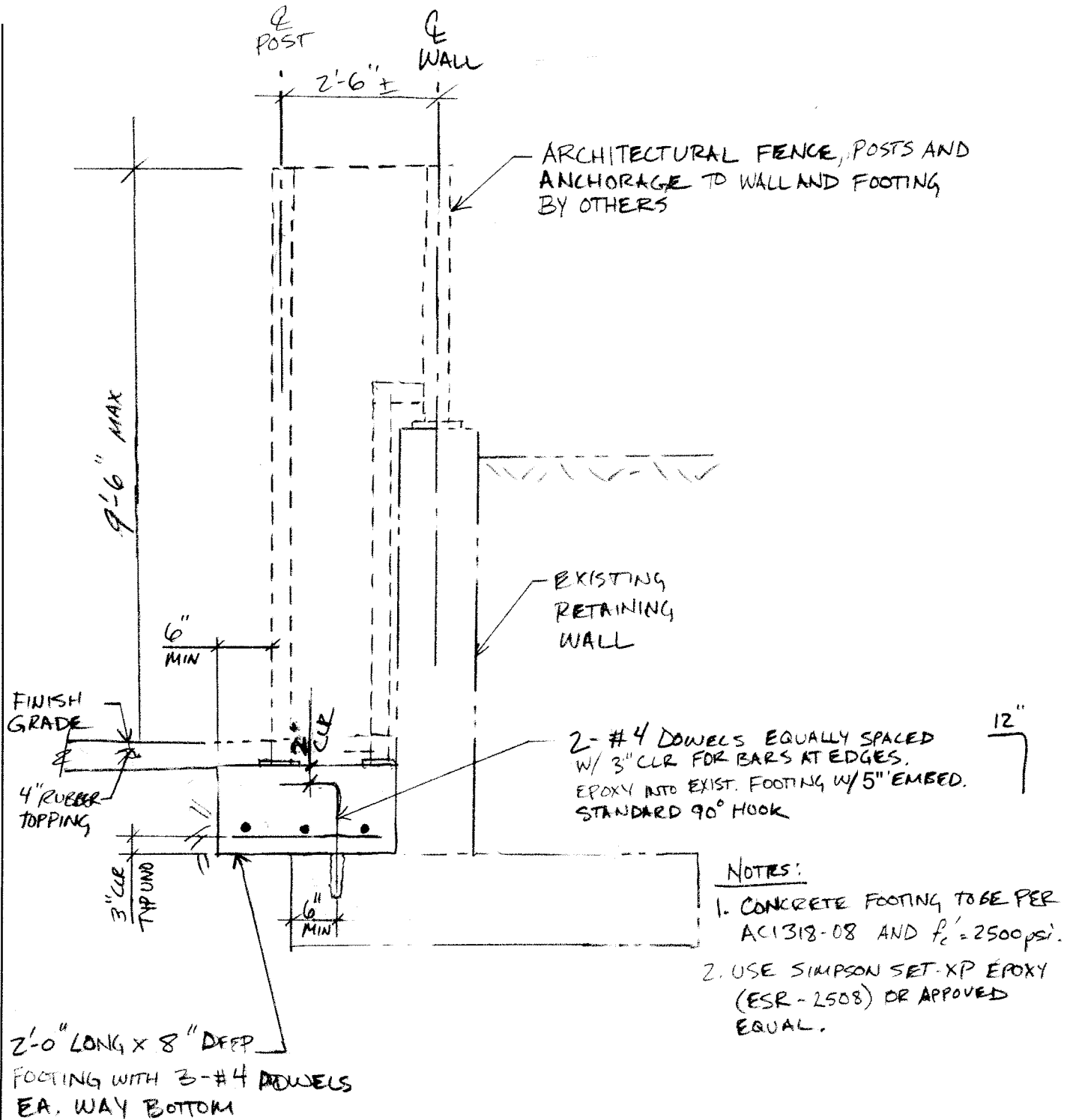
SCALE: 1-1/2"=1'-0"

SKETCH #

SKL-11..01

REF. DETAIL 'A'
SHEET L5.12

DATE: 10-22-12



DETERRENT FENCE AT WALL - WIND DESIGN

WIND LOAD - 2009 IBC → ASCE 7-05:

PER ASCE 7 6.5 METHOD 2 ANALYTICAL PROCEDURE:

