

• 301 Casino Center (5)

Lila Collins 12-19-84

MARTIN

&

Associates

MARTIN & ASSOCIATES

□

STRUCTURAL ENGINEERS

Architect

Project

Sheet 237 of

Job no.

Date

Engineer

$$0.54(1.5) + 0.125(1.7) = 1.0225 \frac{k}{ft}$$

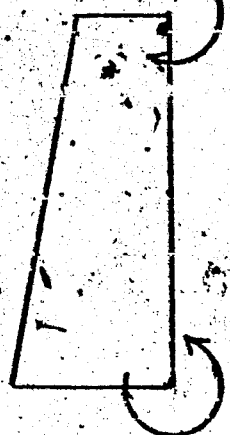


$$4.63(1.5) + 0.70(1.7) = 8.735 \text{ k}$$

LEFT WALL

$$5.39(1.5) + 0.54(1.7) = 9.003 \text{ k}$$

$$1.39(1.5) + 0.128(1.7) = 2.2975 \frac{k}{ft}$$



$$15.19(1.5) + 2.65(1.7) = 27.29$$

$$\therefore M(t) = 0$$

$$15.94(1.5) + 2.44(1.7) = 28.143$$

Lila Collins 12-19-84

MARTIN

&

associates

MARTIN & ASSOCIATES



STRUCTURAL ENGINEERS

Architect

Project

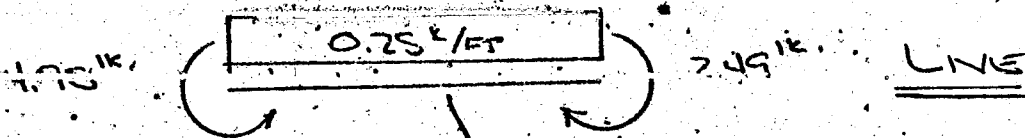
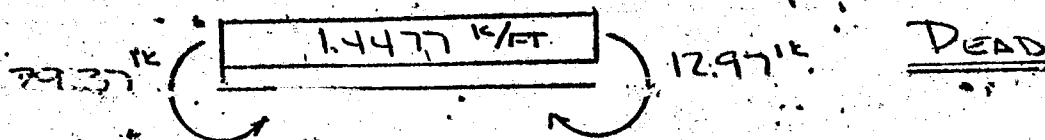
Sheet 240 of

Job no.

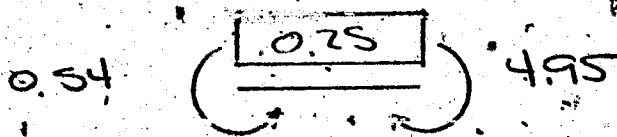
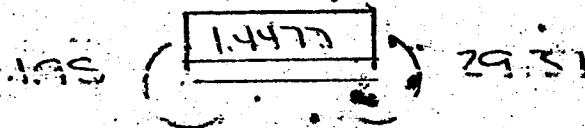
Date

Engineer

BOTTOM 15.85 SPAN



BOTTOM 1.833 SPAN



SIMPLE SPAN SHEARS, MOMENTS
AND DEFLECTIONS BY
JAMES A. GERVIC S.E.

BEAM I.D.: BOT SLAB 15.83 FT

BEAM SPAN 15.833 FEET
MOD. OF ELASTICITY 3122 KSI
I OF BEAM 3375.00 IN⁴

MULTIPLICATION FACTORS:

D.L. MOMENTS & SHEARS 1.5000
L.L. MOMENTS & SHEARS 1.7000
D.L. DEFLECTIONS 1.0000
L.L. DEFLECTIONS 1.0000

LOAD TYPES AND CONFIGURATIONS:

SPAN DEAD LOADS:

UNIF. W= 1.448 K/FT
LEFT END MOMENT -29.37 FT-K
RIGHT END MOMENT -12.97 FT-K

UNIF. W= 0.250 K/FT
LEFT END MOMENT -4.95 FT-K
RIGHT END MOMENT -4.95 FT-K

BEAM MOMENTS AND SHEAR:

X DIST	U.L. MOMENT	MAX. MOMENT	MIN. MOMENT	ABS
0.0	-44.1	-44.1	-52.5	22
1.0	-26.6	-26.6	-31.6	20
2.0	-11.2	-11.2	-13.3	17
3.0	2.0	2.0	2.0	13
4.0	13.1	15.8	13.1	15
4.9	22.1	25.4	22.1	25
5.0	20.0	24.0	20.0	24
6.0	33.7	40.2	33.7	4
7.0	36.3	43.3	36.3	2
8.0	36.8	43.8	36.8	1
9.0	35.4	41.0	35.4	3
10.0	31.3	37.2	31.3	8
12.0	17.4	20.5	17.4	11
13.0	7.2	8.3	7.2	14
14.0	-5.0	-5.0	-6.4	16
15.0	-19.5	-19.5	-23.7	19

REACTIONS:

D.L. LT 18.75K RT 15.64K
MAX L.L. LT 3.63K RT 3.10K
MIN L.L. LT 0.00K RT 0.00K

BEAM I.D.: BOT SLAB 7.833 FT

BEAM SPAN 7.833 FEET
MOD. OF ELASTICITY 3122 KSI
I OF BEAM 3375.00 IN⁴

MULTIPLICATION FACTORS:

D.L. MOMENTS & SHEARS 1.5000
L.L. MOMENTS & SHEARS 1.7000
D.L. DEFLECTIONS 1.0000
L.L. DEFLECTIONS 1.0000

LOAD TYPES AND CONFIGURATIONS:

SPAN DEAD LOADS:

UNIF. W= 1.448 K/FT
LEFT END MOMENT -1.95 FT-K
RIGHT END MOMENT -29.37 FT-K

UNIF. W= 0.250 K/FT
LEFT END MOMENT -0.54 FT-K
RIGHT END MOMENT -4.95 FT-K

BEAM MOMENTS AND SHEAR:

X DIST	D.L. MOMENT	MAX. MOMENT	MIN. MOMENT	ABS
0.0	-2.9	-2.9	-3.8	4
1.0	-0.8	-0.8	-1.2	1
2.0	-0.7	-0.7	-1.1	1
2.9	2.7	2.7	3.4	4
3.0	5.8	6.8	5.8	6
4.0	13.8	15.0	13.8	9
5.0	21.3	21.3	25.4	11
6.0	31.6	31.6	37.7	14
7.0	44.1	44.1	52.5	16

REACTIONS:

D.L. LT 3.25K RT 43.76K
MAX L.L. LT 20.71K RT 3.62K
MIN L.L. LT 0.00K RT 0.00K

Lucia Collins 12-19-84

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SINGLE SPAN SHEARS, MOMENTS
AND DEFLECTIONS BY
JAMES A. GERNIG S.E.

CLARK COUNTY JAIL

BEAM I.D.: TUNNEL TOP

BEAM SPAN: 23.667 FEET
MOD. OF ELASTICITY 3122 KSI.
I OF BEAM 3375.00 IN⁴

MULTIPLICATION FACTORS:

L.L. MOMENTS & SHEARS 1.7000
D.L. DEFLECTIONS 1.0000
L.L. DEFLECTIONS 1.0000

LOAD TYPES AND CONFIGURATIONS:

SPAN DEAD LOADS:

UNIF. W= 1.313 K/FT
CONC. P= -19.39 K, X= 7.83 FT
LEFT END MOMENT -1.62 FT-K
RIGHT END MOMENT -12.63 FT-K

SPAN LIVE LOADS:

UNIF. W= 0.230 K/FT
CONC. P= -3.60 K, X= 7.83 FT
LEFT END MOMENT -0.70 FT-K
RIGHT END MOMENT -2.65 FT-K

BEAM MOMENTS AND SHEAR:

X DIST	D.L. MOMENT	MAX. MOMENT	MIN. MOMENT	POS. V
0.0	-2.4	-2.4	-3.6	4
1.0	-0.3	-0.3	-0.9	2
2.0	-0.1	-0.1	-0.5	1
3.0	-1.8	-1.8	-2.5	3
3.9	-5.4	-5.4	-6.7	6
4.9	-10.9	-10.9	-13.7	9
5.9	-18.3	-18.3	-22.2	10
6.9	-27.7	-27.7	-33.5	13
7.9	-37.3	-37.3	-46.1	20
8.9	-21.8	-21.8	-26.3	18
9.9	-8.2	-8.2	-9.8	16

11.8	10.4	10.4	11.1	8
12.9	21.1	25.7	27.0	6
13.8	27.0	32.8	31.9	4
15.8	33.1	40.2	33.1	1
16.8	33.3	40.4	33.3	1
17.8	31.6	38.3	31.6	3
18.7	28.0	33.8	28.0	6
19.7	22.4	27.0	22.4	8
20.7	14.9	17.9	14.9	10
21.7	5.6	6.4	5.6	13
22.7	-5.3	-5.3	-7.3	15
23.7	-18.9	-18.9	-23.5	18

ADDITIONAL POINT:

X	-38.2	38.2	-16.2	24
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REACTIONS:

D.L. LT	3.14K RT	14.37K
MAX. L.L. LT	8.79K RT	3.14K
MIN. L.L. LT	0.00K RT	0.00K

Eric Adams 12-19-84

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BEAM I.D.: LEFT WALL Mult

BEAM SPAN 9.250 FEET
I OF BEAM 1728.00 IN⁴

LOAD TYPES AND CONFIGURATIONS:

SPAN DEAD LOADS:

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UNIF. W= 1.023 K/FT
RAMP X1= 0.00 X2= 9.25 W= 1.28
LEFT END MOMENT -9.00 FT-K
RIGHT END MOMENT -8.14 FT-K

BEAM MOMENTS AND SHEAR:

X DIST	D.L. MOMENT	MAX. MOMENT	MIN. MOMENT	ABS V
0.0	-9.0	-9.0	-9.0	7
0.9	-3.2	-3.2	-3.2	6
2.8	0.4	0.4	0.4	3
3.7	8.0	8.0	8.0	2
4.6	9.2	9.2	9.2	1
6.5	7.3	7.3	7.3	3
7.4	3.9	3.9	3.9	5
8.3	-1.2	-1.2	-1.2	7
9.3	-8.1	-8.1	-8.1	9

REACTIONS:

D L: LT 6.79K RT 8.57K
MIN L L: LT 0.00K RT 0.00K

*****JAG 1/23/81*****