EXPOSURE CATEGORY = C

$I_w = 1.0$

$h \approx 10 \text{ ft (fence wall)}$

$V = 90 \text{ mph}$

$q_z = 0.00256 K_z K_{zt} K_d V^2 I$; $K_d = 0.85$ (SIGNS/FENCES) - (TBL 6-4)

$K_z = 0.85$ (TBL 6-3)

$K_{zt} = 1.0$

$q_z = 0.00256(0.85)(1)(0.85)(90^2) 1.0$

$q_z = 14.98 \text{ psf} = q_h$

$\frac{F}{A_s} = q_h G C_f = 14.98 \text{ psf} (0.85)(1.65)$

where: $C_f = 1.65$ from TBL

$\frac{B}{S} = \frac{2.5 \text{ ft}}{12.5 \text{ ft}} \leftarrow \text{conservative}$

$= 21.0 \text{ psf}$

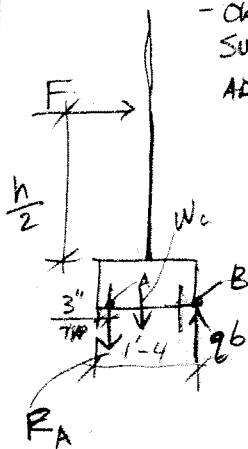
$F = 21 \text{ psf} (2.5 \text{ ft})(10 \text{ ft}) = 525 \text{ lb}$ reacting at 5 ft up.

$F/2 = \frac{525 \text{ lb}}{2} = 263 \text{ lb}$ AT NEW FOOTING

FOOTING FOR FENCE

PROVIDE A 16" WIDE X 2'-0" (OUT FROM WALL) FOOTING -

- OK FOR GRAVITY BY INSPECTION; HOWEVER, NEED ADDITIONAL SUPPORT FOR OVERTURNING. DESIGN FOR OVERTURNING, BY ADDING EPOXY DOWELS AT EDGES



$\sum M_B = 0 = 263 \text{ lb} (5 \text{ ft}) - R_A (2.0 \text{ ft} - .25 \text{ ft}) - W_c (\frac{1.5 \text{ ft}}{2})$

$R_A = \frac{1312.5 - 150(2)(2.0)(\frac{8}{12}) \text{ lb}}{1.0833} = 842 \text{ lb}$

$q_b = 842 \text{ lb}$

$q = 421 \text{ lb/ft} < q_A = 1500 \text{ plf/ft}$ (per IBC CL 18) ✓ OK for bearing on soil.

→ SEE SIMPSON CALCS FOR EPOXY CONNECTION CALCS.

OK TO USE 2'-0" X 2'-0" WIDE FOOTING WITH SIMPSON SET-XP (ESR-2508) WITH 2-#4 DOWELS INTO EXISTING FOOTING. (SEE SK 1)

Anchor Calculations

Anchor Selector (Version 4.11.0.0)

Job Name : 6821 Lorenzi Park PH2

Date/Time : 3/11/2013 12:10:40 PM

1) Input

Calculation Method : ACI 318 Appendix D For Uncracked Concrete

Code : ACI 318-08

Calculation Type : Analysis

Code Report : ICC-ES ESR-2508

a) Layout

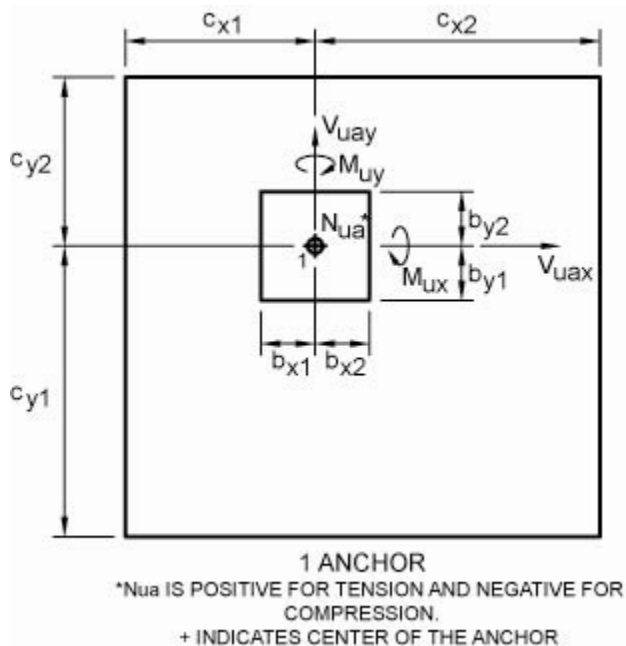
Anchor : 3/8" SET-XP

Number of Anchors : 1

Steel Grade: A615 GR. 60

Embedment Depth : 4 in

Built-up Grout Pads : No



Anchor Layout Dimensions :

c_{x1} : 3 in

c_{x2} : 12 in

c_{y1} : 6 in

c_{y2} : 12 in

b) Base Material

Concrete : Normal weight

f'_c : 2500.0 psi

Cracked Concrete : No

$\lambda_{c,V}$: 1.40

Condition : B tension and shear

iF_p : 1381.3
psi

Thickness, h_a : 8 in

Supplementary edge reinforcement : No

Hole Condition : Dry Concrete

Inspection : Continuous

Temperature Range : 1 (Maximum 110 °F short term and 75 °F long term temp.)

c) Factored Loads

Load factor source : ACI 318 Section 9.2

N_{ua} : 842 lb

V_{uax} : 263 lb

V_{uay} : 0 lb

M_{ux} : 0 lb*ft

M_{uy} : 0 lb*ft

e_x : 0 in

e_y : 0 in

Moderate/high seismic risk or intermediate/high design category : No

Anchor w/ sustained tension : No

Anchors only resist wind and/or seismic loads : No

Apply entire shear load at front row for breakout : No

d) Anchor Parameters

From ICC-ES ESR-2508 :

Anchor Model = SETXP $d_a = 0.375$ in

Category = 1 $h_{ef} = 4$ in

$h_{min} = 5.875$ in $c_{ac} = 12$ in

$c_{min} = 1.75$ in $s_{min} = 3$ in

Ductile = No

2) Tension Force on Each Individual Anchor

Anchor #1 $N_{ua1} = 842.00$ lb

Sum of Anchor Tension $\sum N_{ua} = 842.00$ lb

$e'_{Nx} = 0.00$ in

$e'_{Ny} = 0.00$ in

3) Shear Force on Each Individual Anchor

Resultant shear forces in each anchor:

Anchor #1 $V_{ua1} = 263.00$ lb ($V_{ua1x} = 263.00$ lb , $V_{ua1y} = 0.00$ lb)

Sum of Anchor Shear $\sum V_{uax} = 263.00$ lb, $\sum V_{uay} = 0.00$ lb

$$e'_{Vx} = 0.00 \text{ in}$$

$$e'_{Vy} = 0.00 \text{ in}$$

4) Steel Strength of Anchor in Tension [Sec. D.5.1]

$$N_{sa} = n A_{se} f_{uta} \text{ [Eq. D-3]}$$

Number of anchors acting in tension, $n = 1$

$$N_{sa} = 9900 \text{ lb (for a single anchor) [ICC-ES ESR-2508]}$$

$$i = 0.65 \text{ [D.4.4]}$$

$$iN_{sa} = 6435.00 \text{ lb (for a single anchor)}$$

5) Concrete Breakout Strength of Anchor in Tension [Sec. D.5.2]

$$N_{cb} = A_{Nc} / A_{Nco} \sqrt[3]{\lambda_{ed,N} \lambda_{c,N} \lambda_{cp,N}} N_b \text{ [Eq. D-4]}$$

Number of influencing edges = 1

$$h_{ef} = 4 \text{ in}$$

$$A_{Nco} = 144.00 \text{ in}^2 \text{ [Eq. D-6]}$$

$$A_{Nc} = 108.00 \text{ in}^2$$

Smallest edge distance, $c_{a,min} = 3.00 \text{ in}$

$$\lambda_{ed,N} = 0.8500 \text{ [Eq. D-10 or D-11]}$$

$$\lambda_{c,N} = 1.4100 \text{ [Sec. D.5.2.6]}$$

$$\lambda_{cp,N} = 0.5000 \text{ [Eq. D-12 or D-13]}$$

$$N_b = k_c \sqrt[3]{f'_c h_{ef}^{1.5}} = 6800.00 \text{ lb [Eq. D-7]}$$

$$k_c = 17 \text{ [Sec. D.5.2.6]}$$

$$N_{cb} = 3056.18 \text{ lb [Eq. D-4]}$$

$$i = 0.65 \text{ [D.4.4]}$$

$$iN_{cb} = 1986.51 \text{ lb (for a single anchor)}$$

6) Adhesive Strength of Anchor in Tension [Sec. D.5.3 (AC308 Sec.3.3)]

$$w_{k,uncr} = 1770 \text{ psi [ICC-ES ESR-2508]}$$

$$k_{uncr} = 24 \text{ [ICC-ES ESR-2508]}$$

$$h_{ef} \text{ (unadjusted)} = 4 \text{ in}$$

$$N_{ao} = w_{k,uncr} s_d h_{ef} = 8340.93 \text{ lb [Eq. D-16f]}$$

$$s_{cr,Na} = \min[20 d_a \sqrt[3]{(w_{k,uncr}/1450)}, 3h_{ef}] = 8.286 \text{ in [Eq. D-16d]}$$

$$c_{cr,Na} = s_{cr,Na}/2 = 4.143 \text{ in [Eq. D-16e]}$$

$$N_a = A_{Na} / A_{Nao} \sqrt[3]{\lambda_{ed,Na} \lambda_{p,Na}} N_{ao} \text{ [Eq. D-16a]}$$

$$A_{Na0} = 68.66 \text{ in}^2 \text{ [Eq. D-16c]}$$

$$A_{Na} = 59.19 \text{ in}^2$$

$$\text{Smallest edge distance, } c_{a,\min} = 3.00 \text{ in}$$

$$\lambda_{ed,Na} = \min[0.7 + 0.3c_{a,\min}/c_{cr,Na}, 1.0] = 0.9172 \text{ [Eq. D-16m]}$$

$$\lambda_{p,Na} = 0.3453 \text{ [Sec. D.5.3.14]}$$

$$N_a = 2277.04 \text{ lb [Eq. D-16a]}$$

$$i = 0.65 \text{ [ICC-ES ESR-2508]}$$

$$iN_a = 1480.08 \text{ lb (for a single anchor)}$$

7) Side Face Blowout of Anchor in Tension [Sec. D.5.4]

Concrete side face blowout strength is only calculated for headed anchors in tension close to an edge, $c_{a1} < 0.4h_{ef}$. Not applicable in this case.

8) Steel Strength of Anchor in Shear [Sec D.6.1]

$$V_{sa} = n0.6A_{se}f_{uta} \text{ [Eq. D-20]}$$

$$V_{sa} = 4950.00 \text{ lb (for a single anchor) [ICC-ES ESR-2508]}$$

$$i = 0.60 \text{ [D.4.4]}$$

$$iV_{sa} = 2970.00 \text{ lb (for a single anchor)}$$

9) Concrete Breakout Strength of Anchor in Shear [Sec D.6.2]

Case 1: Anchor checked against total shear load

In x-direction...

$$V_{cbx} = A_{vcx}/A_{vcox} \lambda_{ed,V} \lambda_{c,V} \lambda_{h,V} V_{bx} \text{ [Eq. D-21]}$$

$$c_{a1} = 8.00 \text{ in (adjusted for edges per D.6.2.4)}$$

$$A_{vcx} = 144.00 \text{ in}^2$$

$$A_{vcox} = 288.00 \text{ in}^2 \text{ [Eq. D-23]}$$

$$\lambda_{ed,V} = 0.8500 \text{ [Eq. D-27 or D-28]}$$

$$\lambda_{c,V} = 1.4000 \text{ [Sec. D.6.2.7]}$$

$$\lambda_{h,V} = \sqrt{(1.5c_{a1}/h_a)} = 1.2247 \text{ [Sec. D.6.2.8]}$$

$$V_{bx} = 7(l_e/d_a)^{0.2} \sqrt{d_a} \sqrt{f'_c} (c_{a1})^{1.5} \text{ [Eq. D-24]}$$

$$l_e = 3.00 \text{ in}$$

$$V_{bx} = 7350.83 \text{ lb}$$

$$V_{cbx} = 5356.72 \text{ lb [Eq. D-21]}$$

$$i = 0.70$$

$$iV_{cbx} = 3749.71 \text{ lb (for a single anchor)}$$

In y-direction...

$$V_{cby} = A_{vcy} / A_{vcoy} \lambda_{ed,V} \lambda_{c,V} \lambda_{h,V} V_{by} \text{ [Eq. D-21]}$$

$$c_{a1} = 8.00 \text{ in (adjusted for edges per D.6.2.4)}$$

$$A_{vcy} = 120.00 \text{ in}^2$$

$$A_{vcoy} = 288.00 \text{ in}^2 \text{ [Eq. D-23]}$$

$$\lambda_{ed,V} = 0.7750 \text{ [Eq. D-27 or D-28]}$$

$$\lambda_{c,V} = 1.4000 \text{ [Sec. D.6.2.7]}$$

$$\lambda_{h,V} = \sqrt{(1.5c_{a1} / h_a)} = 1.2247 \text{ [Sec. D.6.2.8]}$$

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a} \sqrt{f_c} (c_{a1})^{1.5} \text{ [Eq. D-24]}$$

$$l_e = 3.00 \text{ in}$$

$$V_{by} = 7350.83 \text{ lb}$$

$$V_{cby} = 4070.06 \text{ lb [Eq. D-21]}$$

$$i = 0.70$$

$$iV_{cby} = 2849.04 \text{ lb (for a single anchor)}$$

Case 2: This case does not apply to single anchor layout

Case 3: Anchor checked for parallel to edge condition

Check anchors at c_{x1} edge

$$V_{cbx} = A_{vcx} / A_{vcox} \lambda_{ed,V} \lambda_{c,V} \lambda_{h,V} V_{bx} \text{ [Eq. D-21]}$$

$$c_{a1} = 3.00 \text{ in}$$

$$A_{vcx} = 40.50 \text{ in}^2$$

$$A_{vcox} = 40.50 \text{ in}^2 \text{ [Eq. D-23]}$$

$$\lambda_{ed,V} = 1.0000 \text{ [Sec. D.6.2.1(c)]}$$

$$\lambda_{c,V} = 1.4000 \text{ [Sec. D.6.2.7]}$$

$$\lambda_{h,V} = \sqrt{(1.5c_{a1} / h_a)} = 1.0000 \text{ [Sec. D.6.2.8]}$$

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a} \sqrt{f_c} (c_{a1})^{1.5} \text{ [Eq. D-24]}$$

$$l_e = 3.00 \text{ in}$$

$$V_{bx} = 1688.04 \text{ lb}$$

$$V_{cbx} = 2363.26 \text{ lb [Eq. D-21]}$$

$$V_{cby} = 2 * V_{cbx} \text{ [Sec. D.6.2.1(c)]}$$

$$V_{cby} = 4726.52 \text{ lb}$$

$$i = 0.70$$

$$iV_{cby} = 3308.56 \text{ lb (for a single anchor)}$$

Check anchors at c_{y1} edge

$$V_{cby} = A_{vcy} / A_{vcoy} \lambda_{ed,V} \lambda_{c,V} \lambda_{h,V} V_{by} \text{ [Eq. D-21]}$$

$$c_{a1} = 6.00 \text{ in}$$

$$A_{vcy} = 96.00 \text{ in}^2$$

$$A_{vcoy} = 162.00 \text{ in}^2 \text{ [Eq. D-23]}$$

$$\lambda_{ed,V} = 1.0000 \text{ [Sec. D.6.2.1(c)]}$$

$$\lambda_{c,V} = 1.4000 \text{ [Sec. D.6.2.7]}$$

$$\lambda_{h,V} = \sqrt{1.5c_{a1} / h_a} = 1.0607 \text{ [Sec. D.6.2.8]}$$

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a} \phi f_c (c_{a1})^{1.5} \text{ [Eq. D-24]}$$

$$l_e = 3.00 \text{ in}$$

$$V_{by} = 4774.51 \text{ lb}$$

$$V_{cby} = 4201.35 \text{ lb [Eq. D-21]}$$

$$V_{cbx} = 2 * V_{cby} \text{ [Sec. D.6.2.1(c)]}$$

$$V_{cbx} = 8402.70 \text{ lb}$$

$$i = 0.70$$

$$iV_{cbx} = 5881.89 \text{ lb (for a single anchor)}$$

Check anchors at c_{x2} edge

$$V_{cbx} = A_{vcx} / A_{vcox} \lambda_{ed,V} \lambda_{c,V} \lambda_{h,V} V_{bx} \text{ [Eq. D-21]}$$

$$c_{a1} = 8.00 \text{ in (adjusted for edges per D.6.2.4)}$$

$$A_{vcx} = 144.00 \text{ in}^2$$

$$A_{vcox} = 288.00 \text{ in}^2 \text{ [Eq. D-23]}$$

$$\lambda_{ed,V} = 1.0000 \text{ [Eq. D-27 or D-28] [Sec. D.6.2.1(c)]}$$

$$\lambda_{c,V} = 1.4000 \text{ [Sec. D.6.2.7]}$$

$$\lambda_{h,V} = \sqrt{1.5c_{a1} / h_a} = 1.2247 \text{ [Sec. D.6.2.8]}$$

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a} \phi f_c (c_{a1})^{1.5} \text{ [Eq. D-24]}$$

$$l_e = 3.00 \text{ in}$$

$$V_{bx} = 7350.83 \text{ lb}$$

$$V_{cbx} = 6302.03 \text{ lb [Eq. D-21]}$$

$$V_{cby} = 2 * V_{cbx} \text{ [Sec. D.6.2.1(c)]}$$

$$V_{cby} = 12604.06 \text{ lb}$$

$$i = 0.70$$

$$iV_{cby} = 8822.84 \text{ lb (for a single anchor)}$$

Check anchors at c_{y2} edge

$$V_{cby} = A_{vcy} / A_{vcy} \setminus_{ed,V} \setminus_{c,V} \setminus_{h,V} V_{by} \text{ [Eq. D-21]}$$

$$c_{a1} = 8.00 \text{ in (adjusted for edges per D.6.2.4)}$$

$$A_{vcy} = 120.00 \text{ in}^2$$

$$A_{vcy} = 288.00 \text{ in}^2 \text{ [Eq. D-23]}$$

$$\setminus_{ed,V} = 1.0000 \text{ [Sec. D.6.2.1(c)]}$$

$$\setminus_{c,V} = 1.4000 \text{ [Sec. D.6.2.7]}$$

$$\setminus_{h,V} = \sqrt{(1.5c_{a1} / h_a)} = 1.2247 \text{ [Sec. D.6.2.8]}$$

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a} \sqrt{f_c} (c_{a1})^{1.5} \text{ [Eq. D-24]}$$

$$l_e = 3.00 \text{ in}$$

$$V_{by} = 7350.83 \text{ lb}$$

$$V_{cby} = 5251.69 \text{ lb [Eq. D-21]}$$

$$V_{cbx} = 2 * V_{cby} \text{ [Sec. D.6.2.1(c)]}$$

$$V_{cbx} = 10503.38 \text{ lb}$$

$$i = 0.70$$

$$iV_{cbx} = 7352.37 \text{ lb (for a single anchor)}$$

10) Concrete Pryout Strength of Anchor in Shear [Sec. D.6.3]

$$V_{cp} = \min[k_{cp} N_a, k_{cp} N_{cb}] \text{ [Eq. D-30a]}$$

$$k_{cp} = 2 \text{ [Sec. D.6.3.2]}$$

$$N_a = 2277.04 \text{ lb (from Section (6) of calculations)}$$

$$N_{cb} = 3056.18 \text{ lb (from Section (5) of calculations)}$$

$$V_{cp} = 4554.08 \text{ lb}$$

$$i = 0.70 \text{ [D.4.4]}$$

$$iV_{cp} = 3187.85 \text{ lb (for a single anchor)}$$

11) Check Demand/Capacity Ratios [Sec. D.7]

Tension

- Steel : 0.1308

- Breakout : 0.4239

- Adhesive : 0.5689

- Sideface Blowout : N/A

Shear

- Steel : 0.0886
- Breakout (case 1) : 0.0701
- Breakout (case 2) : N/A
- Breakout (case 3) : 0.0447
- Pryout : 0.0825

$V_{\text{Max}}(0.09) \leq 0.2$ and $T_{\text{Max}}(0.57) \leq 1.0$ [Sec D.7.1]

Interaction check: PASS

Use 3/8" diameter A615 GR. 60 SET-XP anchor(s) with 4 in. embedment